MASTER WASTEWATER REPORT FOR DEVELOPMENT UNITS 8 & 9 AT EASTMARK

January 15, 2014 WP# 123835.04

REVIEWED BY Lury Sailly DATE



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TABLE OF CONTENTS

| 1.0 | INTR | ODUCTION1 |
|-----------------|-------------------|---|
| | 1.1 | General Background and Project Location |
| | 1.2 | Scope of the Master Wastewater Report1 |
| | 1.3 | Master Wastewater Plan for Eastmark |
| | 1.4 | Study Area and Development Units2 |
| | 1.5 | Basis of Design Reports for Specific Individual Developments2 |
| 2.0 | EXIST | TING CONDITIONS3 |
| | 2.1 | Topographic Conditions3 |
| | 2.2 | Existing Offsite Wastewater Infrastructure |
| | 2.3 | Onsite Wastewater Collection Systems3 |
| 3.0 | WAST | EWATER SYSTEM DESIGN4 |
| | 3.1 | Design Criteria4 |
| | 3.2 | Wastewater Design Flows4 |
| 4.0 | PROP | OSED SYSTEM6 |
| | 4.1 | Planned Wastewater Infrastructure6 |
| | | 4.1.1 Ray Sewer Drainage Basin |
| | 4.2 | Pipe Sizing6 |
| | 4.3 | Sewer Line Infrastructure Phasing |
| 5.0 | CONC | LUSIONS8 |
| | | <u>TABLES</u> |
| | Table I | Wastewater Design Criteria |
| | Table 2 | /t a |
| | Table 3 | |
| | Table 4 | |
| | Table 5 Table 6 | |
| | Table 7 | |
| | Table 8 | Calculated Pipe Capacities, Phase 1 |
| | | PLATES (20952 DARRELL D.) |
| | Plate 1 | Vicinity Map |
| | Plate 2 | Master Sewer Exhibit, Full Build-Out Condition |
| | Plate 3 | Master Sewer Exhibit, Phase 1 |
| km Y:\WP\Rep | orts\Residential\ | Master Sewer Exhibit, Full Build-Out Condition Master Sewer Exhibit, Phase 1 123835.04 Eastmark DU 8 & 9 Master Wastewater Report.docx |

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1.0 INTRODUCTION

1.1 General Background and Project Location

The proposed Development Units 8 & 9 (Site) is anticipated to be an approximate 527-acre Development Unit (DU) within the 3,155-acre Eastmark master planned community, in Mesa, Arizona. It is a Planned Community District (PCD) which will include single-family residential, active-adult residential, various community uses, and open spaces.

The Master Wastewater Report has been prepared in accordance with Wood, Patel & Associates, Inc. (Wood/Patel's) understanding of the City of Mesa's technical requirements for wastewater collection systems, as applicable for Eastmark.

The Site is located within portions of Section 26, Township 1 South, Range 7 East of the Gila and Salt River Meridian. The Site is bounded by Williams Field Road and Pacific Proving Grounds to the south, Ray Road and the Powerline Floodway on the north, Signal Butte Road to the east, and the Crismon Road alignment and Pacific Proving Grounds on the west (refer to Plate 1 - Vicinity Map).

1.2 Scope of the Master Wastewater Report

The DU 8 & 9 Master Wastewater Report presents wastewater design flows and sewer main sizes and locations, as required, to provide wastewater service to the Site during initial and full build-out conditions. The purpose of this report is to provide a sewer analysis to reflect the developed condition of DU 8 & 9 based on a Conceptual Land Use Plan provided by TerraWest Communities, LLC. It is the goal of this DU 8 & 9 Master Wastewater Report to identify the sewers required to serve the Site, while meeting the requirements of the City's Engineering and Design Standards.

Updates to the DU 8 & 9 Master Wastewater Report may be required if significant changes are made to the land uses and assumptions utilized to prepare this report. Additionally, design criteria may change based on actual wastewater generation to calculate demand on the system in the future.



1.3 Master Wastewater Plan for Eastmark

The Master Wastewater Report Update for Eastmark, by Wood, Patel & Associates, Inc., dated May 17, 2013, was submitted to the City of Mesa for review and approval. The master report set the design criteria required within Eastmark, and set sewer basin boundaries tributary to the Elliot Road, Warner Road, and Ray Road offsite sewers. The Master Wastewater Report Update for Eastmark is currently being updated by Wood/Patel to reflect the wastewater collection system within this report, in addition to new information for other development units within Eastmark, and will be submitted to the City of Mesa for review and re-approval.

1.4 Study Area and Development Units

The study area includes the Ray Road and Williams Field Road Sewer Drainage Basins, per the *City of Mesa Wastewater Master Plan Update*, 2009. For a detailed breakdown of modeled land use areas, refer to the following:

- Table 2 Eastmark Modeled Land Use
- Table 3 DU 8 & 9 Land Use, Full Build-Out Condition
- Table 6 DU 8 & 9 Land Use, Phase 1
- Plate 2 DU 8 & 9 Master Sewer Exhibit Full Build-Out Condition
- Plate 3 DU 8 & 9 Master Sewer Exhibit Phase 1

1.5 Basis of Design Reports for Specific Individual Developments

As development progresses within the Site, Basis of Design (BOD) reports are required for specific individual developments to ensure compliance with the Master Report and the Development Unit Master Report, and to identify significant variations in land use, wastewater flows, and the wastewater infrastructure needed to serve the parcel.



2.0 EXISTING CONDITIONS

2.1 Topographic Conditions

The majority of the Site is surrounded by undeveloped desert and test tracks along the northern and western boundaries. The Site is bordered on the east by Bella Via and SB105 subdivisions, which are currently under design. The land generally slopes in a southwesterly direction at approximately 0.5 to 1 percent. The peak elevation within the Site is approximately 1,435 feet above mean sea level (MSL), located near the intersection of Signal Butte Road and Ray Road. The lowest elevation within the Site is approximately 1,410 feet MSL, located near the future intersection of Williams Field and Crismon Roads.

2.2 Existing Offsite Wastewater Infrastructure

Existing public wastewater infrastructure in the vicinity of the Site includes the following:

- An existing 12-inch gravity sewer located along Mountain Road, between Elliot Road and Pecos Road.
- An existing 12-inch gravity sewer located along Signal Butte Road, between Elliot Road and Galveston Road.
- A 27-inch and 30-inch gravity sewer located along Ray Road, between Ellsworth Road and the East Mesa Interceptor (EMI).
- An 18-inch and 21-inch gravity sewer along the Ray Road alignment north of the Powerline Floodway, between Signal Butte Road and Ellsworth Road.

2.3 Onsite Wastewater Collection Systems

The sewer outfall for DU 8 & 9 shall be at the intersection of Ray Road and Inspirian Parkway. An 18-inch sewer line has been designed to extend east from an existing 21-inch sewer line in the south half-street of Ray Road, east of Ellsworth Road, to Inspirian Parkway. This line will be constructed prior to or concurrently with the Site to provide a sewer outfall.



3.0 WASTEWATER SYSTEM DESIGN

3.1 Design Criteria

Wastewater design flows and pipe-sizing criteria utilized in this DU 8 & 9 Master Wastewater Report are based on Wood/Patel's understanding of the following:

- The Master Wastewater Report for Eastmark,
- Applicable wastewater system design criteria listed in the 2012 City of Mesa Engineering & Design Standards,
- Regionally accepted design standards,
- Title 18, Chapter 9 of the *Arizona Administrative Code*.

Table 1 – *Wastewater Design Criteria* presents the Unit Daily Wastewater Flow for each land use category, based on density and population. This design criterion is used in Table 3 – *DU 8 & 9 Land Use, Full Build-Out Condition* and Table 6 – *DU 8 & 9 Land Use, Phase 1* to determine the Unit Daily Wastewater Flow based on a conceptual land use plan. Parcels 9-1 through 9-7 are part of a proposed Active Adult community; therefore, the population density is assumed to be 2 persons per dwelling unit, in lieu of 3 persons per dwelling unit, as specified within the *City of Mesa Engineering & Design Standards* to reflect a realistic estimation of peak flows. The wastewater flow criteria are used to estimate the wastewater design flows and determine pipe sizes.

3.2 Wastewater Design Flows

Wastewater design flows for DU 8 & 9 are estimated using the design criteria listed above and the *City of Mesa 2025 General Plan*. Additionally, sewer service will be extended to the SB105 development, east of DU 8 & 9; therefore, wastewater flows from SB105 have been calculated and accounted for within this report. Projected full build-out average-day wastewater flows for DU 8 & 9 Full Build-Out and Phase 1 are summarized as follows, in millions of gallons per day (MGD):

Full Build-Out

| | DU 8 & 9 | Additional Eastmark | SB105 | Offsite | Total |
|----------------------|----------|------------------------|----------|----------|----------|
| Ray Road Outfall: | 0.28 MGD | 0.57 MGD | 0.08 MGD | 1.04 MGD | 1.98 MGD |
| Elliot Road Outfall: | 0.00 MGD | 0.76 MGD | 0.00 MGD | 0.00 MGD | 0.76 MGD |
| Total: | 0.28 MGD | 1.33 MGD | 0.08 MGD | 1.04 MGD | 2.74 MGD |



Phase 1

| | | Additional | | | |
|----------------------|----------|------------|----------|----------|----------|
| | DU 8 & 9 | Eastmark | SB105 | Offsite | Total |
| Ray Road Outfall: | 0.12 MGD | 0.57 MGD | 0.00 MGD | 1.04 MGD | 1.73 MGD |
| Elliot Road Outfall: | 0.00 MGD | 0.76 MGD | 0.00 MGD | 0.00 MGD | 0.76 MGD |
| Total: | 0.12 MGD | 1.33 MGD | 0.00 MGD | 1.04 MGD | 2.49 MGD |

Sewer pipe capacities are based upon conveying the flow at two-thirds of the pipe capacity. It is Wood/Patel's understanding that wet-weather infiltration is accounted for within the City of Mesa peaking factors listed in the 2012 City of Mesa Engineering & Design Standards.

Detailed design flow calculations are provided in Table 4 – Wastewater Model, Full Build-Out Condition, Table 5 – Calculated Pipe Capacities, Full Build-Out Condition, Table 7 – Wastewater Model, Phase 1, and Table 8 – Calculated Pipe Capacities, Phase 1. Wood/Patel utilized criteria within the 2012 City of Mesa Engineering & Design Standards based on static peaking methodology to calculate peak wet-weather flows for Eastmark. Static methodology is required by the City on an individual project basis to size onsite sewer lines.

It is our understanding the City utilized a diurnal peaking methodology to evaluate the overall tributary area, including Eastmark, to aid in the design of the Ray Road sewer line, from Ellsworth Road to the EMI. Diurnal peaking methodology is based on observed and/or estimated daily wastewater flow cycles for comparable developed areas, and is generally less conservative than static modeling resulting in lower peak flows. As a result, the peak wet-weather flows calculated in this report for Eastmark may vary from those used in designing the Ray Road sewer line. The controlling section of the Ray Road sewer is an offsite 30-inch pipe at 0.2 percent slope. The capacity of this pipe flowing full is 11.94 MGD, and at d/D = 0.9 is 12.7 MGD. Therefore, the peak wet-weather flows for Eastmark would not exceed the capacity of the Ray Road sewer.



4.0 PROPOSED SYSTEM

4.1 Planned Wastewater Infrastructure

DU 8 & 9 is proposed to contribute wastewater flow to the Ray Road Sewer Drainage Basin. Currently, the offsite Ray Road sewer has been constructed downstream of Eastmark, from Ellsworth Road to the EMI. The 18-inch Ray Road sewer, proposed to serve DU 8 & 9, is currently in the review process with the City of Mesa.

4.1.1 Ray Sewer Drainage Basin

The Ray Road Sewer Drainage Basin at Ellsworth Road receives flow from an existing diversion structure at Mountain Road. All flow north of Ray Road is currently diverted to the Ray Road Sewer, while flow from development south of Ray Road is conveyed south to Pecos Road. It is the City's intent to continue this mode of operation to provide additional capacity in the Pecos Road Sewer for future development along Pecos Road. This report considers the total design flow from the existing and proposed developments east of Eastmark for the Ray Road sewer contributing full build-out flow at this time. A portion of the upstream flow is accounted for per the Master Wastewater Report for Ray Road Sewer between Ellsworth and Mountain Roads, prepared by CMX, L.L.C., dated November 18, 2005. Additionally, SB105 has been included in the Ray Road Sewer Basin, which was previously included in the Williams Field Road Sewer Basin. Since the future Williams Field Road Sewer is planned to discharge into the Ray Road sewer at Ellsworth Road, the existing Ray Road Sewer west of Ellsworth Road was designed to accommodate wastewater flows from SB105. DU 7 is currently under construction, and is contributing wastewater flows to the Ray Road Sewer.

4.2 Pipe Sizing

Proposed sewer lines for the Site were sized to accommodate peak wet-weather flow conditions. The onsite collection system includes planned sewer mains with diameters ranging from 8 inches to 18 inches. Refer to Tables 4 and 5 for the Wastewater Model and Calculated Pipe Capacities, and Plate 2 for the planned DU 8 & 9 wastewater infrastructure.



4.3 Sewer Line Infrastructure Phasing

DU 8 & 9 is planned to be developed in phases. A preliminary phase (Phase 1) boundary is shown on Plate 3 – *Master Sewer Exhibit, Phase 1*. Phase 1 includes Parcels 8-1 through 8-5, and 9-1 through 9-3. The analysis in this report identified the required wastewater collection system infrastructure to provide service based on preliminary land use. Local sewer lines for future individual parcels have not been analyzed within this report. Approximately 6,100 linear feet of 18-inch sewer along Ray Road west of Inspirian Parkway, and along Inspirian Parkway south of Ray Road, will be constructed prior to or concurrently with DU 8 & 9 to provide a sewer outfall for the Site. The proposed main trunk line infrastructure is listed below:

Phase 1

- Proposed collection sewers from the intersection of Ray Road and Inspirian Parkway to the Phase 1 boundary through Parcel 9-1 and along proposed South Winchester, will consist of approximately:
 - o 2,300 feet of 15-inch sewer
 - o 500 feet of 12-inch sewer
 - o 1,500 feet of 10-inch sewer
- Proposed Parcels 8-1 through 8-5 main collection sewer will consist of approximately:
 - o 6,700 feet of 8-inch sewer

Remaining DU 8 & 9

- Proposed Parcels 8-6 through 8-9 main collection sewer consisting of approximately:
 - o 2,200 feet of 8-inch sewer
- Proposed Parcels 9-4 through 9-7 collection sewer consisting of approximately:
 - o 300 feet of 10-inch sewer
 - o 4,100 feet of 8-inch sewer



5.0 CONCLUSIONS

The Master Wastewater Report for Development Units 8 & 9 at Eastmark presented herein meets City of Mesa standards and requirements, and serves as a guide for construction documents associated with the planned wastewater system. The following items highlight critical conclusions:

- 1. Development Units 8 & 9 is anticipated to be 469 acres within the 3,155-acre Eastmark master planned community annexed into the City of Mesa.
- 2. The wastewater system presented is based on the projected full build-out condition of the Site.
- 3. Wastewater design criteria are based on Wood/Patel's understanding of the 2012 City of Mesa Engineering & Design Standards, regionally-accepted design standards, and Title 18, Chapter 9 of the Arizona Administrative Code.
- 4. The approximate average daily flow generated at build-out by the Site is 0.28 MGD, per Section 3.2 of this report.
- 5. Proposed onsite sewer mains are sized to accommodate peak wet-weather design flow for the full build-out condition.
- 6. The planned public wastewater collection systems outfall into existing and proposed gravity sewer lines located along Ray Road.
- 7. Wood/Patel's model of the proposed wastewater system provides conveyance and capacity in conformance with City of Mesa's standards, and Title 18 of the *Arizona Administrative Code*.



Wastewater Design Criteria

CMX, 11/18/2005.

Project: Location: References: DU 8 8 9 at Eastmark

Mesa, Arizona

2012 City of Mesa Engineering Design Standards

CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Proj. Number: 123835.04 Proj. Engineer: Darrell Smith, P.E.

UNIT DAILY RESIDENTIAL WASTEWATER FLOWS WASTEWATER DESIGN UNIT DAILY DWELLING UNIT WASTEWATER FLOW\$ DENSITY POPULATION DENSITY (PER CAPITA) **FLOWS** PERSONS VALUE UNITS UNITS Units NOTES VALUE PER ACRE Value Units Value LAND USE CATEGORY LAND USE Low Density Residential GPD/ Persons GPD/AC 100 LDR-1 (LOR 0-1) DU / Acre DU 1.25 Person Persons/ GPD/ GPD/AC DU / Acre 200 LDR-2 LDR 0-1 & LDR 1-2 AVG. Low Density Residential DU 2.50 80 Person Persons GPD/AC DU / Acre 3.60 80 288 LDR-3 (LDR-1-2) 1,2 Source: Dwelling unit GPD/ Medium Density Residential Persons/ GPD/AC density divisions are DU / Acre 9.00 80 Person DU MDR-1 (MDR 2-4) 3.0 GPD/ based on City of Mesa Persons GPD/AC 2025 General Plan. Unli Person GPD/ MDR 2-4 & MDR 4-6 AVG. DU / Acre DΟ 12,50 80 1,000 MDR-2 Medium Density Residential (MDR 4-6) Persons/ wastewater flows are GPD/AC hased on the City of .280 5.0 DU / Acre 3.2 DU 16.00 80 Person MDR-3 Mesa 2012 Engineering Medium Density Residential GPD/ GPD/AC and Design Standards. MDR-4 (MDR 6-10) DU / Acre DΟ 17.55 Person 1.404 GPD/ High Density Residential Persons GPD/AC 22.00 80 1,760 (HDR 10-15) DU / Acre 2.0 שַׁם Person HOR-1 High Density Residential Persons/ GPD/ GPD/AC DU / Acre 28.90 2312 (HDR 15+) 17.0 1.7 80 Person HOR-2 DU Mixed Use/Residential (MUR) Personsi GPD/AC 15.0 DU / Acre 1.7 UG 25.50 80 Person 2.040 MUR-1 Residential UNIT DAILY NON-RESIDENTIAL WASTEWATER FLOWS WASTEWATER DESIGN UNIT DAILY **FLOWS** WASTEWATER (PER CAPITA) DWELLING UNIT DENSITY POPULATION DENSITY **FLOWS** PERSONS LAND USE VALUE UNITS VALUE UNITS PER ACRE Value Units Value Units NOTES GPD/ ROOM Hotel GPD/ GPD/ Source: City of Mesa 2012 Employees Commercial/Retail Engineering and Design Standards. 23.0 23.00 54 Employee AC GPO/ Office Acre Education/Civic/ Employees/ GPD/ Church 15.0 Acre 15.00 54 Employee 810 AC OFFSITE WASTEWATER DESIGN UNIT DAILY **FLOWS** WASTEWATER (PER CAPITA) DWELLING UNIT DENSITY POPULATION DENSITY **PERSONS FLOWS** UNITS NOTES Value Units LAND USE VALUE VALUE LIMITS PER ACRE Value Units GPO/ Employees/ GPD/ 14.00 756 14.0 54 Employee GPD/ AÇ. CC Acre Employees/ GPD AC GPO/ 23.0 Acre 23.00 Employee 1,242 Employees/ GPD/ 14.00 Employee 756 AC RC Acre Source: City of Mesa 2012 Employees/ GPD/ _ _ Engineering and Design Standards 432 AC GPD/ BPL 8.0 Acre 8.00 54 Employee and the City of Mesa 2025 General **Employees** GPD/ Employee 594 AC GPD/ Plan NC 11.0 11.00 Acre Employees/ GPD/ Acre п 7.00 Employee AC GPD/ Employees _ --AC GPD/ 15.0 15.00 54 Employee 810 MUE, <u>Acre</u> Employees GPD/ 810 15.0 15.00 Employee AC ĠI Source: Master Wastewater Report for Ray Road Sewer Between GPD/ Ellsworth and Mountain Roads, by

| Description | Value | Units | Note(s) |
|---|-------|--------|---------|
| General | | | |
| Minimum Velocity (d/D=2/3) | 2 | ft/sec | 1 |
| Maximum Flow Velocity (d/D=2/3) | 9 | ft/sec | 1 |
| Maximum Peak Flow Depth-to-Diameter Ratio (d/D) | 0.67 | + | |
| Minimum Pipe Diameter | 8 | 'n | 1 |
| Manning's "n" value | 0.013 | | 2 |
| Peaking Factor (ADF< 1.0 MG0) | 3 | | f |
| Peaking Factor (1.0 < ADF< 10.0 MGD) | 2.5 | | 1 |
| Peaking Factor (10.0 < ADF< 20.0 MGD) | 2.3 | | 1 1 |

1,040,576 GPD / 1470 Acres = 706 GPD/AC

OFFUPSTREAM

1. Per The City of Mesa 2012 Engineering & Design Standards

2. Title 18, Chapter 9 of the Arizona Administrative Code

Eastmark Modeled Land Use

Proj. Number: 123835.04 Proj. Engineer: Darrell Smith, P.E.

Project:

DU 8 & 9 at Eastmark

Mesa, Arizona Location:

Total Residential Units 3,572 Mixed Use Residential Units : Residential Total 954.8 3,572 HDR-2 ; HDR-1 410 37.3 MDR4 PRELIMINARY RESIDENTIAL LAND USE AND DWELLING UNIT BREAKDOWN MDR-3 148.4 699 MDR-2 22.4 8 MDR-1 726.7 2,364 LDR-3 20.0 8 LDR-2 EASTMARK and Use Acreage Dwelfing Units

| | _ = - | Γ | Τ | Τ | Γ | Τ | | Γ | Γ | Γ | |
|---|--|---|----------|---|---|---|-----------|---------|---------|---------|-----------|
| | Unit Daily Wastewater Flow ⁽³⁾ (GPD/AC) | : | <u> </u> | , | | | 11.680 | 903 | 646 | 574 | 1 |
| | Avg. Day Wastewater Development Flow Unit Flow Area (GPD) (AC) | | 1 | | ŀ | 1 | 65.0 | 581.5 | 198.8 | 270.5 | 1,115.8 |
| | Avg. Day Wastewater Flow (GPD) | _ | , | : | 1 | , | 759.168 | 524.952 | 128.400 | 155.324 | 1,567,844 |
| | Golf (AC) | , | , | , | , | , | - | 1 | : | - | |
| | Other (AC) | , | ** | 1 | | 1 | 1 | 35.3 | 6,8 | | 42.1 |
| | Civic (AC) | : | 1 | : | 1 | 1 | : | 2.5 | ı | 12.4 | 14.9 |
| | Church (AC) | 1 | | , | 1 | 1 | | 13.5 | | 1 | 13.5 |
| | Education (AC) | 1 | ļ | , | | | ı | 20.0 | , | ı | 20.0 |
| | Total Floor Area (sq. ft.) | - | 1 | ı | ı | 1 | 1,340,000 | 15,000 | , | - | 1,355,000 |
| | Gross Non-Residential ⁽²⁾⁽⁵⁾ (AC) | 1 | * | | | | 65.0 | 5.5 | 7 | ; | 70.5 |
| S | Keys ⁽¹⁾ | ı | | | 1 | 1 | - | ı | | ŀ | - |
| LCULATION | Total Dwelling Units | 1 | ı | ı | 1 | , | - | 2,129 | 535 | 806 | 3,572 |
| ER FLOW CA | Residential (AC) | 1 | ļ | - | , | , | , | 504.7 | 192.0 | 258.1 | 954.8 |
| VASTEWAT | Total Area (AC) | ' | - | - | , | 3 | 65.0 | 581,5 | 198.8 | 270.5 | 1,115.8 |
| EASTMARK - WASTEWATER FLOW CALCULATIONS | Development Total Area Residential (AC) | - | 2 | 3 | 4 | 5 | 9 | 7 | 8 | ð | Subtotal: |

(1) Anticipated number of "Keys" represents hotel and resort uses. This includes approximately 8 acres within DU-1 and 133 acres within DU-5. UNIT DAILY RESIDENTIAL WASTEWATER FLOWS⁽⁴⁾

(2) Non-residential wastewater flows are calculated based on net non-residential acreage. (3) Unit dally wastewater flow calculations do not include golf course acreage. UNIT DAILY WASTEWATER FLOWS

(4) See Table 1 - Wastewater Design Criteria for additional design criteria information.

(6) Non-residential acreage of approximately 260 acres for the First Solar site is not represented within Table 4 as it drains to the Elliot Road Sewer.

GPD/AC

8

LPR-1

Units

Value

LAND USE CATEGORY

Abbreviations: UNIT DAILY WASTEWATER
Value Units JNIT DAILY NON-RESIDENTIAL WASTEWATER FLOWS⁽⁹) GPD/AC GPD/AC GPD/AC GPD/AC GPD/AC GPD/AC GPD/AC GPD/AC 200 288 288 720 720 720 760 760 760 LAND USE LDR-3 LDR-3 MDR-2 MDR-4 MDR-1 HDR-1 MUR-1

GPD/ROOM

150

GPD/AC GPD/AC

810.

1,242

Commercial / Retail / Office

Hotel

(Net Area) Education/Civic/ Church

AC = Acres GPD = Gallons Per Day GPD/AC = Gallons Per Day Per Acre

DU 8 & 9 Land Use, Full Build-Out Condition

TABLE 3 - DU 8 & 9 LAND USE, FULL BUILD-OUT CONDITION

Proj. Number: 123835.04 Proj. Engineer: Darrell Smith, P.E.

Project: Location:

DU 8 & 9 at Eastmark Mesa, Arizona

| | Total Avg Day | 17,760 | 20,880 | 15,360 | 10,080 | ŀ | 21.840 | 17,760 | 9,360 | 15,360 | 30,240 | 15,840 | 10,044 | 25,440 | 23,200 | 14,400 | 36,160 | 283,724 |
|--|---------------------------------|--------|--------|--------|--------|----------|--------|--------|-------|--------|--------|--------|--------|--------|--------|--------|--------|--------------------|
| | Avg Day | 17,760 | 20,880 | 15,360 | 10,080 | 1 | 21,840 | 17,760 | 9,360 | 15,360 | 30,240 | 15,840 | 10,044 | 25,440 | 23,200 | 14,400 | 36,160 | l _ |
| | GPDC | 80 | 80 | 80 | 80 | : | 80 | 08 | 80 | 08 | 80 | 80 | 54 | 80 | 80 | 80 | 80 | |
| | Total Population | 222 | 261 | 192 | 126 | ı | 273 | 222 | 117 | 192 | 378 | 198 | 186 | 318 | 290 | 180 | 452 | 3607 |
| | Population Density (persons/ PA | 3 | 3 | 3 | 3 | ŀ | က | က | 3 | က | 2 | 2 | 15 | 2 | 2 | 2 | 2 | |
| | Land Use Du | MDR-1 | MDR-1 | MDR-1 | MDR-1 | PARK | MDR-1 | MDR-1 | LDR-3 | MDR-1 | MDR-1 | MDR-1 | CIVIC | MDR-1 | MDR-1 | MDR-2 | MDR-1 | |
| Z | | | | | | | _ | | | | | | | | | | | : |
| REAKDOW | Non-Residential Acres | 1 | - | - | | 6.8 | - | 1 | - | ŀ | 1 | - | 12.4 | - | | - | - | 19.2 |
| G UNIT BR | Density (DU/AC) | 3,23 | 2:90 | 2.59 | 2.01 | 1 | 3.86 | 2.62 | 1.95 | 2.95 | 3.50 | 3.11 | 1 | 3.22 | 3.64 | 4.02 | 3.72 | |
| ND DWELLIN | Residential Acres | 22.9 | 30.0 | 24.7 | 20.9 | t | 23.6 | 28.2 | 20.0 | 21.7 | 54.0 | 31.8 | 1 | 49.4 | 39.8 | 22.4 | 60.7 | 450.1 |
| LAND USE A | No. of DUs | 74 | 87 | 64 | 42 | | 91 | 74 | 39 | 64 | 189 | 66 | 1 | 159 | 145 | 06 | 226 | 1443 |
| PRELIMINARY LAND USE AND DWELLING UNIT BREAKDOWN | Parcel | 8-1 | 8-2 | 8-3 | 8-4 | 8-5 | 9-8 | 8-7 | 8-8 | 8-9 | 9-1 | 9-5 | 9-3 | 9-4 | 9-5 | မှ | 2-6 | DU 8 & 9 Totals |

¹⁾ Parcels 9-1 through 9-7 are part of a proposed Active Adult community therefore the assumption of 2 persons per dwelling unit in lieu of 3 persons per dwelling unit for the population density would produce a more accurate estimation of peak flows.

Wastewater Model, Full Build-Out Condition

CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Project: Location: References: DU 8 & 9 at Eastmark

Mesa, Arizona

City of Mesa 2012 Engineering and Design Standards Arizona Administrative Code, Title 18, Chapter 9

Proj. Number: 123835.04 Proj. Engineer: Darrell Smith, P.E.

| FROM NODE | TO NODE | SEWER AREA(S)/PARCEL SERVED | AREA SERVED (ACRES) | UNIT FLOW (GPD/AC) | PARCEL ADF (GPD) | SEWER NODE ADF (GPD) | TOTAL ADF (GPD) | PEAKING FACTOR | PEAK WET WEATHER FLOW (GPD) |
|--|----------------------|-----------------------------------|---------------------------|--|------------------------|----------------------------|-----------------------|-------------------|--------------------------------------|
| | ONT WOOM TO | ream Wastewater Flo | 25.3 | ************************************** | | C4 200 | 61.382 | 3.0 | 784,146 |
| R21 | R19 | 7-9 7-11 | 7.5 19.1 | 814 1340 | 6,102 25,600 | 61,382 | 01,362 | 9.0 | |
| | Contract Section | 7-6 | 26.8 | * 699 | 18,720 | | | | |
| R24 | R19 | 7-7 7-10 | 31.4 25.4 | 795 1:090 | 24,960 27,680 | 95,200 | 95,200. | 9.0 | 285,600 |
| | | 7-12 | 19.6 | 1,216 | 23,840 | | | | |
| R19* | F22 | 7 | 7. Z#. | | 30.00 75 a 10.00 | Table 2 (1 Law 2011) | 156.582 | 3.0 | 469,746 |
| | | 7-13 | 20.1 17.3 | 1,043 | 20,960 30,400 | | | | |
| R22 | R23 | 7-15 | 18.1 | 1,189 | 21,520 | 112,940 | 269,522 | 3.0 | 808,566 |
| S.P. | 9 | 7.23 | 20.0 | 1,760 | 35,200 | | | 5 | |
| | | 7-24 *** 7-16 | 6.0 25.4 | 810 945 | 4,860 24,000 | Marie Vice and a | | | |
| R23 | R26 | 7-17 | 23.4 | 800 | 18,720 | 58,920 | 328,442 | 3:0 | 985,326 |
| | . The second second | 7-22 7-27 | 20.0 | 810 | 16,200 | | | | |
| | CONTRACTOR OF STREET | 7-18 | 35,3 28.9 | 706 | 20:400 | | | | |
| 9 (53) - 1 (4 (4 (4 (4 (4 (4 (4 (4 (4 (4 (4 (4 (4 | | 7-19 | ≥ 25.1 | 937 | 23,520 | | | | 2×8000×0× |
| R26 | R27 | 7-20 7-21 | 20,1 22.1 | 955 923 | 19,200 20,400 | 92,430 | 420,872 | 3,0 | 1,262,616 |
| | | 7.25 | 25 | 821 | 2,052 | - 2 | | | |
| | | 7-26 | 5.5 | 1.247 | € 6858 ≪ | | 3 24 | | |
| | | OFFUPSTREAM* | 1,473 | 707 | 1,041,710 | | | | |
| R46 | R45 | PHASE 1 ADMIN | 15 | 3,200, 22 | 48,002 | 1 145 632 | 1,145,632 | 2.5 | 2.864.080 |
| | ייייי | 7-1 | 24.0 | 926 . 579 | 19,440 j 20,160 | 1.50 | , , , , , | | |
| 2 | 7 | 7-5 | 34.8 26.9 | 607 | 16.320 | | | | |
| R45 | R27 | 7-2 | 20.8 | 1,058 | 22,000 | 48,160 | 1 193 792 | 2.5 | 2:984,480 |
| R27 | R12 | 7-3 | 33.1 | 790 | 26,160 | | 1,614,664 | ,2:5 | 4,036,660 |
| R12 | R11 | 7. 9 J | | 6/8/ 4 /7/8 | 7.7 .0 14 | nit de s | 1.614,664 | 2.5 | 4,036,660 |
| R11 | R2 | , and a second | | | | ea ano | 1,614,664 | 2.5 | 4,036,660 |
| R42 R41 | R41 R40 | SB105 9-6 | 99.1 22.4 | 840 643 | 83,280 14,400 | 83,280 14,400 | 83,280 97,680 | 3.0 | 249,840 293,040 |
| R40 | R35 | 9-2 | 31.8 | 498 | 15,840 | 25,884 | 123,564 | 3.0 | 370,692 |
| 1140 | 1700 | 9-3 | 12.4 | 810 | 10,044 | 20,004 | 120,004 | 0.0 | 0,0,002 |
| R36C | R36A | 9-7 18% of 9-5 | 60.7 7.2 | <u>596</u> 578 | 36,160 4,160 | 48,800 | 48,800 | 3.0 | 146,400 |
| | 7,00 | 33% of 9-4 | 16.3 | 520 | 8,480 | | | | • |
| R36B | R36A | 82% of 9-5 | 32.6 | 584 | 19,040 | 36,000 | 36,000 | 3.0 | 108,000 |
| R36A | R35 | 67% of 9-4 | 33.1 | 512 | 16,960 | | 84,800 | 3.0 | 254,400 |
| R35 | R34 | | | | | | 208,364 | 3.0 | 625,092 |
| R34 | R32 | 18% of 9-1 | 9.7 | 561 | 5,440 | 5,440 | 213,804 | 3.0 | 641,412 |
| | | 8-6 8-7 | 23.6 28.2 | 925 630 | 21,840 17,760 | | | | |
| R39 | R38 A | 42% of 8-3 | 10.4 | 623 | 6,480 | 58,320 | 58,320 | 3.0 | 174,960 |
| nos | KJOA | 8-5 | 6.8 | | | 30,020 | 00,020 | 0.0 | 11-1,000 |
| | | 67% of 8-4 26% of 8-2 | 14,0 6.4 | 480 863 | 6,720 5,520 | | | | |
| | | 58% of 8-3 | 14.3 | 621 | 8,880 | | | | |
| R38B | R38A | 56% of 8-2 | 16.8 | 700 | 11,760 | 38,400 | 38,400 | 3.0 | 115,200 |
| R38A | R33 | 8-1 | 22.9 | 776 | 17,760 | | 96,720 | 3.0 | 290,160 |
| * 1001 <u>}</u> | 1100 | 8-9 | 21.7 | 708 | 15,360 | | ~~;·=V | V.V. | _551.00 |
| R37 | R33 | 8-8 | 20.0 | 468 | 9,360 | 31,680 | 31,680 | 3.0 | 95,040 |
| ·• | | 33% of 8-4 18% of 8-2 | 6.9 | 487 667 | 3,360 | · | | | |
| R33 | R32 | 10% DI 8-2 | 5.4 | 667 | 3,600 | | 128,400 | 3.0 | 385,200 |
| R32 | R31 | 42'% of 9-1 | 22.7 | 557 | 12,640 | 12,640 | 354,844 | 3.0 | 1,064,532 |
| R31 | R30 R2 | 40'% of 9-1 | 21.6 | 563 | 12,160 | 12,160 | 367,004 367,004 | 3.0 3.0 | 1,101,012 1,101,012 |
| R30 | | | | | - | 1 | | | |

CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Project: Location: References: DU 8 & 9 at Eastmark

Mesa, Arizona

City of Mesa 2012 Engineering and Design Standards

Arizona Administrative Code, Title 18, Chapler 9

Proj. Number: 123835.04 Proj. Engineer: Darrell Smith, P.E.

| FROM Node | TO NODE | SEWER AREA(S)/PARCEL SERVED | AREA SERVED (ACRES) | UNIT FLÓW (GPD/AC) | PARCEL ADF (GPD) | SEWER NODE ADF (GPD) | TOTAL ADF (GPD) | PEAKING FACTOR | PEAK WET WEATHER FLOW (GPD) |
|---------------------------|--------------------|-----------------------------------|---------------------------|-----------------------|------------------------|----------------------------|-----------------------|----------------|--------------------------------------|
| Ray Road Onsit | e And Offsite Upst | ream Wastewater Flo | ews | | | | | | |
| R1A | R1 | | | | | | 1,981,668 | 2.5 | 4,954,170 |
| R1 | RAY ROAD SEWER | | | | ₩- | | 1,981,668 | 2.5 | 4,954,170 |
| Total DU 8 & 9 F Sewer | low to Ray Road | | 469.3 | | 283,724 | 283,724 | 283,724 | | 851,172 |
| Total Eastmark : Sewer | Flow to Ray Road | | 2543.4 | | 856,678 | 856,678 | 856,678 | | 2,570,034 |
| Total Flow to Ra | y Road Outfall at | , | 2636.5 | | 1,981,668 | 1,981,668 | 1,981,668 | | 4,954,170 |

Calculated Pipe Capacities, Full Build-Out Condition

DU 8 & 9 at Eastmark

Project: Location: References:

TABLE 5 - CALCULATED PIPE CAPACITIES, FULL BUILD OUT CONDITION CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS Proj. Number: 123835.04 Proj. Engineer: Darrell Smith, P.E.

Mesa, Arizona ADEQ Bulletin No. 11 City of Mesa 2012 Engineering and Design Standards

| TO NOTES PIPE DIA MODELED PIPE PEAK WET AETHER WEATHER WEATHER WELDCITY CIPD) CIPD | | | | | | | | PEA | PEAK FLOW RESULTS | JLTS | |
|--|------------|-----------------|--|-----------------------|------------------------------------|---------------------------|---|-------------------------|--|---|--|
| Bright 12 0.00045 1,522,778 184/146 0.24 3.3 R19 R22 R19 8 0.0056 1,522,778 285,610 0.59 2.3 R23 R23 15 0.0026 1.522,778 489,746 0.039 2.7 R23 R23 15 0.0020 1.903,379 888,566 0.45 2.7 R27 R26 15 0.0020 1.903,379 888,566 0.59 2.7 R27 R27 16 0.0020 1.903,379 888,566 0.59 2.7 R27 R27 18 0.0020 1.903,379 888,480 0.59 2.7 R27 R27 18 0.0020 1.903,379 4.036,660 0.59 2.7 R41 10 0.0020 1.596,069 4.036,660 0.63 2.7 R41 10 0.0020 5.596,069 4.036,660 0.65 2.2 R36 R35 10 | FROM | TO | NOTES | PIPE DIA. (INCHES) | MODELED PIPE SLOPE (FT / FT) | PIPE CAPACITY (GPD) | PEAK WET WEATHER FLOW (GPD) | d/D (WET WEATHER) | FLOW VELOCITY (FT/S) AT d/D=2/3 | SURPLUS CAPACITY (WET WEATHER) | PERCENT OF CAPACITY (WET WEATHER) |
| R19 R2 0.00504 1.522,178 1841446 0.024 3.3 R22 R23 12 0.0050 1.522,778 2.66,000 0.050 2.7 R22 15 0.0020 1.502,379 98.8,566 0.59 2.7 R26 15 0.0020 1.903,379 98.8,566 0.55 2.7 R27 15 0.0020 1.903,379 98.8,526 0.55 2.7 R27 16 0.0020 1.903,379 98.8,526 0.55 2.7 R27 17 0.0020 1.903,379 1.262,616 0.59 2.7 R27 18 0.0020 1.903,379 4.036,660 0.65 2.7 R41 21 0.0020 5.596,069 4.036,660 0.63 4.0 R41 10 0.0020 5.596,069 4.036,660 0.63 2.3 R34 10 0.0020 5.596,069 4.036,660 0.63 2.3 R35 | Ray Road B | asin Pipe Sizes | | | Let | | 5. W. | | | | |
| R19 R22 12 0.0050 1562,778 285,600 0.50 2.8 R22 15 0.0020 1.562,778 888,566 0.45 2.7 R26 15 0.0020 1.903,379 885,586 0.51 2.7 R26 15 0.0020 1.903,379 885,586 0.51 2.7 R27 18 0.0024 6.596,609 4.036,660 0.63 4.0 R12 21 0.0029 5.596,099 4.036,660 0.63 4.0 R21 21 0.0029 5.596,099 4.036,660 0.63 4.0 R34 10 0.0029 5.596,099 4.036,660 0.63 4.0 R41 10 0.0030 5.596,099 4.036,660 0.63 2.2 R35 10 0.0030 5.596,099 4.036,660 0.63 2.2 R35 10 0.0020 5.596,099 4.036,660 0.63 2.2 R35 <td< th=""><th>77</th><th>E KIB</th><th>100 A 100 A</th><th>X</th><th>0.0045</th><th>1,522,778</th><th>184 146</th><th>0.24</th><th>ः ° 3.3ः ः</th><th>1,338,632</th><th>12.1%</th></td<> | 77 | E KIB | 100 A | X | 0.0045 | 1,522,778 | 184 146 | 0.24 | ः ° 3.3ः ः | 1,338,632 | 12.1% |
| R22 12 0.0045 1.52.776 469.746 0.39 3.3 R23 15 0.0020 1.903.379 888.866 0.45 2.7 R26 15 0.0020 1.903.379 888.866 0.51 2.7 R27 18 0.0020 1.903.379 9.85.326 0.51 2.7 R27 18 0.0020 1.903.379 2.864.080 0.65 2.7 R12 21 0.0029 5.586.089 4.036.660 0.63 4.0 R11 21 0.0039 5.586.089 4.036.660 0.63 4.0 R40 10 0.0030 5.586.089 4.036.660 0.63 4.0 R41 10 0.0030 5.586.089 4.036.660 0.63 2.3 R44 10 0.0027 740.224 249.840 0.40 2.3 R35A 8 0.0027 740.224 249.840 0.40 2.2 R35A 8 0.0033< | 8 R24 | | | zwyde 🗞 🚈 | 0.0050 | S 564,029 | 285,600 | 0.20 | 2.8 | 278.429 | 20.6% |
| R23 15 0.0020 1.903.379 808.566 0.45 2.7 R26 15 0.0020 1.903.379 808.526 0.51 2.7 R27 15 0.0020 1.903.379 9.85,326 0.51 2.7 R27 18 0.0020 1.903.379 9.86,326 0.51 2.7 R27 18 0.0020 6.623.956 2.864.080 0.46 6.4 R12 21 0.0020 5.710.307 2.84.480 0.51 5.5 R11 21 0.0020 5.596.099 4.036.660 0.63 4.0 R1 10 0.0027 7.40.224 2.93.040 0.40 2.3 R35 10 0.0027 7.40.224 293.040 0.40 2.2 R35 10 0.0027 7.40.224 293.040 0.40 2.2 R35 8 0.0023 4.51.224 1.46.400 0.39 2.2 R35 10 0.0023 <td>R19</td> <td>R22</td> <td>1. 52. 40.24 (S. 10.24 (S.</td> <td>12</td> <td>0.0045</td> <td>ΙΝ</td> <td>469,746</td> <td>0.39</td> <td>3.3</td> <td>1.053.032</td> <td>30.8%</td> | R19 | R22 | 1. 52. 40.24 (S. 10.24 (S. | 12 | 0.0045 | ΙΝ | 469,746 | 0.39 | 3.3 | 1.053.032 | 30.8% |
| R26 15 0.0020 1.903.379 985.326 0.51 2.7 R27 15 0.0020 1.903.379 1.626.016 0.59 2.7 R45 18 0.0020 5.710.307 2.664.01 0.46 6.4 R12 21 0.0029 5.596.069 4.036.660 0.653 4.0 R12 21 0.0020 5.596.069 4.036.660 0.653 4.0 R41 10 0.0020 5.596.069 4.036.660 0.653 4.0 R44 10 0.0027 7.40.224 2.984.480 0.43 2.3 R40 10 0.0027 7.40.224 2.98.40 0.40 2.3 R35 10 0.0027 7.40.224 2.98.40 0.40 2.3 R35 8 0.0027 7.40.224 2.49.84 0.40 2.3 R35 10 0.0027 7.40.224 2.49.84 0.42 2.2 R35 10 0.002 | R22 | | | . 15 · | 0.0020 | 1.903.379 | 808,566 | 0.45 | 2.7 | 1.094.813 | 42.5% |
| R27 15 0.0020 1.903.379 1.262.616 0.59 2.7 R45 18 0.0094 6.623.956 2.864.080 0.46 6.4 R72 21 0.0020 5.756.069 4.036.660 0.63 4.0 R1 21 0.0020 5.596.069 4.036.660 0.63 4.0 R4 10 0.0027 7.40.224 2.93.840 0.63 4.0 R35 10 0.0027 7.40.224 2.94.840 0.40 2.3 R35 10 0.0027 7.40.224 2.94.840 0.63 2.2 R35 10 0.0027 7.40.224 2.94.840 0.63 2.2 R35 10 0.0027 7.40.224 2.94.840 0.65 2.3 R35 8 0.0027 7.40.224 2.94.840 0.60 2.3 R35 1 0.0027 7.40.224 2.94.840 0.50 2.3 R34 1 0.0027 <td>R23</td> <td></td> <td>5 68475 8 5</td> <td>15</td> <td>0,0020</td> <td>1.903.379</td> <td>985,326</td> <td>0.51</td> <td>2.7</td> <td>918.053</td> <td>51.8%</td> | R23 | | 5 68475 8 5 | 15 | 0,0020 | 1.903.379 | 985,326 | 0.51 | 2.7 | 918.053 | 51.8% |
| R45 18 0.00094 6.623.956 2.864.080 0.46 6.4 R27 18 0.0070 5.710.307 2.884.480 0.651 5.5 R12 21 0.0020 5.596.069 4.036.660 0.653 4.0 R41 10 0.0030 5.596.069 4.036.660 0.63 4.0 R41 10 0.0027 7.40.224 2.49.840 0.40 2.3 R35 10 0.0027 7.40.224 2.49.840 0.40 2.3 R35 10 0.0027 7.40.224 2.49.840 0.40 2.3 R35 8 0.0027 7.40.224 370.692 0.50 2.2 R36 8 0.0033 451.224 146.400 0.40 2.2 R35 10 0.0033 451.224 146.400 0.42 2.2 R34 1 0.0025 7.41.224 146.400 0.43 2.2 R34 1 0.0025 | R26 | | | | 0.0020 | 1,903,379 | 1,262,616 | 0,59 | -2.7 | 640,763 | 66.3% |
| R27 18 0.0070 5.710.307 2.984.480 0.51 5.5 R12 21 0.0029 5.596.069 4.036.660 0.653 4.0 R11 21 0.0020 5.596.069 4.036.660 0.653 4.0 R41 10 0.0020 5.596.069 4.036.660 0.653 4.0 R35 10 0.0027 740.224 249.840 0.40 2.3 R35 10 0.0027 740.224 293.040 0.43 2.2 R36A 8 0.0027 740.224 293.040 0.43 2.2 R35A 8 0.0023 451.224 108.000 0.39 2.2 R35A 10 0.0025 704.975 254.400 0.42 2.2 R35A 8 0.0025 704.975 254.400 0.42 2.2 R34 15 0.0026 1.167.463 625.092 0.53 2.5 R38A 8 0.0028 | R46 | | | ે જે 18 ે | 0.0094 | 6,623,956 | 2.864.080 | 0.46 | 6.4 | 3.759.876 | 43.2% |
| R12 21 0.0029 5.596.069 4.036.660 0.63 4.0 R11 21 0.0030 5.596.069 4.036.660 0.63 4.0 R41 10 0.0027 740.224 249.80 0.40 2.3 R40 10 0.0027 740.224 249.80 0.40 2.3 R35 10 0.0027 740.224 230.040 0.40 2.3 R35 10 0.0027 740.224 249.80 0.40 2.3 R36 8 0.0033 451.224 146.400 0.43 2.2 R35 10 0.0033 451.224 146.400 0.42 2.2 R35 10 0.0033 451.224 146.400 0.42 2.2 R34 12 0.0035 704.975 254.400 0.44 2.2 R34 15 0.0025 704.975 254.400 0.43 2.2 R38 8 0.0026 777.885< | R45 | | | | 0.0070 | 5,710,307 | 2,984,480 | 0.51 | 5.5 | 2.725.827 | 52.3% |
| R11 21 0.0030 5.596,069 4,036,660 0.63 4,0 R41 10 0.0027 740,224 249,840 0.40 2.3 R41 10 0.0027 740,224 293,040 0.43 2.3 R36A 8 0.0027 740,224 370,692 0.50 2.3 R36A 8 0.0033 451,224 176,400 0.39 2.2 R35A 10 0.0025 1.167,463 625,092 0.50 2.2 R34 12 0.0025 1.167,463 625,092 0.42 2.5 R34 12 0.0025 1.167,463 641,412 0.43 2.3 R34 8 0.0038 473,785 174,960 0.43 2.3 R38 8 0.0038 767,080 115,200 0.27 3.8 R33 8 0.0038 473,785 9.040 0.31 2.3 R34 13 0.044 1.566, | R27 | | | 21 | 0 | 2,596,069 | 4.036.660 | 0.63 | 4.0 | 1.559.409 | 72.1% |
| R2 R2 21 0.0030 5.596,069 4,036,660 0.653 4,0 R41 10 0.0027 740,224 249,840 0.43 2.3 R35 10 0.0027 740,224 293,040 0.43 2.3 R36A 8 0.0027 740,224 146,400 0.39 2.2 R36A 8 0.0033 451,224 146,400 0.34 2.2 R35A 10 0.0025 704,975 254,400 0.42 2.5 R34 12 0.0025 704,975 254,400 0.42 2.5 R34 12 0.0025 1,167,463 625,092 0.53 2.5 R34 15 0.0014 1,586,149 641,412 0.44 2.2 R35 8 0.0038 473,785 174,960 0.43 2.3 R35 8 0.0038 473,785 95,040 0.31 2.4 R34 14 1,064,532 | R12 | | | ್ಟ ≅ 21 - ∵ | 0:0030 | 5,596,069 | 4.036,660 | 0.63 | 4.0 | 1.559.409 | 72.1% |
| R41 10 0.0027 740,224 249,840 0.40 2.3 R40 10 0.0027 740,224 283,040 0.43 2.3 R36A 8 0.0027 740,224 370,692 0.50 2.3 R36A 8 0.0033 451,224 146,400 0.39 2.2 R36A 8 0.0035 704,975 254,400 0.42 2.2 R34 12 0.0025 7.167,463 625,092 0.53 2.5 R34 15 0.0025 1.167,463 625,092 0.53 2.5 R34 15 0.0014 1.586,149 641,412 0.44 2.2 R38A 8 0.0038 473,785 174,960 0.44 4.3 R33 8 0.0038 473,785 1.506 0.44 2.3 R33 8 0.0040 496,346 385,200 0.67 2.4 R31 18 0.0040 496,346 </td <td>RIT</td> <td>) 75</td> <td>200</td> <td></td> <td>0:0030</td> <td>5,596,069</td> <td>4,036,660</td> <td>∴0:63</td> <td>4.0</td> <td>1.559,409</td> <td>72.1%</td> | RIT |) 75 | 200 | | 0:0030 | 5,596,069 | 4,036,660 | ∴0:63 | 4.0 | 1.559,409 | 72.1% |
| R40 10 0.0027 740,224 293,040 0.43 2.3 R35A 10 0.0027 740,224 370,692 0.50 2.3 R36A 8 0.0033 451,224 146,400 0.39 2.2 R34 10 0.0035 451,224 146,400 0.39 2.2 R34 10 0.0025 704,975 254,400 0.42 2.2 R34 12 0.0025 1,167,463 625,092 0.53 2.5 R38A 8 0.0038 473,785 174,960 0.43 2.3 R33 8 0.0038 767,080 115,200 0.27 3.8 R33 8 0.0038 473,785 95,040 0.31 2.3 R33 8 0.0040 496,346 385,200 0.60 2.2 R34 18 0.0044 1,586,149 1,064,532 0.60 2.2 R34 18 0.0044 2,284,12 | R42 | R41 | | 10 | 0.0027 | 740,224 | 249,840 | 0.40 | 2.3 | 490.384 | 33.8% |
| R35 10 0.0027 740,224 370,692 0.50 2.3 R36A R36A 8 0.0033 451,224 146,400 0.39 2.2 R36A R36A 8 0.0033 451,224 108,000 0.34 2.2 R35 R34 12 0.0025 1.167,463 625,092 0.53 2.2 R32 R32 8 0.0045 1.167,463 641,412 0.44 2.2 R38 0.0014 1.586,149 641,412 0.44 2.2 R34 8 0.0038 767,080 115,200 0.44 2.3 R33 8 0.0038 767,080 115,200 0.40 4.3 R33 8 0.0040 496,346 385,200 0.67 2.4 R34 15 0.0040 496,346 385,200 0.67 2.2 R34 18 0.0040 2.284,123 1.101,012 0.48 2.2 R3 </td <td>R41</td> <td>R40</td> <td></td> <td>10</td> <td>0.0027</td> <td>740,224</td> <td>293,040</td> <td>0.43</td> <td>2.3</td> <td>447,184</td> <td>39.6%</td> | R41 | R40 | | 10 | 0.0027 | 740,224 | 293,040 | 0.43 | 2.3 | 447,184 | 39.6% |
| R36A 8 0.0033 451.224 146,400 0.39 2.2 R36A 8 0.0033 451,224 108,000 0.34 2.2 R35 10 0.0025 704,975 254,400 0.42 2.2 R34 12 0.0025 1.167,463 625,092 0.53 2.5 R34 15 0.0014 1.586,149 641,412 0.44 2.2 R34 8 0.0014 1.586,149 641,412 0.44 2.2 R33 8 0.0038 767,080 174,960 0.43 2.3 R33 8 0.0127 879,886 290,160 0.40 4.3 R33 8 0.0038 473,785 95,040 0.31 2.4 R31 15 0.0040 436,346 385,200 0.67 2.4 R30 18 0.0040 2.284,123 1,101,012 0.48 2.2 R4 21 16,0995,040 0. | R40 | R35 | | 10 | 0.0027 | 740,224 | 370,692 | 0.50 | 2.3 | 369,532 | 50.1% |
| R36A 8 0.0033 451,224 108,000 0.34 2.2 R35 10 0.0025 704,975 254,400 0.42 2.2 R34 12 0.0025 1.167,463 625,092 0.53 2.5 R34 15 0.0014 1.586,149 641,412 0.44 2.2 R38 8 0.0014 1.586,149 641,412 0.44 2.3 R38 8 0.0038 473,785 174,960 0.43 2.3 R33 8 0.00127 879,886 290,160 0.40 4.3 R34 8 0.0038 473,785 95,040 0.31 2.3 R34 8 0.0040 496,346 385,200 0.67 2.4 R34 15 0.0041 1,586,149 1,064,532 0.60 2.2 R30 18 0.0011 2,284,123 1,101,012 0.48 2.2 R3 18 0.0011 2,2 | R36C | R36A | | Ø | 0.0033 | 451,224 | 146,400 | 0.39 | 2.2 | 304,824 | 32.4% |
| R35 10 0.0025 704,975 254,400 0.42 2.2 R34 12 0.0025 1,167,463 625,092 0.53 2.5 R32 15 0.0014 1,586,149 641,412 0.44 2.2 R38A 8 0.0038 473,785 174,960 0.43 2.3 R33 8 0.0038 767,080 115,200 0.27 3.8 R33 8 0.00127 879,886 290,160 0.40 4.3 R34 8 0.0038 473,785 95,040 0.31 2.3 R34 15 0.0040 496,346 385,200 0.67 2.2 R34 18 0.0014 1,586,149 1,064,532 0.60 2.2 R2 18 0.0014 2,284,123 1,101,012 0.48 2.2 R34 18 0.0011 2,284,123 1,101,012 0.48 2.2 R1A 11 0.001 | R36B | R36A | | ထ | 0.0033 | 451,224 | 108,000 | 0.34 | 2.2 | 343,224 | 23.9% |
| R34 12 0.0025 1,167,463 625,092 0.53 2.5 R32 15 0.0014 1,586,149 641,412 0.44 2.2 R38A 8 0.0038 473,785 174,960 0.43 2.3 R33 8 0.0098 767,080 115,200 0.27 3.8 R33 8 0.0127 879,886 290,160 0.40 4.3 R33 8 0.0127 879,886 290,160 0.31 2.3 R34 8 0.0038 473,785 95,040 0.31 2.3 R31 15 0.0040 496,346 385,200 0.67 2.4 R30 18 0.0014 1,586,149 1,064,532 0.60 2.2 R2 18 0.0011 2,284,123 1,101,012 0.48 2.2 R2 18 0.0014 2,284,123 1,101,012 0.62 2.2 R3 10,004 22 1,8 | R36A | R35 | | 10 | 0.0025 | 704,975 | 254,400 | 0.42 | 2.2 | 450,575 | 36.1% |
| R32 15 0.0014 1,586,149 641,412 0.44 2.2 R38A 8 0.0038 473,785 174,960 0.43 2.3 R38A 8 0.0098 767,080 115,200 0.27 3.8 R33 8 0.0127 879,886 290,160 0.40 4.3 R33 8 0.0127 879,886 290,160 0.31 2.3 R34 8 0.0038 473,785 95,040 0.31 2.3 R31 15 0.0040 496,346 385,200 0.67 2.4 R30 18 0.0014 1.586,149 1.064,532 0.60 2.2 R2 18 0.0011 2,284,123 1,101,012 0.48 2.2 R2 18 0.0011 2,284,123 1,101,012 0.48 2.2 R1A PEPP PLAN 21 0.0046 4954,170 0.43 2.2 | R35 | R34 | | 12 | 0.0025 | 1,167,463 | 625,092 | 0.53 | 2.5 | 542,371 | 53.5% |
| R38A 8 0.0038 473,785 174,960 0.43 2.3 R38A 8 0.0098 767,080 115,200 0.27 3.8 R33 8 0.0127 879,886 290,160 0.40 4.3 R33 8 0.0127 879,886 290,160 0.40 4.3 R34 8 0.0038 473,785 95,040 0.31 2.3 R31 15 0.0040 496,346 385,200 0.67 2.4 R30 18 0.0014 1.586,149 1.064,532 0.60 2.2 R2 18 0.0011 2,284,123 1,101,012 0.48 2.2 R1A 21 0.0046 6,0046 6,0044 7.407,124 4,054,170 0.43 7.7 | R34 | R32 | | 15 | 0.0014 | 1,586,149 | 641,412 | 0.44 | 2.2 | 944,737 | 40.4% |
| R38A 8 0.0098 767,080 115,200 0.27 3.8 R33 8 0.0127 879,886 290,160 0.40 4.3 R33 8 0.0127 873,785 95,040 0.31 2.3 R32 8 0.0040 496,346 385,200 0.67 2.4 R31 15 0.0014 1.586,149 1.064,532 0.60 2.2 R30 18 0.0011 2,284,123 1,101,012 0.48 2.2 R1A 21 0.0046 6,00395,087 4,954,170 0.62 5.0 R1A PEP PILAN 21 0.0046 73,407,124 4,054,170 0.42 7.7 | R39 | R38A | | & | 0.0038 | 473,785 | 174,960 | 0.43 | 2.3 | 298,825 | 36.9% |
| R33 8 0.0127 679,886 290,160 0.40 4.3 R32 8 0.0038 473,785 95,040 0.31 2.3 R31 15 0.0040 496,346 385,200 0.67 2.4 R31 15 0.0014 1.586,149 1.064,532 0.60 2.2 R30 18 0.0011 2.284,123 1.101,012 0.48 2.2 R1 21 0.0014 2.284,123 1.101,012 0.48 2.2 R1 21 0.0046 6.0046 6.0046 7.407,124 4.054,170 0.05 5.0 | R38B | R38A | | ∞ | 0.0098 | 767,080 | 115,200 | 0.27 | 3.8 | 651,880 | 15.0% |
| R33 8 0.0038 473,785 95,040 0.31 2.3 R32 8 0.0040 496,346 385,200 0.67 2.4 R31 15 0.0014 1.586,149 1.064,532 0.60 2.2 R30 18 0.0011 2,284,123 1.101,012 0.48 2.2 R1A 21 0.0011 2,284,123 1.101,012 0.48 2.2 R1A 21 0.0046 6,0995,087 4,954,176 0.62 5.0 R1 24 0.0046 73,407,124 4,054,176 0.43 7.7 | R38A | R33 | | 8 | 0.0127 | 879,886 | 290,160 | 0.40 | 4.3 | 589,726 | 33.0% |
| R32 8 0.0040 496,346 385,200 0.67 2.4 R31 15 0.0014 1,586,149 1,064,532 0.60 2.2 R30 18 0.0011 2,284,123 1,101,012 0.48 2.2 R1A 18 0.0011 2,284,123 1,101,012 0.48 2.2 R1A 21 0.0046 6,0995,087 4,954,176 0.62 5.0 R1A PERPITAN 24 0.0046 73,407,124 4,054,170 0.43 7.7 | R37 | R33 | | æ | 0.0038 | 473,785 | 95,040 | 0.31 | 2.3 | 378,745 | 20.1% |
| R31 15 0.0014 1.586.149 1.064,532 0.60 2.2 R30 18 0.0011 2.284,123 1.101,012 0.48 2.2 R2 18 0.0011 2.284,123 1.101,012 0.48 2.2 R1A 21 0.0046 6.995,087 4.954,176 0.62 5.0 R1A PERPITAN 24 0.0046 7.3407,124 4.054,176 0.43 7.3 | R33 | R32 | | 8 | 0.0040 | 496,346 | 385,200 | 0.67 | 2.4 | | 77.6% |
| R30 18 0.0011 2.284,123 1,101,012 0.48 2.2 R2 18 0.0011 2.284,123 1,101,012 0.48 2.2 R1A 21 0.0046 6.995,087 4,954,170 0.62 5.0 R1 PER PI AN 24 0.064 170 0.43 7.5 | R32 | R31 | | 15 | 0.0014 | 1,586,149 | 1,064,532 | 09'0 | 2.2 | 521,617 | 67.1% |
| R1A R1A 0.0011 2.284.123 1,101,012 0.48 2.2 2.2 2.2 2.2 2.2 2.3 2.2 2.3 2. | R31 | R30 | | 18 | 0.0011 | 2,284,123 | 1,101,012 | 0.48 | 2.2 | 1,183,111 | 48.2% |
| R1A 24,770 0.62 5.0 5.0 8.0 8.0 8.0 8.0 8.0 8.0 8.0 8.0 8.0 8 | R30 | R 2 | | 18 | 0.0011 | 2,284,123 | 1,101,012 | 0.48 | 2.2 | 1,183,111 | 48.2% |
| CONTRACTOR OF THE CONTRACTOR O | -62 | RIA | | | 0.0046 | 6,995,087 | 4,954,170 | 🐔 📉 0.62 💸 🐣 | 5.0 | 2,040,917 | ~ 70.8% |
| | RIA | 2 | PER PLAN | 24° | 0.0081 | 13,197,121 | 4,954,170 | 0.42% | 7.2 | 8,242,951 | 37.5% |

Page 1

DU 8 & 9 Land Use, Phase 1

DU 8 & 9 at Eastmark Mesa, Arizona Project: Location:

Proj. Number: 123835.04 Proj. Engineer: Darrell Smith, P.E.

| | | | | | | | | | | _ |
|--|---|----------|--------|--------|--------|------|--------|--------|------------|-----------------|
| | Total Avg Day | 17,760 | 20,880 | 15,360 | 10,080 | - | 30.240 | 15.840 | 10.044 | 120,204 |
| | Avg Day | 17,760 | 20,880 | 15,360 | 10,080 | 1 | 30,240 | 15,840 | 10,044 | 120,204 |
| | GPDC | 80 | 80 | 80 | 80 | : | 80 | 80 | 54 | |
| | Total Population | 222 | 261 | 192 | 126 | - | 378 | 198 | 186 | 1563 |
| | Population Density (persons/ DU or Acre) | 3 | က | 3 | က | ı | 2 | 2 | 15 | |
| | Land Use | MDR-1 | MDR-1 | MDR-1 | MDR-1 | PARK | MDR-1 | MDR-1 | CIVIC | |
| BREAKDOWN | Non-Residential Acres | 2 | | _ | : | 6.8 | • | | 12.4 | 19.2 |
| | Density (DU/AC) | 3.23 | 2.90 | 2.59 | 2.01 | | 3.50 | 3.11 | | |
| ND DWELLIN | Residential Acres | 22.9 | 30.0 | 24.7 | 20.9 | ** | 54.0 | 31.8 | - | 184.3 |
| PRELIMINARY LAND USE AND DWELLING UNIT | No. of DUs | 74 | 87 | 64 | 42 | : | 189 | 99 | : | 555 |
| PRELIMINARY | Parcel | 8-1 | 8-2 | 8-3 | 8-4 | 8-5 | 9-1 | 9-2 | 6-3 | DU889 Totals |

1) Parcels 9-1 through 9-2 are part of a proposed Active Adult community therefore the assumption of 2 persons per dwelling unit in lieu of 3 persons per dwelling unit for the population density would produce a more accurate estimation of peak flows.

Wastewater Model, Phase 1

TABLE 7 - WASTEWATER MODEL, PHASE I CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Project: Location: References:

DU 8 & 9 at Eastmark
Mesa, Arizona
City of Mesa 2012 Engineering and Design Standards
Arizona Administrative Code, Title 18, Chapter 9

Proj. Number: 123835.04 Proj. Engineer: Darrell Smith, P.E.

| FROM Node | TO NODE | SEWER AREA(S)/PARCEL SERVED | AREA SERVED (ACRES) | UNIT FLOW (GPD/AC) | PARCEL ADF (GPD) | SEWER NODE ADF (GPD) | TOTAL ADF (GPD) | PEAKING FACTOR | PEAK WET WEATHER FLOW (GPD) |
|-----------------------------------|-------------------|--|---------------------------|-----------------------|--|---|-----------------------|-------------------|--------------------------------------|
| | | ream Wastewater Flo 7-8 | 25.3. | | 29,680 | | | | |
| R21 | R19 | 7-9 7-11 | 7.5 19.1 | 814 1,340 | 6.102 25,600 | 61,382 | 61,382 | 3.0 | 184,146 |
| | | 7-ë | 26.8 | 699 | 18,720 | | | ni i i | |
| R24,2 | R19 | 7-7 7-10 | 31.4 25.4 | 795 1,090 | 24,960 27,680 | 95,280 | 95,200 | 30 | 285,600 |
| R19 | R22 | 7-12 | 19.6 | 1,216 | 28,840 | | 156.582 | 30 | 469:746 |
| (8.7 | 2.00 | 740 | 20.1 | 1.043 | 20:960 | | 130,002 | - 26 XTXXXX | 702,130 |
| R22 | R23 | 7-147 7-15 | 17.3 18.1 | 1,757 1,189 | 30,400 21,520 | 112.940 | 269,522 | 3.0 | 808,566 |
| | | 7-23 | 20.6 | 1,760 | 35,200 | | | | 10 P |
| | | 7/24 7/16 | 6.0 25.4 | 810 945 | 4,860 24,000 | | | | |
| R23. | R26 | 7:17 | 23.4 | 800 | 18,720 | 58,920 | 328,442 | 3.0 | 985,326 |
| | l, 45. | 7-22 7-27 | 20.0 35.3 | 810 _ | 16,200 | 4 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 | | | |
| | 14.2 | 7-18 | 28.9 | 706 937 | 20,400 | | | | |
| R26 | R27 | 7-20 | 25,1 20.1 | 955 | 23,520 19,200 | 92,430 | 420,872 | 3,0 | ¥1,262,616 |
| | No. | 7-21 7-25 | 22.1 2.5 | 923 821 | 20,400 . 2,052 | | | | |
| | | 7-26 | 5.5 | 1,247 | 6.858 | | | | |
| | 888 9 6 6 5 6 | OFFUPSTREAM ⁽¹⁾ FIRST SOLAR | 1,473 | 707 | 1.041.710 | | | 7.888.538 | |
| *R46 | R45 | PHASE I ADMIN | 15 | 3,200 | 48,002 | 1.145.632 | 1,145,632 | 25 | 2,864,080 |
| | | 7-1 7-4 | 21.0 34.8 | 926 679 | 19,440 20,160 | | | | jaki. |
| | | 7.5 | 26.9 | 607 | 18,320 | | | | |
| R45 | R27 | 7-2 7-3 | 20.8 33.1 | 4,058 790 | 22,000 26,160 | 48,160 | 1.193,792 | 2.5 | 2,984,480 |
| R27 R42 | R12 | | | | ************************************** | Han nigar | 1,614,664 | 2.5 2.5 | 4,036,660 4,036,660 |
| %R11 | R2 | | | | | - | 1,614,664 | 2.5 | 4,036,660 |
| R41 | R40 | 9-2 | 31.8 | 498 | 15,840 | 00.004 | - | 3.0 | 77.050 |
| R40 | R35 | 9-3 | 12,4 | 810 | 10,044 | 25,884 | 25,884 | 3.0 | 77,652 |
| R35 R34 | R34 | 18% of 9-1 | 9.7 | 561 | 5,440 | 5,440 | 25,884 31,324 | 3.0 | 77,652 93,972 |
| 1,00 | | 42% of 8-3 | 10.4 | 623 | 6,480 | VI | | | |
| R39 | R38A | 8-5 67% of 8-4 | 6.8 14.0 | 480 | 6,720 | 18,720 | 18,720 | 3.0 | 56,160 |
| | | 26% of 8-2 | 7.8 | 708 | 5,520 | | | | |
| R38B | R38A | 58% of 8-3 56% of 8-2 | 14.3 16.8 | 621 700 | 8,880 11,760 | 38,400 | 38,400 | 3.0 | 115,200 |
| | | 8-1 | 22.9 | 776 | 17,760 | · | | | 474.000 |
| R38A | R33 | 33% of 8-4 | - 6.9 | 487 | 3,360 | | 57,120 | 3.0 | 171,360 |
| R37 | R33 | 18% of 8-2 | 5.4 | 667 | 3,600 | 6,960 | 6,960 | 3.0 | 20,880 |
| R33 R32 | R32 R31 | 42% of 9-1 | 22.7 | 557 | 12,640 | 12,640 | 64,080 108,044 | 3.0 | 324,132 |
| R31 | R30 | 40'% of 9-1 | 21.6 | 563 | 12,160 | 12,160 | 120,204 | 3.0 | 360,612 |
| R30 R2 | R1A | | - | | | - | 120,204 1,734,868 | 3.0 2.5 | 360,612 4,337,170 |
| R1A | Rí | | | | | - | 1,734,868 | 2.5 | 4,337,170 |
| R1 | RAY ROAD SEWER | | - | 1 | -1 | | 1,734,868 | 2.5 | 4,337,170 |
| otał DU 8 & 9 Pi ay Road Sewer | | | 203.5 | | 120,204 | 120,204 | 120,204 | | 360,612 |
| otal Eastmark F ewer | low to Ray Road | | 2279.0 | | 693,158 | 693,158 | 693,158 | | 2,079,474 |
| otal Flow to Ray | y Road Outfall at | | 2273.0 | | 1,734,868 | 1,734,868 | 1,734,868 | | 4,337,170 |

Calculated Pipe Capacities, Phase 1

Project: Location: References:

TABLE 8 - CALCULATED PIPE CAPACITIES, PHASE 1

CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Proj. Number: 123835.04 Proj. Engineer: Darrell Smith, P.E.

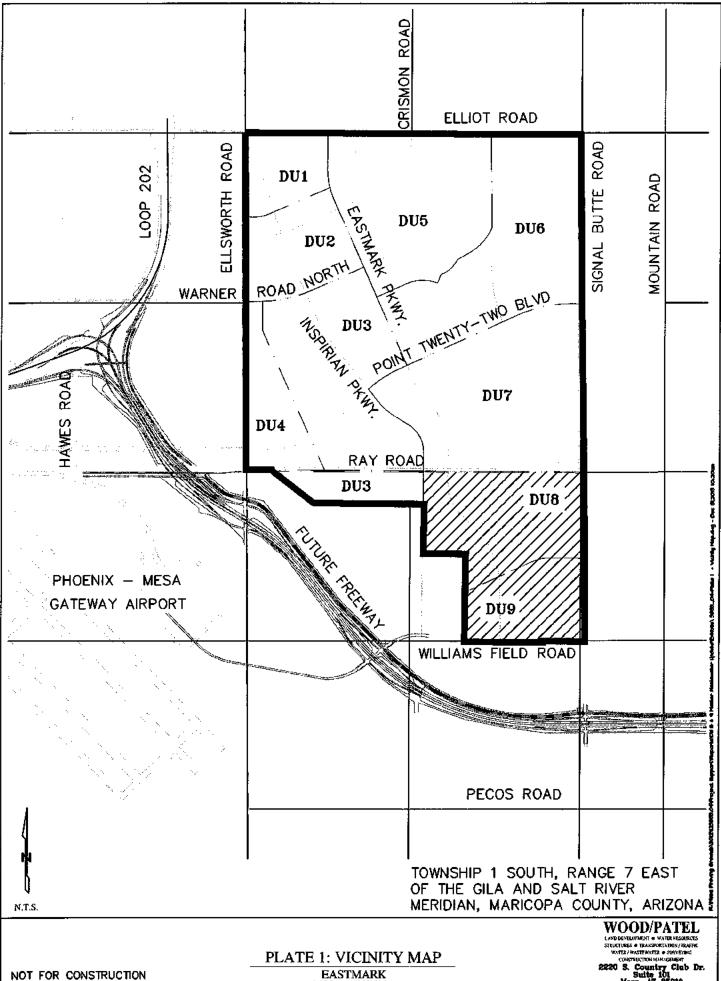
DU 8 & 9 at Eastmark Mesa, Arizona ADEQ Bulletin No. 11 City of Mesa 2012 Engineering and Design Standards

| MODELED PIPE SLOPE |
|-----------------------|
| (FT / FT) |
| |
| 0.0045 |
| 0.0050 |
| 0.0045 |
| 0.0020 |
| 0.0020 |
| 0.0020 |
| 0.0094 |
| 0.0070 |
| 0.0029 |
| 0.0030 |
| 0.0030 |
| 0.0027 |
| 0.0027 |
| 0.0025 |
| 0.0014 |
| 0.0038 |
| 0.0098 |
| 0.0127 |
| 0.0038 |
| 0.0040 |
| 0.0014 |
| 0.0011 |
| 0.0011 |
| 0.0046 |
| 0.0081 |

Page 1

PLATE 1

Vicinity Map

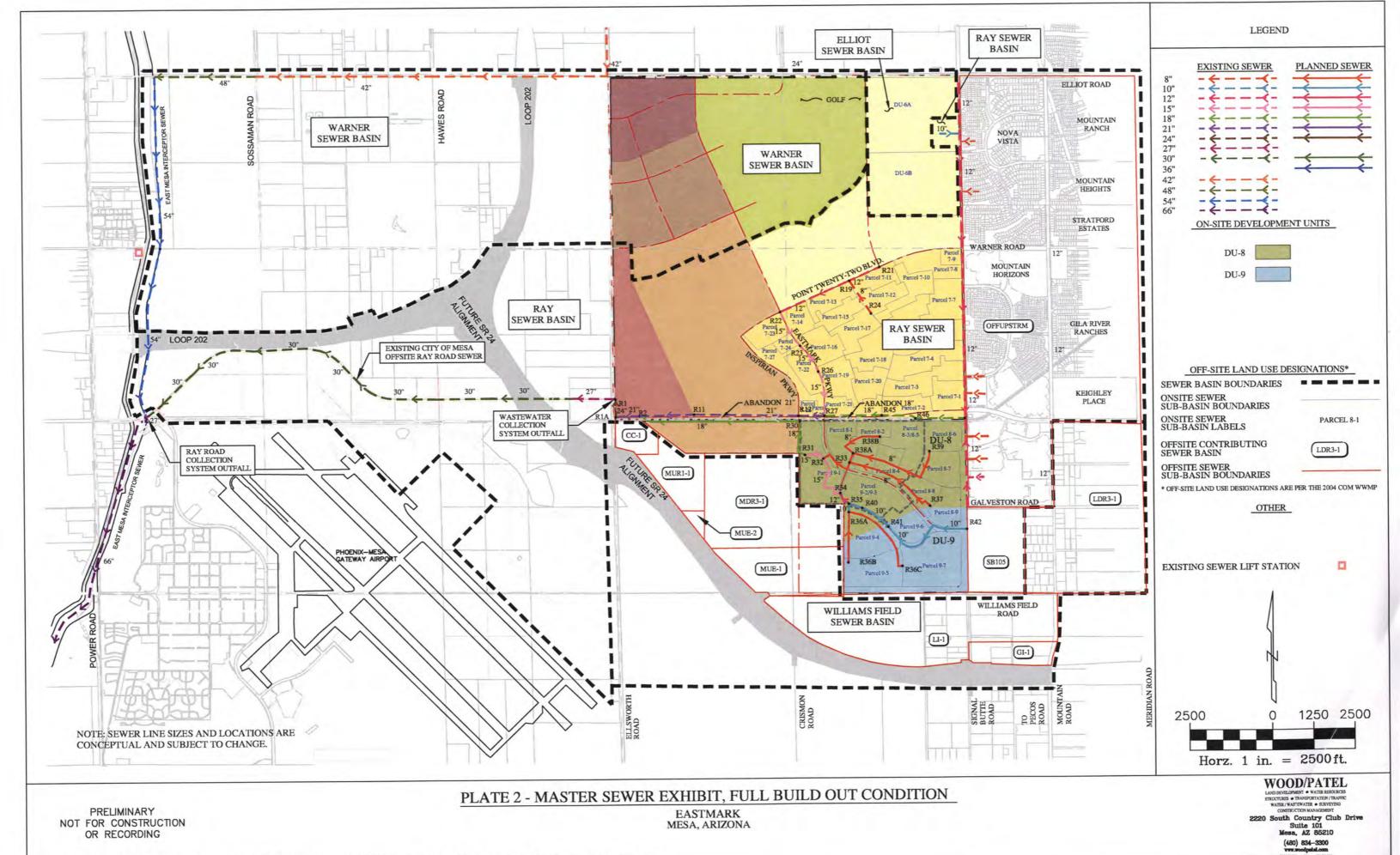


EASTMARK MESA, ARIZONA

OR RECORDING

PLATE 2

Master Sewer Exhibit, Full Build-Out Condition



R:\Mesa Proving Grounds\2012\23835.04\Project Support\Reports\DU 8 & 9 Master WasterHater Update\Exhibits\ 3835_04-Plate 2 - Master Sewer Exhibit - FBO.dwg - Jan 14,2014 2:59pm

PLATE 3

Master Sewer Exhibit, Phase 1

