

**MASTER DRAINAGE REPORT  
FOR  
DEVELOPMENT UNITS 8 & 9  
AT  
EASTMARK**

February 4, 2013  
WP# 123835

*Submitted to:*

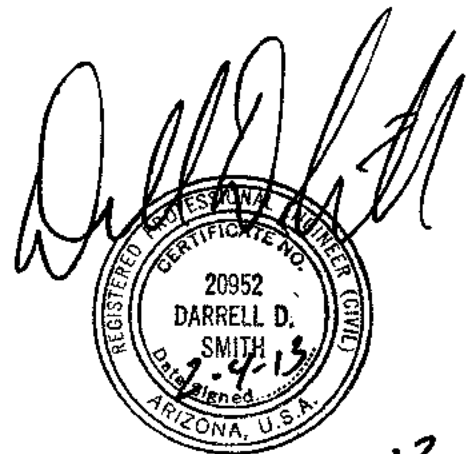
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*Expires 6-30-13*

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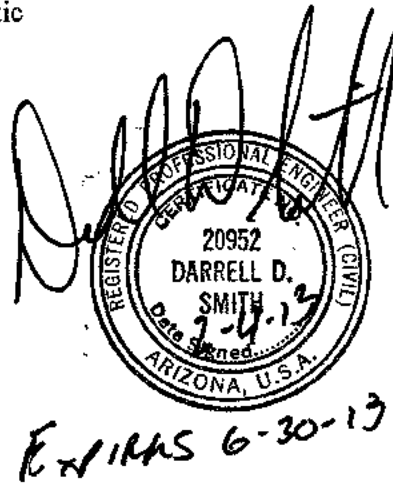
EXPIRES 6-30-13

## APPENDICES

- Appendix A Proposed Condition Data and Hydrology
- Hydrology Proposed Condition 100-year, 24-hour HEC-1 Output
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  - Post Developed HEC-1 Sub-Basin Data
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  - Post Developed HEC-1 Routing Data
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## PLATES

- Plate 1 Vicinity Map  
Plate 2 Soils Map  
Plate 3 Flood Insurance Rate Map  
Plate 4 Section 404 Jurisdictional Delineation Map  
Plate 5 Post Developed HEC-1 Schematic



## 1.0 INTRODUCTION

### 1.1 General Background and Project Location

Development Units 8 and 9 are located in the southern portion of the Eastmark development, formerly known as Mesa Proving Grounds. The proposed Development Units 8 and 9 are approximately 527-acres within the 3,155-acre Eastmark master planned community, in Mesa, Arizona. It is a Planned Community District (PCD) which will include residential, various community uses (club amenities), and open spaces.

This Master Drainage Report has been prepared in accordance with Wood, Patel & Associates, Inc. (Wood/Patel's) understanding of the City of Mesa (City) and the Flood Control District of Maricopa County (FCDMC) drainage requirements.

The Site is located within a portion of Section 26 of Township 1 South, Range 7 East of the Gila and Salt River Meridian. The Site is bounded by the Ray Road alignment to the north, Williams Field Road on the south, Signal Butte Road to the east, and Spine Road West on the west. Please refer to the attached Plate 1 – *Vicinity Map*.

The Site consists of multiple automotive test tracks and undisturbed desert. The Site was previously used by General Motors as a desert automobile testing facility. The majority of the Site is surrounded by automotive test tracks and undisturbed desert along the northern, western, and southern boundaries. Along the eastern boundary, the Site is bordered by two residential developments, Nova Vista and Mountain Horizons which are mostly constructed and Bella Via, which is currently under construction.

In addition, the Powerline Floodway Channel traverses the northern boundary of the Site south and parallel to the Ray Road alignment. This is a major FCDMC facility that provides conveyance of discharge from the Powerline Flood Retarding Structures, approximately three miles east of the Site, and drainage conveyance for stormwater runoff for areas adjacent to the channel. Ultimately, the flow is conveyed to the East Maricopa Floodway (EMF) west of the Site.

## **1.2 Scope of the DU 8 & 9 Master Drainage Report**

The DU 8 & 9 Master Drainage Report was prepared to support the development of approximately 20 acres of amenities and 1550 single family residential dwelling units. The drainage analysis is consistent with procedures and standards of the City of Mesa and the Flood Control District of Maricopa County. The proposed drainage plan provides an outline for the required major drainage facilities for storage and conveyance of stormwater runoff for the development of DU 8 & 9 at Eastmark. Updates to the Master Drainage Report may be required if significant changes are made to the land uses and assumptions utilized to prepare this report.

## 2.0 DESCRIPTION OF STUDY AREA

### 2.1 Existing Soil Conditions

According to the Natural Resources Conservation Service's Soil Survey, Eastmark is located within the Aguila-Carefree soil survey area. The majority of the surface soils onsite are classified as sandy loam, clay loam, or loam. Please refer to Plate 2 – *Soils Map*, and Appendix A for information pertaining to existing soil conditions.

### 2.2 Rainfall Seasons

There are two distinct rainfall seasons associated with the desert southwest corresponding to the project area. The first season occurs during the winter months from November to March when the area is subjected to occasional storms from the Pacific Ocean. While classified as a rainfall season, there can be long periods where there can be little or no precipitation. Generally storms occurring during the winter rainfall season are classified as being long duration, low intensity storms.

The second rainfall period occurs during the summer months of June through August, and is commonly referred to as Monsoon Season. During this season, Arizona is subjected to widespread thunderstorm activity whose moisture supply originates both in the Gulf of Mexico and along Mexico's west coast. These thunderstorms are typically classified as being short duration, high intensity storms with extreme variability per location.

### 2.3 FEMA Flood Insurance Rate Map (FIRM)

The Maricopa County, Arizona and Incorporated Areas Flood Insurance Rate Map (FIRM) Panel Number 04013CIND0A dated September 30, 2005, references Panel Number 04013C2705F, which indicates the Site is within Zone "D". Panel Number 04013C2705F has a note that states "Panel Not Printed – Area in Zone D".

Zone "D" is defined by FEMA as follows:

*"Areas in which flood hazards are undetermined."*

Please refer to Plate 3 – *Flood Insurance Rate Map* for illustration.

## **2.4 Section 404 Jurisdictional Areas**

A Jurisdictional Delineation has been completed by the Corps for the Mesa Proving Grounds. A portion of the Powerline Floodway channel and a small wash have been designated as Jurisdictional and fall within the DU 8 & 9 boundary. Please refer to Plate 4 – *Section 404 Jurisdictional Delineation Map* for location of Jurisdictional Areas.

Proposed disturbances to the Jurisdictional areas are required to be permitted with the Corps. A Section 404 Individual Permit will be required for disturbance during development with conditions that must be adhered to.

## **2.5 Master Drainage Report for Mesa Proving Grounds**

The Master Drainage Report for Mesa Proving Grounds, dated December 20, 2011 by Wood, Patel and Associates, Inc. has set the drainage criteria for the site. The report includes a pre-developed condition HEC-1 model (MGPEX.DAT) as well as a full build out model (20MPGDU7.DAT) which were modified versions of the current flood control district area drainage master plan models. The East Mesa Area Drainage Master Plan (ADMP), prepared in 1998 by Dibble & Associates, Inc. and Hoskin Ryan Consultants, Inc., is a regional drainage study prepared for the FCDMC. A revised update for the Master Report is being submitted for review and approval, concurrent with this DU Master Plan.

Eastmark is located in the eastern portion of the study, which is bound by the FRS in Pinal County to the east and the EMF to the west. In general, the area drains northeast to southwest and outlets into the EMF. The ADMP sets the regional drainage constraints for facilities within the study area of Eastmark. The full build out model was utilized to verify that the development of Eastmark does not negatively impact any drainage infrastructure downstream.

### 3.0 EXISTING DRAINAGE CONDITION

#### 3.1 Existing Drainage Plan

The Site generally slopes in a southwesterly direction at approximately 0.5 to 1 percent. The peak elevation within the Site is 1440 feet mean sea level (MSL), is located near the intersection of Signal Butte Road and Ray Road. The lowest elevation within the Site is approximately 1,410 feet MSL, located at Williams Field east of Crismon Road alignment (southwest corner of Site). The Site is covered with typical Sonoran Desert vegetation including mesquite trees, saguaro cactus, creosote, etc. is approximately 1,425 feet above MSL.

The existing site is made up of three sub-basins which discharge to the west into the Pacific Proving Grounds site, and has been modeled accordingly within the current 100-year, 24-hour FCDMC model and the Master Drainage Report model. The outfall west of the site discharges in locations of existing washes and the existing channel along the north side of Williams Field Road. Proposed 100-year, 2-hour retention will decrease the runoff volume and peak flow produced on Site.

##### 3.1.1 Northern Boundary

The northern boundary of DU 8 & 9 is bound by the Powerline Floodway. The floodway provides a low flow outlet to FRS dams upstream of the Site and also storm water conveyance for areas adjacent to the channel. The channel precludes storm water generated to the north from entering the Site.

##### 3.1.2 Eastern Boundary

Offsite flows impacting the eastern boundary from Ray Road to Williams Field Road are diverted south by existing berms and a channel along the west side of Signal Butte Road. There are two point impacts from the Mountain Horizons development. One is approximately 1,000 feet south of the Powerline Floodway, where offsite flows are conveyed through a double-barrel 10-foot by 3-foot box culvert. The second point impact is at Galveston Street, where offsite flows are conveyed through a double-barrel 8-foot by 3-foot box culvert. These combined flows are approximately 694 cfs. Between Galveston Street and Williams Field



Road, the upstream impact is from the undeveloped Pacific Proving Grounds, which discharges flow in washes and sheet flow of approximately 775 cfs. The flow continues westerly in a channel between the test track and Williams Field Road, and discharges to the Pacific Proving Grounds. There is an existing channel corridor on Pacific Proving Grounds that receives this flow in the existing condition, routes the flow around the property, and discharges to the Ellsworth Channel. A combined onsite and offsite flow of approximately 1,090 cfs discharges to the Pacific Proving Grounds in this location. In the post-developed condition, the onsite peak flow discharging to the channel is reduced due to onsite retention for the 100-year, 2-hour storm event. A HEC-RAS model, contained in Appendix C, shows this channel has proper capacity to receive the post-developed flow of approximately 946 cfs.

### **3.1.3 Western Boundary**

The western boundary is not impacted by any offsite flows entering the Site. There are several discharge points to Pacific Proving Grounds along this common boundary. In the existing conditions, the 4 discharges are approximately 90 cfs, 225 cfs, 156 cfs, and 1,090 cfs from north to south, respectively.

### **3.1.4 Southern Boundary**

There are no offsite impacts crossing the southern boundary along Williams Field Road.

## 4.0 PROPOSED DRAINAGE CONDITION

### 4.1 Proposed Drainage Plan

The drainage concept for DU 8 & 9 is to route offsite flows around the site and direct onsite storm water runoff to retention basins for storage. Offsite runoff impacting the northern boundary will continue to be picked up by the Powerline Floodway, preventing any storm water produced to the north from entering the Site.

Onsite runoff produced onsite will utilize roadways for overland flow conveyance to localized retention basins. Where street capacities are exceeded, vertical curb and/or underground storm drain systems or roadside channels may be utilized to convey the excess runoff volume. Please refer to Plate 5 – *Proposed Condition HEC-1 Schematic* for watershed delineations and locations.

Retention basins will be sized to retain the runoff volume from a 100-year, 2-hour storm event in accordance with jurisdictional requirements. Emergency overflow routes must be provided in the event that retention basin capacities are exceeded due to a storm larger than the design event or back-to-back storms. Retention basins will be designed to drain retained runoff within 36 hours after a storm event. Land uses depicted in the hydrologic models are proposed and subject to change.

In all locations, lowest floor elevations shall be set a minimum of 1 foot above the emergency overflow elevation or any 100-year water surface elevation adjacent the Site, whichever is greater.

### 4.2 Proposed Condition Hydrology

A proposed condition HEC-1 model (EMDU89.DAT) was created to assess the impact of the developed DU 8 & 9 to the downstream drainage infrastructure. The model was created based upon the most current post developed condition model. Watersheds within the Eastmark DU 8 & 9 were modeled with medium density residential, active open space and general transportation land uses per the FCDMC's DDMSW program.

Retention for DU 8 & 9 was calculated based on the previously mentioned land uses and applied to the proposed condition HEC-1 model. Based on point precipitation frequency

estimates from NOAA Atlas 14, the 100-year, 2-hour precipitation is 2.19 inches. Flows in excess of the 100-year, 2-hour basin capacities within DU 8 & 9 were routed downstream to historic outfall locations.

Offsite routing parameters along Signal Butte Road and Williams Field Road were updated to reflect future channels planned to convey offsite flows along the boundary. The following table provides a summary of 100-year, 24-hour discharges for the existing and proposed conditions which indicates decreased peak discharges leaving the Site.

EXISTING CONDITION		PROPOSED CONDITION	
Location ID	Discharge	Location ID	Discharge
C79B1	1090 cfs	CP22B	939 cfs
C79B2	417 cfs	CP19B	126 cfs
C79A	1726 cfs	CP79A1	1093 cfs

### 4.3 Proposed Hydraulics

#### 4.3.1 Street Hydraulics

Arterials and major collectors shall be designed to convey the peak flows generated by a 10-year peak storm within the roadway infrastructure with a spread limited to 1 traffic lane in each direction. All other public roadways shall be designed to convey the peak flows generated by a 10-year peak storm between the curbs. All roadways shall be designed to convey the 100-year storm within the right-of-way and adjacent parkway. Where the peak flows exceed the capacity of the public street to convey the peak flows, storm drains or other drainage facilities shall be installed and sized to carry the excess flows (i.e., when the 10-year peak exceeds the spread criteria or exceeds the curb capacity of the public street, or when the right-of-way cannot convey the 100-year peak). Storm drain and/or channel systems will convey stormwater runoff to retention basins located throughout the Site.

### 4.4 Retention

#### 4.4.1 Retention Storage

The 100-year, 2-hour required retention volume for DU 8 & 9 was estimated to be 65.5 acre-feet based on conceptual land use. If actual land uses and required

retention volumes vary from this report updates to this report may be required to analyze impacts to downstream drainage infrastructure.

See *Table 5 – Proposed Condition Onsite Retention Volume Summary* in Appendix A for a detailed summary of required retention volumes per applicable watershed. Proposed DU 8 & 9 volumes are based on a 100-year, 2-hour precipitation depth of 2.19 inches obtained from NOAA Atlas 14 Precipitation Frequency Data. Retention basins will be required to dissipate stormwater within 36 hours.

Proposed channels adjacent to Signal Butte Road and Williams Field Road shall convey stormwater runoff produced within the roadway rights-of-way, combined with offsite flows east of the Site to drainage facilities downstream of Mesa Proving Grounds. The flow impacting the Site north of Ray Road will be conveyed in a channel southerly along Signal Butte and combined with the Powerline Floodway, then conveyed through the Site and discharged at Ray and Ellsworth Roads, as it does in the historical condition. The flow impacting the Site south of Ray Road will be conveyed in a channel southerly along Signal Butte, then westerly along Williams Field Road, and discharged to the existing channel on Pacific Proving Grounds, as it does in the historical condition.

Retention basins will be designed to drain after the storm event within 36 hours. Please refer to *Table 5 – Proposed Condition Onsite Retention Volume Summary* in Appendix B for detailed calculations of the retention volumes.

#### **4.4.2 Stormwater Quality**

The required retention storage volume for the Site exceeds the first flush requirement of storing the first one-half inch of runoff. All runoff will have settlement time within retention basins prior to draining by percolation, drywells, release into natural watercourses, and/or release into existing storm drain systems.

#### **4.5 Maintenance**

Ongoing maintenance of the designed or recommended drainage systems will be required to preserve the design integrity and purpose of the drainage system. Failure to provide

maintenance can prevent the drainage system from performing to its intended design purpose, and can result in reduced performance. Maintenance is the responsibility of private developers and owners associations for facilities on private property within all easements and private streets, except for drainage structures within public rights-of-way accepted by the City of Mesa for maintenance. Ownership and maintenance responsibilities will be associated with developments discharging to retention facilities and will be managed by the owners associations established for the Site. A regular maintenance program is required to have drainage systems perform to the level of protection or service as presented in this report.

## 5.0 CONCLUSIONS

Based on the analysis of the *Master Drainage Report for Development Units 8 & 9 at Mesa Proving Grounds*, the following conclusions can be made:

1. This *Master Drainage Report for Development Units 8 & 9 at Eastmark* is prepared in accordance with Wood, Patel & Associates, Inc.'s understanding of the drainage parameters set by the Flood Control District of Maricopa County, the City of Mesa, and the *Master Drainage Report for Eastmark*.
2. Offsite flows shall be conveyed around the Site adequately per jurisdictional requirements. FCDMC may require a review of this *DU8&9 Master Drainage Report*.
3. Peak flows for the proposed condition 100-year, 24-hour storm shall not negatively impact downstream drainage infrastructure.
4. Onsite retention shall be provided to retain runoff generated by the 100-year, 2-hour storm event for developed areas.
6. Flow in excess of onsite storage capacity shall outfall to emergency overflow routes.
7. Lowest floor elevations shall be set a minimum of 1 foot above the adjacent 100-year water surface elevation or emergency outfall water surface elevation, whichever is greater.
8. Drainage infrastructure will be designed in accordance with the appropriate criteria per the City of Mesa and/or Flood Control District of Maricopa County.
9. Ongoing maintenance is required for all drainage systems in order to assure design performance.

## 6.0 REFERENCES

1. *Master Drainage Report for Mesa Proving Grounds*, Wood, Patel and Associates, Inc., September 15, 2011.
2. *Drainage Design Manual for Maricopa County, Arizona, Volumes 1 and 2*, Flood Control District of Maricopa County, 2011.
3. *Drainage Policies and Standards for Maricopa County, Arizona*, Flood Control District of Maricopa County, July, 2010.
4. *2009 Engineering & Design Standards*, City of Mesa, 2009.
5. *Flood Insurance Rate Map*, Federal Emergency Management Agency (FEMA), September 30, 2005.
6. *HEC-1 Flood Hydrograph Package*, US Army Corps of Engineers, June 1998.
7. *FlowMaster 2005*, Haestad Methods, Inc., 2004.

**APPENDIX A**  
**Proposed Condition Data and Hydrology**



**Hydrology Proposed Condition**  
**100-Year, 24-Hour HEC-1 Output**

EMD089.txt

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*   VERSION 4.1
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* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*****

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1OB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.

THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION.

NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS WRITE STAGE FREQUENCY, DSS READ TIME SERIES AT DESIRED CALCULATION INTERVAL, LOSS RATE, GREEN AND AMPT INFILTRATION

KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1 HEC-1 INPUT PAGE 1

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13 ID
14 ID THIS MODEL IS AN EXERPT OF THE MODEL PROVIDED BY THE FLOOD CONTROL
15 ID DISTRICT OF MARICOPA COUNTY (MS4-SEM.DAT). ONSITE WATERSHEDS WERE
16 ID UPDATED TO REFLECT CURRENT PLAN FOR DEVELOPMENT UNITS 8 & 9.
17 ID
18 ID MODEL REVISED BY:
19 ID WOOD, PATEL & ASSOCIATES, INC.
20 ID DARREN E. SMITH, P.E.
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47 ID SITE.
48 ID
49 ID MODEL REVISED BY:
50 ID WOOD, PATEL & ASSOCIATES, INC.
51 ID DANIEL W. MATTHEWS, E.I.T.
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1 HEC-1 INPUT PAGE 2

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79 ID 02B. LAND USE WAS CHANGED TO INDUSTRIAL FOR 02B AND ENTIRELEY  
80 ID RESIDENTIAL FOR 02A.  
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82 ID  
83 ID MODEL REVISED BY:  
84 ID WOOD, PATEL & ASSOCIATES, INC.  
85 ID STEPHEN M. SCINTIO, P.E.  
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115 ID WOOD, PATEL & ASSOCIATES, INC.  
116 ID DANIEL W. MATTHEWS, E.I.T.  
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141 ID AS REQUESTED BY FLOOD CONTROL DISTRICT OF MARICOPA COUNTY TO REDUCE THE  
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143 ID WITHIN THE EAST MESA ADMP.  
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147 ID DANIEL W. MATTHEWS, E.I.T.  
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EMD089.txt
PLAN\2ND SUBMITTAL\POST-DEVELOPED 100YR2HR RETENTION MODEL (MPG20RT2)\
MPG20RT2.DAT
.....
FILE: MPG20RT2.DAT
MODEL REVISED: 01-08-08
PROJECT: MESA PROVING GROUNDS
MODEL REVISION DESCRIPTION:

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HEC-1 INPUT
LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
166 ID THIS MODEL SHOULD REPLACE WS4-SEM.DAT IN THE HEC-1 RUN SEQUENCE SPECIFIE
167 ID BELOW. REFERENCING WS2-NEM.DSS IS STILL REQUIRED.
168 ID
169 ID
170 ID THIS IS A 100-YEAR, 2-HOUR RETENIION SCENARIO MODEL USING
171 ID THE 20MSF COMMERCIAL SPACE AND 15K DU LAND PLAN PROVIDED
172 ID BY SWABACK PARTNERS ON 12/12/07.
173 ID
174 ID
175 ID THIS MODEL IS AN EXERPTI OF THE MODEL PROVIDED BY THE FLOOD CONTROL
176 ID DISTRICT OF MARICOPA COUNTY (WS4-SEM.DAT). WATERSHEDS 68A, 68B,
177 ID 70A, 70B, 71, 73B, 73C, 74B, 74C, 75, 77B, 77C, 78B, 78C, AND 79A
178 ID HAVE ALL BEEN UPDATED TO REFLECT CURRENT WATERSHED DELINEATIONS,
179 ID NEW DEVELOPMENT, CURRENT RETENTION, AND FLOOD ROUTING. BASIN 75
180 ID HAS BEEN UPDATED TO REFLECT PLANNED DEVELOPEMENT FOR THE MESA
181 ID PROVING GROUNDS SITE.
182 ID
183 ID MODEL REVISED BY:
184 ID WOOD, PATEL & ASSOCIATES, INC.
185 ID DANIEL W. MATTHEWS, E.I.T.
186 ID
187 ID FILE PATH:
188 ID R:\MESA PROVING GROUNDS\2006\062753\PROJECT SUPPORT\HYDRO\MDR-20-15 LAND
189 ID PLAN\HYDROLOGY\POST-DEVELOPED 100YR2HR RETENTION MODEL (MPG20RT2)\
190 ID MPG20RT2.DAT
191 ID
192 ID .....
193 ID
194 ID
195 ID ID Kirkham Michael:
196 ID Last Revised Date: 1/22/03
197 ID Filename: WS4-SEM.DAT
198 ID
199 ID Comments Dated 1/22/03 (CJ)
200 ID
201 ID This model should be used ONLY for the Rittenhouse and Chandler Heights
202 ID Basin Design Project - Final Design Analyses.
203 ID
204 ID This model is one of several models that represent the ENF watershed.
205 ID This model covers the Southeast Mesa Area and should reference as a DSS
206 ID the watershed model for the Northeast Mesa Area (Filename WS2-NEM.DAT).
207 ID
208 ID This model is necessary to determine the input hydrographs for the
209 ID Rittenhouse Basin Design HEC-RAS Unsteady State analysis. To develop
210 ID the necessary input hydrographs the following models should be run in order.
211 ID Because the files utilize a TAPE21 file to export import hydrographs
212 ID between models, prior to running the FIRST model (WS1-NHM.DAT) any existing
213 ID TAPE21 file in the directory should be deleted. The run procedure order is:
214 ID
215 ID 1) WS1-NHM.DAT
216 ID 2) WS2-NEM.DAT
217 ID 3) WS3-QCSW.DAT
218 ID 4) WS4-SEM.DAT (referencing WS2-NEM.DSS for the DSS file)
219 ID 5) RT1-BASE.DAT
220 ID

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HEC-1 INPUT
LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
221 ID The necessary input hydrographs for the Rittenhouse Basin analysis
222 ID are determined in RT1-BASE. In that output file, the hydrograph at
223 ID RMFLD1 should be exported and used as the input hydrograph at the
224 ID ENF Reach 4 Cross Section 17.002. And the hydrograph at RITTEN should
225 ID be exported and used as the input hydrograph for the Rittenhouse Main
226 ID Channel at Cross Section 820.00
227 ID
228 ID
229 ID
230 ID *****
231 ID **** NOTE BY PRIMATECH ENGINEERS: ****
232 ID **** DATE: 06/12/2001 ****
233 ID **** THE NEW FILE NAME IS: SEBTALT2.DAT ****
234 ID **** THE FILE WAS RENAMED AS <<RTBTALT2.DAT>> FOR THE EAST MARICOPA ****
235 ID **** FLOODWAY CAPACITY MITIGATION PROJECT, BY FLOOD CONTROL DISTRICT OF ****
236 ID **** MARICOPA COUNTY. ****
237 ID **** THE FILE WAS RENAMED <<RTBTALT3.DAT>> AND UPDATED USING GREEN AND ****
238 ID **** AMPF FUTURE CONDITIONS FOR BASINS 258 TO 268. ****
239 ID *****
240 ID
241 ID
242 ID THIS MODEL WAS ORIGINALLY MIDDCUT.DAT
243 ID IT HAS BEEN MODIFIED BY CPE (7/2000)

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EMH089.txt
244 ID FOR ALTERNATIVE 2 FOR THE EAST MARICOPA FLOWWAY
245 ID CAPACITY MITIGATION AND MULTI-USE CORRIDOR STUDY
246 ID TO ROUTE BOTH THE POWERLINE FLOWWAY
247 ID AND THE SANTAN FREEWAY CHANNEL INTO THE RAY BASIN PRIOR THEIR OUTFALL
248 ID INTO THE EMF
249 ID
250 ID
251 ID
252 ID
253 ID Model files changed by Collins/Pina Engineering
254 ID to reflect multi-use design concepts (recreation
255 ID and environment) proposed throughout the entire
256 ID EMF Corridor. July 2000
257 ID
258 ID VERSION 8.06 CPE 7/31/00
259 ID
260 ID
261 ID
262 ID
263 ID
264 ID FILENAME: MIDDOUT.DAT
265 ID
266 ID ALL CIP INFRASTRUCTURE IS IN PLACE, FUTURE CONDITIONS LANDUSE IS IN PLACE
267 ID FLOW IS ROUTED UP ELLSWORTH ROAD IN A EARTH LINED CHANNEL
268 ID
269 ID
270 ID PRODUCED BY DIBBLE AND ASSOCIATES AND ROSKIN ENGINEERING CONSULTANTS.
271 ID
272 ID File Name: Final8.dat
273 ID Revised - Jan. 2000 by SZ (Wood/Patel) from Final7.dat - new 2-V & Sidewalk
274 ID Revised - Jan. 2000 by SZ (Wood/Patel) from Final6.dat - 60% review comments
275 ID Revised - Dec. 1999 by SZ (Wood/Patel) from Final5.dat
276 ID Revised - Dec. 1999 by SZ (Wood/Patel) from Final4.dat

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
276 ID Revised - Nov. 1999 by SZ (Wood/Patel) from Final3.dat
277 ID Revised - June 1999 by SZ (Wood/Patel) for Final Model from Opt1.dat.
278 ID Revised - May 1999 by SZ (Wood/Patel) for Option 1, Based on Model SDIB.DAT
279 ID REVISID - MAY, 1999 BY VAS TO INCORPORATE INCREASE OF SUBBASIN RETENTION AND
280 ID REVISIONS TO THE REGIONAL DETENTION BASIN STORAGE
281 ID REVISID - FEB, 1999 BY VALERIE SWICK, FCD OF MARICOPA COUNTY
282 ID REVISID - MAY, 1998 BY D&A
283 ID
284 ID REVISID BY VALERIE SWICK, FEB. 26, 1998
285 ID
286 ID
287 ID FLOWS FROM DETENTION BASIN LOCATED AT NE CORNER OF ELLIOT AND ELLSWORTH ROADS
288 ID IS ROUTED TO THE SOUTHWEST BY SIPHON DRAW TO SUBBASIN 70A. FROM THERE THEY
289 ID WILL BE ROUTED BY A CHANNEL TO THE EMF. FLOWS FROM SUBBASINS ADJACENT TO
290 ID SANTAN FREEWAY ALIGNMENT WILL BE ROUTED SOUTH TO SUBBASIN 70A WHERE THEY WILL
291 ID BE COMBINED WITH FLOW IN SIPHON DRAW.
292 ID
293 ID EAST MESA AREA DRAINAGE MASTER PLAN
294 ID AREA SOUTH OF SUPERSTITION (U.S. HWY 60)
295 ID AUGUST 1997
296 ID SOUTHEAST MESA HIGH RESOLUTION MODEL
297 ID
298 ID *****FUTURE CONDITION MODEL OF THE WATERSHED*****
299 ID
300 ID *****ATTENTION*****
301 ID SUBBASINS 75, 79A, 79B, 78E, LANDUSES WERE NOT
302 ID CHANGED BECAUSE IT WAS FELT THAT THEIR FUTURE CONDITIONS LANDUSES WOULD BE
303 ID SIMILAR TO THE EXISTING CONDITIONS LANDUSES.
304 ID RETENTION VOLUMES WILL ALSO NOT BE UTILIZED FOR SUBBASINS 75, 79A, 79B, 78E
305 ID SOME QUEEN CREEK SUBBASINS WILL ALSO NOT HAVE RETENTION VOLUMES, EITHER
306 ID BECAUSE THEY LIE IN PINAL COUNTY AND WE DONT KNOW PINAL COUNTIES PLANS OR
307 ID THEY LIE IN THE SANTAN MOUNTAINS AND WON'T GET DEVELOPED
308 ID WILLIAMS GATEWAY AIRPORT (SUBBASINS 80A, 80B, 81A, AND 81B) ARE MODELED AS
309 ID FUTURE CONDITIONS AND HAVE RETENTION VOLUMES FOR THE 100YR 2HR STORM
310 ID
311 ID FILENAME: SDIBS.DAT
312 ID
313 ID THIS MODEL REPRESENTS THE FUTURE CONDITION OF THE WATERSHED.
314 ID TOTAL DRAINAGE AREA IS APPROXIMATELY 213 SQ. MI.
315 ID THIS MODEL USES A Kn VALUE OF 0.09 FOR DESERT LAND USE DUE TO SHEET FLOW
316 ID CONDITIONS.
317 ID
318 ID 100-YEAR 24-HOUR FREQUENCY
319 ID AREAL REDUCTIONS FROM FCD HYDROLOGY MANUAL
320 ID THIS MODEL INCLUDES INFLOW FROM NORTH OF THE SUPERSTITION FREEWAY
321 ID AND EAST OF THE CAP
322 ID
323 ID DATA FROM THE QUEEN CREEK ADMS HAS BEEN ADDED TO CALCULATE FLOWS INTO THE
324 ID EMF. MUSKINGUM ROUTING NESTEPS WERE ADJUSTED TO BE WITHIN THE SUGGESTED
325 ID RANGE.
326 ID
327 ID METHODOLOGY
328 ID THE US CORPS OF ENGINEERS FLOOD HYDROLOGY MODEL HEC-1 DATED SEP1990 VER 4.0
329 ID SCS TYPE II RAINFALL DISTRIBUTION
330 ID S-GRAPH HYDROGRAPH
331 ID GREEN AND AMPT INFILTRATION EQUATION USED FOR CALCULATING LOSSES
332 ID
333 ID HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
331 ID NORMAL DEPTH STORAGE CHANNEL ROUTING
332 ID APPROXIMATE DIRECTION, LOCATION, AND LENGTH OF THE WASHES HAVE BEEN
333 ID EVALUATED BASED ON FIELD INVESTIGATION, USGS MAPS, LANDIS AERIAL SURVEYS
334 ID DATED 1994

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EMDU89.txt

335 ID THE NOAA TECHNICAL MEMORANDUM NOAA ATLAS 2 DEPTH AREA RATIOS  
 336 ID  
 337 ID ORIGINAL STUDY PERFORMED BY LISA C. YOUNG AND AFSHIN AKOURAIYAN, UPDATED BY  
 338 ID DAVID DEGERNESS (OCT-DEC, 1996). REVIEWED BY VALERIE A. SWICK  
 339 ID AND AMIR MOTAMEDI OF THE FLOOD CONTROL DISTRICT  
 340 ID HYDROLOGY BRANCH ENGINEERING DIVISION, FLOOD CONTROL  
 341 ID DISTRICT OF MARICOPA COUNTY, DECEMBER - JULY 1995.  
 342 ID  
 343 ID ASSUMED VELOCITY OF 1 FT/SEC FOR SHEET FLOW, 2-3 FT/SEC FOR WASH/NATURAL  
 344 ID CHANNEL, 3 FT/SEC FOR ROAD AND GRASS CHANNEL, 10FT/SEC FOR CONCRETE CHANNEL  
 345 ID  
 346 ID VELOCITIES FOR ADMP IMPROVEMENT CHANNELS FROM DIBBLE AND ASSOCIATES  
 347 ID SUGGESTED ALTERNATIVES (JULY 1, 1997)  
 348 ID  
 349 ID  
 350 ID \*\*\*\*\*  
 351 ID \*\*\*\* THE FOLLOWING NOTE WAS ADDED BY PRIMATECH ENGINEERS ON 06-12-2001 \*\*\*\*  
 352 ID \*\*\*\*\*  
 353 ID NOTE: MUST USE NEBUILD.DSS AS THE DSS FILE TO IMPORT FLOWS ACROSS THE  
 354 ID SUPERSTITION FREEWAY.  
 355 ID \*\*\*\*\*  
 356 ID  
 357 ID NOTE: MUST USE NDIBF.DSS AS THE DSS FILE TO IMPORT FLOWS ACROSS THE  
 358 ID SUPERSTITION FREEWAY.  
 359 ID \*\*\*\*\*  
 360 ID  
 361 ID DDM MCVH2 SE MESA ADMP - SOUTH OF SUPERSTITION FWY, FUTURE CONDITIONS

DIAGRAM

II	5	1APR97	0000	600						
362	IO	5								
363	IN	15								
364	JD	3.60	0.01							
365	PC	.000	.002	.005	.008	.011	.014	.017	.020	.026
366	PC	.029	.032	.035	.038	.041	.044	.048	.052	.060
367	PC	.064	.068	.072	.076	.080	.085	.090	.095	.105
368	PC	.110	.115	.120	.126	.133	.140	.147	.155	.172
369	PC	.181	.191	.203	.218	.236	.257	.283	.387	.707
370	PC	.735	.758	.776	.791	.804	.815	.825	.834	.842
371	PC	.856	.863	.869	.875	.881	.887	.893	.898	.908
372	PC	.913	.918	.922	.926	.930	.934	.938	.942	.950
373	PC	.953	.956	.959	.962	.965	.968	.971	.974	.977
374	PC	.983	.986	.989	.992	.995	.998	1.000		
375	JD	3.58	1.0							
376	JD	3.49	5.0							
377	JD	3.38	10.0							
378	JD	3.24	30.0							
379	JD	3.10	60.0							
380	JD	3.05	90.0							
381	JD	3.00	120.0							
382	JD	2.97	150.0							

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

383 KK S0SS  
 384 KM INFLOW FROM SOSSAMAN BASIN VIA SOSSAMAN CHANNEL  
 385 KM QI CARDS ARE BASED ON THE PEAK OF 1800CFS TO SOSSAMAN CHANNEL  
 386 BA 12.50  
 387 ZR -QI A-SOSSAMAN DRAIN B-AT SUPERSTITION C-FLOW E-SWIN F-100YR  
 \*

388 KK RS0SS  
 389 KM ROUTE FLOWS VIA SOSSAMAN CHANNEL TO BASELINE ROAD  
 390 RS 1 FLOW -1  
 391 RC .030 .025 .030 3500 .005  
 392 RX 0 5 10 35 75 110 115 120  
 393 RY 10 10 10 4 4 10 10 10  
 \*

394 KK 59A  
 395 KM BASIN 59A  
 396 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 397 KM L= .9 Lca= .3 S= 34.9 Km= .070 LAG= 29.7  
 398 KM PHOENIX VALLEY S-GRADE WAS USED FOR THIS BASIN  
 399 BA .26  
 400 LG .23 .25 4.55 .42 33.00  
 401 UI 30. 77. 144. 186. 246. 364. 293. 226. 172. 123.  
 402 UI 64. 48. 30. 15. 9. 9. 9. 0. 0. 0.  
 403 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 \*

404 KK R59A  
 405 KM RETAIN THE 100 YEAR 2 HOUR RUNOFF VOLUME  
 406 DT D59A 2  
 407 DI 0 10000  
 408 DQ 0 10000  
 \*

409 KK C59A  
 410 KM SOSSAMAN DRAIN AT BASELINE ROAD  
 411 HC 2  
 \*

412 KK 59A59B  
 413 KM ROUTE 559A TO 59B VIA SOSSAMAN CHANNEL  
 414 KM BLOCK WALL ON LEFT BANK, SOSSAMAN ROAD ON RIGHT BANK  
 415 RS 2 FLOW -1  
 416 RC .025 .018 .013 6500 .0015  
 417 RX 0 3 13 38 78 103 128 203  
 418 RY 16 10 10 0 0 10 8 10

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

419 KK 59B  
 420 KM BASIN 59B  
 421 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 422 KM L= 1.2 Lca= .7 S= 33.9 Kp=.087 LAG= 58.3  
 423 KM PHOENIX VALLEY S-GRADE WAS USED FOR THIS BASIN  
 424 BA 2.30  
 425 LC .24 .24 4.65 .41 24.00  
 426 UI 54. 54. 93. 193. 249. 284. 318. 361. 415. 501.  
 427 UI 653. 666. 546. 473. 422. 364. 318. 273. 233. 172.  
 428 UI 110. 94. 89. 68. 54. 54. 18. 17. 17. 17.  
 429 UI 17. 17. 17. 17. 0. 0. 0. 0. 0. 0.  
 430 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

431 KK R59B  
 432 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 433 DT D59B 63  
 434 DI 0 10000  
 435 DQ 0 10000

436 KK C59B  
 437 KM SOSSAMAN CHANNEL AT GUADALUPE ROAD  
 438 HC 2

439 KK 59B60  
 440 KM ROUTE 59B TO 60 GUADALUPE CHANNEL.  
 441 RS 6 FLOW -1  
 442 RC .02 .013 .02 5500 .0005  
 443 RX 0 318 522 522 560 560 580 2580  
 444 RY 8.5 8.5 8.5 0 0 8 7 6

445 KK 60  
 446 KM BASIN 60  
 447 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 448 KM L= 2.4 Lca= 1.4 S= 31.8 Kp=.087 LAG= 102.0  
 449 KM PHOENIX VALLEY S-GRADE WAS USED FOR THIS BASIN  
 450 BA 2.30  
 451 LC .18 .24 4.65 .43 35.00  
 452 UI 76. 76. 76. 76. 130. 250. 286. 330. 364. 395.  
 453 UI 422. 452. 484. 522. 571. 607. 689. 820. 915. 1008.  
 454 UI 885. 793. 723. 669. 626. 591. 536. 496. 459. 424.  
 455 UI 387. 362. 324. 276. 219. 169. 134. 134. 126. 125.  
 456 UI 121. 76. 76. 76. 76. 56. 23. 23. 23. 23.  
 457 UI 23. 23. 23. 23. 23. 23. 23. 23. 23. 23.  
 458 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 459 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

460 KK R60  
 461 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 462 DT D60 170  
 463 DI 0 10000  
 464 DQ 0 10000

465 KK EMFGJA  
 466 KM COMBINE 559 AND 560 AT EMF, GUADALUPE ROAD  
 467 KO 21  
 468 HC 2

469 KK GUATEL  
 470 KM ROUTE EMF FLOW FROM GUADALUPE ROAD TO ELLIOT ROAD  
 471 RS 3 FLOW -1  
 472 RC .03 .022 .03 6000 .0003  
 473 RX 0 500 520 553 693 726 740 742  
 474 RY 14 12 11 0 0 11 11 12

475 KK 64  
 476 KM BASIN 64  
 477 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 478 KM L= 1.2 Lca= .6 S= 25.4 Kn= .051 LAG= 34.4  
 479 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 480 BA .81  
 481 LG .18 .25 4.70 .41 54.00  
 482 UI 79. 155. 338. 438. 543. 709. 988. 778. 624. 493.  
 483 UI 388. 253. 139. 120. 79. 45. 24. 24. 24.  
 484 UI 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 485 UI 0. 0. 0. 0. 0. 0. 0. 0. 0.

486 KK R64  
 487 KM R64 IS WHAT REMAINS AFTER THE DIVERSION OF FLOW UP TO 67 AC-FT. THIS IS SENT  
 488 KM TO TAPE 21 FOR RECALL INTO FCD'S EMF MODELS. KK BLOCK THERE MUST BE UPDATED  
 489 KM TO REFLECT THE CHANGE OF WHAT GETS SENT TO THE TAPE 21.  
 490 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 491 KO 21  
 492 DT D64 67  
 493 DI 0 10000  
 494 DQ 0 10000

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

495 KK EMFELL  
 496 KM COMBINE EMF FLOW WITH FLOW FROM SUBBASIN 64 AT ELLIOT ROAD  
 497 HC 2

498 KK ELTWAR  
 499 KM ROUTE EMF FLOW AT ELLIOT ROAD TO WARNER ROAD VIA THE EMF  
 500 RS 4 FLOW -1  
 501 HC .03 .022 .03 5500 .0003  
 502 RX 0 500 520 553 693 726 740 742  
 503 RY 14 12 11 0 0 11 11 12

504 KK 62A  
 505 KM BASIN 62A  
 506 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 507 KM L= .8 Lca= .5 S= 30.0 Kn= .020 LAG= 10.2  
 508 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 509 BA .38  
 510 LG .18 .25 4.50 .52 80.00  
 511 UI 335. 1057. 1010. 367. 93. 38. 0. 0. 0. 0.  
 512 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

513 KK R62A  
 514 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 515 DT D62A 33  
 516 DI 0 10000  
 517 DQ 0 10000

518 KK 62ATB  
 519 KM ROUTE 62A TO 62B BY A CHANNEL ALONG BASELINE ROAD THROUGH SANTAN FWY  
 520 RS 1 FLOW -1  
 521 RC .030 .030 .030 1200 .002  
 522 RX 0 200 250 275 295 320 370 570  
 523 RY 10 8 7 0 0 7 8 10

524 KK 62B  
 525 KM BASIN 62B  
 526 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 527 KM L= .6 Lca= .3 S= 47.5 Kn= .021 LAG= 8.0  
 528 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 529 BA .23  
 530 LG .11 .25 4.65 .46 76.00  
 531 UI 334. 940. 431. 83. 0. 0. 0. 0. 0.  
 532 UI 0. 0. 0. 0. 0. 0. 0. 0. 0.

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

533 KK R62B  
 534 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 535 DT D62B 19  
 536 DI 0 10000  
 537 DQ 0 10000

538 KK C62B



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EMD089.txt
539 KM COMBINE FLOW FROM BASIN 62A AND 62B
540 HC 2
*
*
541 KK 62BTD
542 KM ROUTE 62B TO 62D VIA HAWES ROAD
543 RS 5 FLOW -1
544 RC .045 .04 .045 5230 .0041
545 RX 0 100 125 127 177 179 224 324
546 RY 3 2 1.50 0 0 1.5 2 3
*
*
547 KK 62D
548 KM BASIN 62D
549 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
550 NM L= .9 Lca= .3 S= 30.7 Kn= .045 LAG= 21.3
551 NM PHOENIX MOUNTAIN S-GRAH WAS USED FOR THIS BASIN
552 BA .46
553 LG .23 .25 4.65 .40 50.00
554 UI 76. 300. 519. 753. 475. 369. 286. 203. 163. 111.
555 UI 85. 63. 47. 36. 26. 14. 14. 14. 14. 0.
556 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
557 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
*
*
558 KK R62D
559 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
560 DT D62D 35
561 DI 0 10000
562 DQ 0 10000
*
*
563 KK CP62D
564 KM COMBINE FLOWS FROM SUBBASINS 62B AND 62D
565 HC 2
*

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REC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
566 KK 62DIF
567 KM ROUTE 62D TO 62F VIA HAWES ROAD
568 RS 9 FLOW -1
569 RC .045 .024 .045 3600 .0033
570 RX 0 500 750 753 793
571 RY 3 1.5 1.25 0 0 1.25 1.5 3
*
*
572 KK 62F
573 KM BASIN 62F
574 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
575 NM L= .6 Lca= .4 S= 31.9 Kn= .042 LAG= 18.1
576 NM PHOENIX VALLEY S-GRAH WAS USED FOR THIS BASIN
577 BA .26
578 LG .21 .25 4.65 .41 54.00
579 UI 66. 224. 350. 546. 371. 235. 98. 56. 18. 15.
580 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
581 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
*
*
582 KK R62F
583 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
584 DT D62F 18
585 DI 0 10000
586 DQ 0 10000
*
*
587 KK CP62F
588 KM COMBINE FLOWS FROM 62D AND 62F
589 HC 2
*
*
590 KK 62T63
591 KM ROUTE CP62F TO SUBBASIN 63 VIA WASH.
592 KM WASH CROSSES HAWES, NORTH OF ELLIOT
593 RS 4 FLOW -1
594 RC .045 .04 .045 6000 0.0055
595 RX 0 500 750 770 780 800 1050 1550
596 RY 5 4 3 0 0 3 4 5
*
*
597 KK 63
598 KM BASIN 63
599 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
600 NM L= 1.4 Lca= .7 S= 20.2 Kn= .035 LAG= 26.8
601 NM PHOENIX VALLEY S-GRAH WAS USED FOR THIS BASIN
602 BA .91
603 LG .18 .25 4.65 .43 55.00

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

605 UI 170. 113. 35. 35. 35. 35. 0. 0. 0. 0.  
 606 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

607 KK R63  
 608 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 609 DI D63 71  
 610 DI 0 10000  
 611 DQ 0 10000

612 KK CP63  
 613 KM COMBINE FLOWS FROM SUBBASIN 63 AND CP62F  
 614 KC 2

615 KK 63I71  
 616 KM ROUTE CP63 TO S71 VIA SHEET FLOW  
 617 KM SOSSAMAN SOUTH OF ELLIOT  
 618 RS 11 FLOW -1  
 619 RC .055 .045 .055 5280 .0005  
 620 RX 0 1000 1005 1010 1013 1043 1543 2043  
 621 RY 6 5 0 0 3 5 2 5

622 KK 68B1 BASIN  
 623 KM BASIN 68B1  
 624 NM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 625 NM L=0.75 Lca=0.38 S=32.3 Kn=0.030 LAG=13.9  
 626 NM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 627 BA 0.146  
 628 LG 0.10 0.25 5.20 0.36 80  
 629 UI 68 206 374 269 135 48 17 11 0 0  
 630 UI 0 0 0 0 0 0 0 0 0 0

631 KK 68B2 BASIN  
 632 KM BASIN 68B2  
 633 NM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 634 NM L=0.55 Lca=0.28 S=32.4 Kn=0.030 LAG=11.0  
 635 NM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 636 BA 0.060  
 637 LG 0.10 0.25 5.20 0.36 80  
 638 UI 45 141 173 74 23 6 0 0 0 0  
 639 UI 0 0 0 0 0 0 0 0 0 0

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

640 KK 68B3 BASIN  
 641 KM BASIN 68B3  
 642 NM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 643 NM L=0.36 Lca=0.18 S=32.2 Kn=0.030 LAG=7.9  
 644 NM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 645 BA 0.036  
 646 LG 0.10 0.25 5.20 0.36 80  
 647 UI 52 148 63 12 0 0 0 0 0 0  
 648 UI 0 0 0 0 0 0 0 0 0 0

649 KK CP68  
 650 KM COMBINE FLOWS FROM BASINS 68B1, 68B2, AND 68B3  
 651 KC 3

652 KK R68  
 653 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 654 DI D68B 24  
 655 DI 0 10000  
 656 DQ 0 10000

657 KK 68B169  
 658 NM ROUTE S68B TO S69 VIA WASH CROSSING HAWES  
 659 RS 4 FLOW -1  
 660 RC .045 .04 .045 2750 .0036  
 661 RX 0 500 950 1003 1007 1057 1511 2011  
 662 RY 4 3.5 3 0 0 2 2.5 3

663 KK 69  
 664 NM BASIN 69  
 665 NM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 666 NM L= .7 Lca= .3 S= 22.4 Kn= .020 LAG= 9.0

EMDU99.txt  
 PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN

667	KM									
668	BA	.05								
669	LG	.10	.25	4.76	.45	80.00				
670	UI	104.	320.	213.	54.	11.	0.	0.	0.	0.
671	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.

R69  
 RETAIN 100 YR 2 HR RUNOFF VOLUME

672	KK	R69
673	KM	RETAIN 100 YR 2 HR RUNOFF VOLUME
674	DT	D65 9
675	DI	0 10000
676	DQ	0 10000

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

C69  
 COMBINE FLOWS FROM SUBBASIN 68B AND 69

69T71  
 ROUTE S69 TO S71 VIA WASH AND SHEET FLOW, INCREASE OVERBANK N VALUES

680	KK	69T71
681	KM	ROUTE S69 TO S71 VIA WASH AND SHEET FLOW, INCREASE OVERBANK N VALUES
682	RS	11 FLOW -1
683	RC	.055 .045 .055 6000 .0033
684	RK	0 500 1000 1001 1002 1500 2000 2500
685	RY	4 3 2 0 0 2 3 4

25 BASIN  
 THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 L=0.90 Lca=0.41 S=16.7 Kn=0.035 LAG=20.2  
 PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN

686	KK	25 BASIN
687	KM	BASIN 25
688	KM	THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
689	KM	L=0.90 Lca=0.41 S=16.7 Kn=0.035 LAG=20.2
690	KM	PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
691	BA	0.205
692	LG	0.10 0.25 5.00 0.39 80
693	UI	40 151 225 371 334 221 139 60 37 11
694	UI	11 10 0 0 0 0 0 0 0 0
695	UI	0 0 0 0 0 0 0 0 0 0
696	UI	0 0 0 0 0 0 0 0 0 0
697	UI	0 0 0 0 0 0 0 0 0 0

RET25 DIVERT  
 RETAIN 100 YR 2 HR RUNOFF VOLUME

698	KK	RET25 DIVERT
699	KM	RETAIN 100 YR 2 HR RUNOFF VOLUME
700	DT	D25RET 0
701	DI	0.0 100.0 1000.0 10000.0 0.0 0.0 0.0 0.0 0.0 0.0
702	DQ	0.0 100.0 1000.0 10000.0 0.0 0.0 0.0 0.0 0.0 0.0

25T71 ROUTE  
 ROUTE BASIN 25 TO BASIN 71 VIA WASH AND SHEET FLOW

703	KK	25T71 ROUTE
704	KM	ROUTE BASIN 25 TO BASIN 71 VIA WASH AND SHEET FLOW
705	RS	11 FLOW -1
706	RC	0.045 0.040 0.045 5686 0.0050 0.00
707	RK	0.00 500.00 1000.00 1003.00 1007.00 1011.00 1511.00 2011.00
708	RY	3.00 2.50 2.00 0.00 0.00 2.00 2.50 3.00

71  
 BASIN 71  
 THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 L= 1.6 Lca=.8 S= 26.4 Kn=.020 LAG= 16.8  
 PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 BASIN AREA UPDATED FROM 1.09 TO 0.861 BECAUSE AREA FOR BASIN 25 WAS REMOVED  
 FROM BASIN 71

709	KK	71
710	KM	BASIN 71
711	KM	THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
712	KM	L= 1.6 Lca=.8 S= 26.4 Kn=.020 LAG= 16.8
713	KM	PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
714	KM	BASIN AREA UPDATED FROM 1.09 TO 0.861 BECAUSE AREA FOR BASIN 25 WAS REMOVED
715	KM	FROM BASIN 71
716	BA	0.861
717	LG	.10 .25 4.65 .47 80.00
718	UI	331. 1085. 1805. 2349. 1459. 780. 329. 144. 67. 67.
719	UI	0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

R71  
 RETAIN 100 YR 2 HR RUNOFF VOLUME  
 RETENTION VOLUME WAS REDUCED FROM 106 AC-FT TO 84.1 AC-FT BECAUSE 21.9 AC-FT  
 WAS ACCOUNTED FOR IN RET25.

721	KK	R71
722	KM	RETAIN 100 YR 2 HR RUNOFF VOLUME
723	KM	RETENTION VOLUME WAS REDUCED FROM 106 AC-FT TO 84.1 AC-FT BECAUSE 21.9 AC-FT
724	KM	WAS ACCOUNTED FOR IN RET25.
725	KM	
726	DT	D71 84.1
727	DI	0 10000
728	DQ	0 10000

C71  
 COMBINE FLOWS FROM 63T71, 69T71, BASIN 71, AND BASIN 25  
 CONCENTRATION POINT IS ALONG SOSSAMAN AT THE MESQUITE ST ALIGNMENT

729	KK	C71
730	KM	COMBINE FLOWS FROM 63T71, 69T71, BASIN 71, AND BASIN 25
731	KM	CONCENTRATION POINT IS ALONG SOSSAMAN AT THE MESQUITE ST ALIGNMENT
732	HC	4

EMDU89.txt

733 KK 71T72  
 734 KM ROUTE C71 TO S72 VIA DIKE  
 735 KM WASH WEST OF INTERSECTION OF SOSSAMAN & WARNER  
 736 RS 8 FLOW -1  
 737 RC .055 .045 .055 3750 .0037  
 738 RK 0 500 1000 1007 1017 1025 1530 2030  
 739 RY 9 8.5 8 0 0 8 8.5 9  
 \*

740 KK 72  
 741 KM BASIN 72  
 742 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 743 KM L= 1.6 Lca= .9 S= 13.1 Kn= .020 LAG= 20.3  
 744 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 745 BA .84  
 746 LG .10 .25 5.40 .33 80.00  
 747 UI 161. 600. 906. 1496. 1347. 912. 566. 247. 153. 50.  
 748 UI 43. 43. 0. 0. 0. 0. 0. 0. 0. 0.  
 749 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 \*

750 KK R72  
 751 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 752 DT D72 83  
 753 DI 0 10000  
 754 DQ 0 10000  
 \*  
 \* CONCENTRATION POINT ADDED PRIOR TO EMF COMBINE SO THAT FLOWS CAN BE SENT TO  
 \* TAPE21.  
 \*

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1  
 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

755 KK CPRNOX  
 756 KO 21  
 757 HC 2  
 \*

758 KK EMEWAR  
 759 KM COMBINE ROUTED FLOW FROM 71 WITH 72 WITH EMF (HYDROGRAPH ELTHAR)  
 760 HC 2  
 \*

761 KK WARTKN  
 762 KM ROUTE EMF WARNER ROAD FLOW TO KNOX ROAD  
 763 RS 2 FLOW -1  
 764 RC .03 .022 .03 2500 .0003  
 765 RX 0 500 520 553 693 726 740 742  
 766 RY 14 12 11 0 0 11 11 12  
 \*

767 KK 26 BASIN  
 768 KM BASIN 26  
 769 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 770 KM L=0.38 Lca=0.19 S=23.7 Kn=0.035 LAG=10.2  
 771 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 772 BA 0.045  
 773 LG 0.10 0.25 4.90 0.41 80  
 774 UI 40 127 122 44 11 4 0 0 0 0  
 775 UI 0 0 0 0 0 0 0 0 0 0  
 \*

776 KK RET26 DIVERT  
 777 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 778 DT 26RET 4.8 0.0  
 779 DI 0 10000  
 780 DQ 0 10000  
 \*

781 KK 26T70B ROUTE  
 782 KM ROUTE BASIN 26 TO BASIN 70B VIA WASH AND SHEET FLOW  
 783 RS 6 FLOW -1  
 784 RC 0.045 0.040 0.045 4688 0.0057 0.60  
 785 RX 0.00 500.00 1000.00 1003.00 1007.00 1011.00 1511.00 2011.00  
 786 RY 3.00 2.50 2.00 0.00 0.00 2.00 2.50 3.00  
 \*

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1  
 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

787 KK 70B  
 788 KM BASIN 70B  
 789 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 790 KM L= 1.6 Lca= 1.1 S= 29.8 Kn= .022 LAG= 20.7  
 791 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 792 KM BASIN AREA UPDATED FROM 0.38 TO 0.335 BECAUSE AREA FOR BASIN 26 WAS REMOVED  
 793 KM FROM BASIN 70B

EMDU89.LXC

794 BA .335  
 795 LG .11 .15 8.00 .12 76.00  
 796 UI 68. 259. 390. 636. 615. 418. 267. 119. 73. 30.  
 797 UI 19. 19. 0. 0. 0. 0. 0. 0. 0. 0.  
 798 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 \*  
 \*

799 KK R70B  
 800 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 801 KM RETENTION VOLUME WAS REDUCED FROM 38 AC-FT TO 33.2 AC-FT BECAUSE 4.8 AC-FT  
 802 KM WAS ACCOUNTED FOR IN RET26.  
 803 DI D70B 33.2  
 804 DI 0 10000  
 805 DQ 0 10000  
 \*  
 \*

806 KK CP70B  
 807 KM COMBINE ROUTED FLOW FROM 26 AND 70B  
 808 HC 2  
 \*  
 \*

809 KK 70BT76  
 810 KM ROUTE 70B TO 76B VIA WASH CROSSING SOSSAMAN, SOUTH OF WARNER ROAD  
 811 RS 11 FLOW -1  
 812 RC .045 .04 .045 5500 .0041  
 813 RK 0 500 1000 1003 1007 1011 1511 2011  
 814 RY 4 3.5 3 0 0 2 2.5 3  
 \*

815 KK 76B  
 816 KM BASIN 76B  
 817 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 818 KM L= 1.8 Lca= .9 S= 27.4 Kn= .021 LAG= 10.9  
 819 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 820 BA .64  
 821 LG .10 .15 8.80 .09 78.00  
 822 UI 148. 515. 789. 1294. 957. 629. 303. 157. 70. 35.  
 823 UI 35. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 824 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 \*  
 \*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

825 KK R76B  
 826 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 827 DI D76B 66  
 828 DI 0 10000  
 829 DQ 0 10000  
 \*  
 \*

830 KK KNOX  
 831 KM COMBINE FLOWS AT KNOX ROAD  
 832 NO 21  
 833 HC 2  
 \*  
 \*

834 KK EMFKNX  
 835 KM COMBINE FLOWS INTO THE EMF AT KNOX ROAD  
 836 KM THIS COMBINES HYDROGRAPHS WAKIK, 70BT76 and R76B.  
 837 HC 2  
 \*  
 \*

838 KK KXTRY  
 839 KM ROUTE EMF KNOX ROAD FLOW TO RAY ROAD  
 840 RS 2 FLOW -1  
 841 RC .03 .022 .03 3000 .0003  
 842 RK 0 500 520 553 693 726 740 742  
 843 RY 14 12 11 0 0 11 11 12  
 \*  
 \*

844 KK 65A  
 845 KM BASIN 65A  
 846 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 847 KM L= 1.6 Lca= .9 S= 51.2 Kn= .053 LAG= 41.5  
 848 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 849 BA 2.54  
 850 LG .15 .25 5.10 .35 59.00  
 851 UI 206. 248. 755. 1001. 1188. 1423. 1760. 2487. 2309. 1835.  
 852 UI 1546. 1269. 1035. 777. 457. 351. 292. 206. 143. 63.  
 853 UI 63. 63. 63. 63. 0. 0. 0. 0. 0. 0.  
 854 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 \*  
 \*

855 KK R65A  
 856 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 857 DI D65A 174  
 858 DI 0 10000  
 859 DQ 0 10000  
 \*  
 \*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

860 KK CAPIA
861 KM INFLOW FROM EAST OF THE CAP THROUGH 2 - 72" PIPE OVERCHUTES
862 KM STATION #131+90 AND 158+00 SALT-GILA AQUEDUCT REACH 2
863 KM QI CARDS BASED ON PEAK OUTFLOW FROM OVERCHUTES OF 217 CFS.
864 BA 6.4
865 ZR -QI A=CAPIA B-OVERCHUTE C-FLOW E-5MIN F=100YEAR
* IN 60
* BA .01
* QI 0 65 217 217 217 217 217 217 217 217 2
* QI 217 217 217 217 217 217 217 217 217 217 2
* QI 217 217 217 217 217
*
*
866 KK RCAP1A
867 KM ROUTE FLOW FROM CAP OVERCHUTE TO A POINT ON THE MARICOPA/FINAL COUNTY LINE
868 KM 2000 FEET NORTH OF THE GUADALUPE ROAD COUNTY LINE INTERSECTION. ROUTING WILL
869 KM BE BY A NATURAL CHANNEL. THIS IS THEN ROUTED FOR 1200 FT
870 KM IN A CHANNEL (DIBBLE ID MN3) TO THE POINT WHERE THE ROUTED CAP1B FLOW
871 KM INTERCEPTS THE CHANNEL. ORIGINAL SLOPE = .01
872 RS 3 FLOW -1
873 RC .045 .04 .045 4900 .010
874 RK 0 500 1000 1006 1026 1032 1511 2011
875 RY 4 3.5 3 0 0 3 3.5 4
*
*
876 KK RRCAP1A
877 KM REACH MN-5 AND CULVERT MNC-1
878 KM ROUTE FLOW FROM WHERE RCAP1A FLOWS INTO THE NEW CHANNEL ALONG MERIDIAN ROAD
879 KM USES REVISED ROUTING PARAMETERS, CHANNEL MN-5 SHAPE
880 RS 1 FLOW -1
881 RC 0.025 0.015 0.025 2350 .0017
882 RK 0 8 16 27 43 53 61 69
883 RY 5.1 5.2 5.3 0 0 5.3 5.2 5.1
*
*
884 KK CAPIB
885 KM INFLOW FROM EAST OF THE CAP THROUGH 2 - 72" PIPE OVERCHUTES
886 KM STATION #131+90 AND 158+00 SALT-GILA AQUEDUCT REACH 2
887 KM QI CARDS BASED ON PEAK OUTFLOW FROM OVERCHUTES OF 217 CFS.
888 BA 6.4
889 ZW -QI A=CAPIB B-OVERCHUTE C-FLOW E-5MIN F=100YEAR
* IN 60
* BA .01
* QI 0 65 217 217 217 217 217 217 217 217 2
* QI 217 217 217 217 217 217 217 217 217 217 2
* QI 217 217 217 217 217
*
*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

890 KK RCAP1B
891 KM ROUTE FLOW FROM CAPIB OVERCHUTE TO A POINT ALONG THE MARICOPA/FINAL COUNTY
892 KM LINE 1000 FEET NORTH OF THE INTERSECTION OF GUADALUPE ROAD AND THE COUNTY
893 KM LINE. ROUTING WILL BE BY A NATURAL CHANNEL. ORIGINAL SLOPE=.01
894 RS 1 FLOW -1
895 RC .045 .04 .045 4900 .010
896 RK 0 500 1000 1006 1026 1032 1511 2011
897 RY 4 3.5 3 0 0 3 3.5 4
*
*
898 KK C65A1
899 KM COMBINE FLOWS FROM SUBBASIN 65A(EAST OF MERIDIAN RD) AND CAPIA AND CAPIB
900 HC 3
*
*
901 KK C6SATB1
902 KM REACH MN-4, MN-3B AND MN-3A
903 KM ROUTE FLOW FROM BASIN 65A TO OFF-LINE DETENTION BASIN DIVERSION STRUCTURE
904 KM USES REVISED ROUTING PARAMETERS, CHANNEL MN-3A SHAPE.
905 RS 1 FLOW -1
906 RC 0.025 0.015 0.025 3760 .0015
907 RK 0.0 8.0 14.0 34 56 74 82 90
908 RY 8.8 8.9 9.0 0 0 9.0 8.9 8.8
*
*
909 KK D1DB65
910 KM DETENTION BASIN DIVERSION STRUCTURE
911 KM DIVERT FLOW FROM CHANNEL TO OFF-LINE BASIN
* KO 2
DI DB65A
912 DI 0 100 434 582 854 1206 1624 2096 3188
913 DI 0 0 434 582 854 1206 1624 2096 3188
914 DQ 0 0 148 420 772 1190 1662 2754
*
*
915 KK C65B1T2
916 KM REACH MN-2

```

EMD089.txt

917	KM	ROUTE FLOW FROM BASIN DIVERSION STRUCTURE TO BASIN OUTLET									
918	RS	1	FLOW	-1	0						
919	RC	.025	.015	.025	700	.0019					
920	RX	0.0	8.0	16.0	25	35	44	52	60		
921	RY	8.7	8.8	4.5	0	0	4.5	8.8	8.7		

922 KK DIB65P  
 923 KM RETURN DIVERT TO DETENTION BASIN  
 924 DR DB65A

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

925 KK DB65A  
 926 KM SIPHON DRAW BASIN  
 927 KM OFF-LINE DETENTION BASIN LOCATED AT ELLIOT AND MERIDIAN ROAD  
 928 KM WITH 36" OUTLET PIPE AND 235' WEIR  
 \* KO 2  
 929 RS 1 STOR 0  
 930 SA 0 4.47 15.05 27.12 37.88 42.12  
 931 SE 90 91 93 95 97 98  
 932 SL 91.5 7.07 .62 .5  
 933 SF 97.0 235 2.5 1.5

934 KK C65A2  
 935 KM RECOMBINE FLOWS FROM DETENTION BASIN AND SUNLAND SPRINGS CHANNEL.  
 936 HC 2

937 KK 65AT-1  
 938 KM REACH MN-1  
 939 KM ROUTE FROM DETENTION BASIN OUTLET TO ELLIOT ROAD AND MERIDIAN ROAD  
 940 KM CONCRETE CHANNEL ON EAST SIDE OF MERIDIAN ROAD ALGN.  
 941 RS 1 FLOW -1  
 942 RC .025 .015 .025 630 .0014  
 943 RX 0 8 16 26 38 48 56 64  
 944 RY 4.7 4.8 4.9 0 0 4.9 4.8 4.7

945 KK 65AT-2  
 946 KM REACH ET-10, ET-11, ET-12 plus culverts ETC-4 AND ETC-3.  
 947 KM ROUTE FROM ELLIOT AND MERIDIAN, ALONG ELLIOT ROAD IN ELLIOT CHANNEL  
 948 KM TO ABOUT 0.6 MILES EAST OF CRISMON ROAD.  
 949 KM EARTH CHANNEL PORTION ON NORTH SIDE OF ELLIOT  
 \* KO 1  
 950 RS 4 FLOW -1  
 951 RC .025 .025 .025 7680 .0005  
 952 RX 0 8 22 58 71 107 115 123  
 953 RY 6.1 6.0 5.9 0 0 5.9 6.0 6.1

954 KK 65AW  
 955 KM BASIN 65AW  
 956 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 957 KM L= .9 Lea= .6 S= 54.7 Km= .049 LAG= 26.1  
 958 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 959 BA .43  
 960 LG .24 .25 5.30 .29 32.00  
 961 UI 56. 176. 295. 391. 603. 594. 432. 316. 210. 101.  
 962 UI 73. 43. 17. 17. 17. 0. 0. 0. 0. 0.  
 963 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

964 KK R65AW  
 965 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 966 DT D65AW 31  
 967 DI 0 10000  
 968 DC 0 10000

969 KK 65AT65  
 970 KM ROUTE C65A TO BASIN 65B VIA A WASH. (THIS WASH IS WORTH OF SIPHON DRAW)  
 971 KM THIS IS THE PART OF 65A WHICH IS WEST OF THE MERIDIAN RD ALIGNMENT  
 \* KO 3  
 972 RS 11 FLOW -1  
 973 RC .045 .04 .045 9500 .007  
 974 RX 0 500 1000 1003 1053 1056 1511 2011  
 975 RY 4 3.5 3 0 0 2 2.5 3

976 KK 65B  
 977 KM BASIN 65B

EMDUS9.txt

978 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 979 KM L= 2.0 Lca= 1.2 S= 37.5 Kn=.036 LAG= 36.6  
 980 KM PHOENIX VALLEY S-CRAPH WAS USED FOR THIS BASIN  
 981 BA 1.37  
 982 LG .18 .25 6.00 .24 53.00  
 983 UI 126. 218. 506. 669. 809. 1014. 1468. 1422. 1102. 901.  
 984 UI 720. 562. 337. 218. 182. 126. 71. 39. 39. 39.  
 985 UI 39. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 986 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

987 KK D65B  
 988 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 989 DT D65B 120  
 990 DI 0 10000  
 991 DQ 0 10000  
 \*  
 \*  
 992 KK CP65B  
 \* KO 1  
 993 KM COMBINE FLOW FROM SUBBASIN 65AW (WEST OF MERIDIAN RD) WITH FLOW FROM  
 994 KM SUBBASIN 65B  
 995 HC 2  
 \*  
 \*

1

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

996 KK DI65B  
 997 KM DIVERSION STRUCTURE TO ROUTE PEAK FLOW TO NEW  
 998 KM ELLIOT BASIN (EAST)  
 999 KM By-pass 30 cfs to Elliot Channel, and Divert Remaining to E Basin  
 \* The Existing Model By-pass 51 cfs. SZ, 5-17-99  
 1000 DT DIRS65  
 1001 DI 0 15.0 30 100.0 200 350.0 500 700 900 1500.  
 1002 DQ 0 0 0 70.0 170 320.0 470 670 870 1470.  
 \*  
 \*

1003 KK CP65A  
 1004 KM COMBINE FLOWS FROM ELLIOT CHANNEL AT NON DIVERTED FLOWS (51CFS) FROM  
 1005 KM SUBBASIN 65B  
 1006 HC 2  
 \*  
 \*

1007 KK 65AT-3  
 \* KM REACH ET-9 plus culvert ETC-2  
 \* KM ROUTE FROM ABOUT 0.6 MILES EAST OF CRISMON ROAD  
 \* KM TO ABOUT 0.18 MILES EAST OF CRISMON ROAD (CP65)  
 \* KM CONCRETE CHANNEL PORTION ON SOUTH SIDE OF ELLIOT  
 \* RD 2340 0.0019 0.013 CIRC 7.5  
 1008 KM E. 104th St to E. of EA (Sta. 83+10 to Sta. 90+50)  
 \* RD card used for routing  
 1009 RD 740 0.0064 0.012 CIRC 6.5  
 \*  
 \*

1010 KK DR65B  
 \* KO 1  
 1011 KM RETURN DIVERT TO EAST DETENTION BASIN  
 1012 DR DIRS65  
 \*  
 \*

1013 KK RS65A  
 1014 KM ELLIOT BASIN, EAST  
 1015 KM Bleed-off Pipe Size - 12", SZ, 5-17-99  
 \* Since the bleed-off pipe length is short, no routing is provided.  
 \* The Existing Pipe Size - 24"  
 \* KO 1  
 1016 RS 1 \$TOR 0  
 1017 SV 0 5.40 9.30 13.90 18.80 24.00 29.50 35.30 41.40 48.00  
 1018 SE 1429.0 1433.0 1434.0 1435.0 1436.0 1437.0 1438.0 1439.0 1440.0 1441.0  
 1019 SL 1430.0 0.7854 .62 .5  
 1020 SS 1439.0 200 2.5 1.5  
 \*  
 \*

1

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1021 KK CP65  
 \* "CP65B" changed to "CP65" -- SZ, 5-14, 1999  
 1022 KM COMBINE FLOWS FROM EAST ELLIOT BASIN AND ELLIOT CHANNEL  
 1023 KM BEFORE COMBINING WITH FLOWS FROM THE BYPASS CRISMON CHANNEL  
 \* KO 1  
 1024 HC 2  
 \*  
 \*

1025 KK 65T66  
 \* KM REACH ET-8  
 \* KM ROUTING IN ELLIOT CHANNEL FROM ABOUT



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```

* KM 0.18 MILES EAST OF CRISMON ROAD (CP65) TO CRISMON ROAD (CP66).
* Moved from "65AT-3" to here
1026 * E. of EA to W. of EA (Sta. 76+06 to Sta. 83+10)
* RD card used for routing
* RD 940 0.0060 0.013 CIRC 9.5
1027 * RD 704 0.0064 0.012 CIRC 7.5
*
*
1028 KK 65T66A
1029 KM W. of EA to E. of Crismon Rd. (Sta. 69+00 to Sta. 76+06)
* New additional routing operation
* RD card used for routing
1030 RD 706 0.0047 0.012 CIRC 7.5
*
*
1031 KK 65T66B
1032 KM E. of Crismon Rd. to W. of Crismon Rd. (Sta. 61+25 to Sta. 69+00)
* New additional routing operation
* RD card used for routing
1033 RD 775 0.0048 0.012 CIRC 9.5
*
*
1034 KK ADOT-E
1035 KM INFLOW FROM NORTH OF THE SUPERSTITION FREEWAY ENTERING 67A
1036 KM FROM EAST ADOT DETENTION BASIN 4105.
* KO 1
1037 BA 0.01
1038 SR -QI A-ADOT EAST BASIN B-AT SUPERSTITION C-FLOW E-5MIN F-100YR
*
*
1039 KK AET67A
1040 KM ROUTE SUPERSTITION FLOW THROUGH 67A TO BASELINE ROAD
1041 IN 15
1042 RS 3 FLOW -1
1043 RC .045 .040 .045 5500 .010
1044 RX 0 100 110 120 130 140 150 250
1045 RY 5 4 3 1 1 3 4 5
*
*

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HEC-1 INPUT

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LINE ID .....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1046 KK 67A
1047 KM BASIN 67A
1048 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
1049 KM L= 1.0 Lea= .7 S= 42.9 Kn= .042 LAG= 25.7
1050 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
1051 BA -.30
1052 LG -.21 .25 4.70 .39 43.00
1053 DI 39. 126. 208. 277. 433. 400. 292. 213. 134. 67.
1054 DI 47. 25. 12. 12. 12. 0. 0. 0. 0. 0.
1055 DI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
*
*
1056 KK R67A
1057 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
1058 DI D67A 21
1059 DI 0 10000
1060 DQ 0 10000
*
*
1061 KK C67A
1062 KM COMBINE FLOWS FROM ADOT-E AND SUBBASIN 67A
1063 KC 2
*
*
1064 KK 67ATC
1065 KM ROUTE 67A TO 67C VIA WASH CROSSING BASELINE
1066 RS 4 FLOW -1
1067 RC .055 .045 .055 6300 .0071
1068 RX 0 500 980 1003 1007 1031 1511 2011
1069 RY 4 3.5 3 0 0 3 3.5 4
*
*
1070 KK SUP2
1071 KM INFLOW FROM NORTH OF SUPERSTITION FREEWAY, DISCHARGING INTO 67B
* KO 1
1072 BA 0.01
1073 SR -QI A-ADOT WEST BASIN B-AT SUPERSTITION C-FLOW E-5MIN F-100YR
*
*
1074 KK RSUP2
1075 KM ROUTE SUP2 THROUGH SUBBASIN 67B
* KO 2
1076 IN 15
1077 RS 11 FLOW -1
1078 RC .045 .045 .045 4500 .0056
1079 RX 0 500 1000 1003 1007 1011 1511 2011

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1080 RY 4 3.5 3 0 0 2 2.5 3  
 \*  
 \*

## HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1081 KK 67B  
 1082 KM BASIN 67B  
 1083 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 1084 KM L= 1.2 Lca= .9 S= 28.0 Kn= .034 LAG= 26.4  
 1085 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 1086 BA .53  
 1087 LG .17 .25 4.90 .38 56.00  
 1088 UI 68. 210. 356. 470. 713. 741. 536. 395. 269. 131.  
 1089 UI 94. 59. 21. 21. 21. 0. 0. 0. 0. 0.  
 1090 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 \*  
 \*

1091 KK R67B  
 1092 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 1093 DT D67B 41  
 1094 DI 0 10000  
 1095 DQ 0 10000  
 \*  
 \*

1096 KK C67B  
 1097 KM COMBINE FLOWS FROM SUP2 AND SUBBASIN 67B  
 \* KO 2  
 1098 HC 2  
 \*  
 \*

1099 KK 67BIC  
 1100 KM REACH CN-4, CN-5 plus culvert CNC-4.  
 1101 KM ROUTE FLOW IN THE CRISMON CHANNEL FROM BASELINE ROAD (C67B) TO  
 1102 KM GUADALUPE ROAD (C67C)  
 1103 RS 2 FLOW -1  
 1104 RC .025 .015 .025 5180 .0019  
 1105 RX 0 8 16 24.4 36.4 44.8 52.8 60.8  
 1106 RY 4.0 4.1 4.2 0 0 4.2 4.1 4.0  
 \*  
 \*

1107 KK 67C  
 1108 KM BASIN 67C  
 1109 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 1110 KM L= 1.2 Lca= .7 S= 40.2 Kn= .049 LAG= 32.3  
 1111 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 1112 BA .93  
 1113 LG .25 .25 5.10 .32 31.00  
 1114 UI 96. 213. 432. 557. 702. 1006. 1133. 842. 667. 518.  
 1115 UI 365. 193. 157. 96. 59. 30. 30. 30. 30. 0.  
 1116 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 1117 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 \*  
 \*

## HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1118 KK R67C  
 1119 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 1120 DT D67C 67  
 1121 DI 0 10000  
 1122 DQ 0 10000  
 \*  
 \*

1123 KK C67C  
 1124 KM COMBINE SUBBASINS 67C AND 67A AND 67B  
 1125 HC 3  
 \*  
 \*

1126 KK 67CT67  
 1127 KM REACH CN-3 plus culvert CNC-3  
 1128 KM ROUTE FLOW IN THE CRISMON CHANNEL FROM C67C (8 GUADALUPE ROAD & CRISMON ROAD)  
 1129 KM TO C67D (AT APPROX. 1/2 MILE SOUTH OF GUADALUPE ROAD).  
 Sta. 39+00 to Guadalupe Rd.  
 1130 RS 1 FLOW -1  
 1131 RC .025 .015 .025 2420 .0018  
 1132 RX 0 6 12 24 64 76 82 88  
 1133 RY 4 3 2 0 0 2 3 4  
 \*  
 \*

1134 KK 67D  
 1135 KM BASIN 67D  
 1136 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 1137 KM L= .6 Lca= .4 S= 34.7 Kn= .050 LAG= 20.5  
 1138 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 1139 BA .13  
 1140 LG .25 .25 5.20 .30 30.00  
 1141 UI 23. 87. 152. 216. 202. 137. 86. 38. 23. 9.  
 \*  
 \*

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1142 UI 6. 6. 0. 0. 0. 0. 0. 0. 0. 0.
1143 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
*
*
1144 KK R67D
1145 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
1146 DI D67D 9
1147 DI 0 10000
1148 DQ 0 10000
*
*
1149 KK C67D
1150 KM COMBINE HYDROGRAPHS AT CP67D
1151 HC 2
*
*

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HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1152 KK 67DI66
1153 KM REACH CN-2 plus culvert CNC-2
1154 KM ROUTE FLOW IN THE CRISMON CHANNEL FROM APPROX. 1/2 MILE SOUTH
1155 KM OF GUADALUPE ROAD TO THE INLEW SPILLWAY FOR THE ELLIOT DETENTION BASIN.
*
* Sta. 20+00 to Sta. 39+00
1156 RS 1 FLOW -1
1157 RC .032 .032 .032 1900 0.0035
1158 RX 0 6 12 24 64 76 82 88
1159 RY 4 3 2 0 0 2 3 4
*
*
1160 KK 66A
1161 KM BASIN 66A
1162 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
1163 KM L= .7 Lca= .3 S= 55.9 Kn= .047 LAG= 17.1
1164 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
1165 BA .26
1166 LG .25 6.00 .22 35.00
1167 UI 78. 256. 417. 576. 363. 205. 84. 39. 16. 16.
1168 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
1169 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
*
*
1170 KK R66A
1171 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
1172 DI D66A 21
1173 DI 0 10000
1174 DQ 0 10000
*
*
1175 KK 66ATB
1176 KM ROUTE S66A TO 66B VIA WASH CROSSING BASELINE
1177 RS 9 FLOW -1
1178 RC .045 .04 .045 7500 .0077
1179 RX 0 500 980 1003 1007 1031 1511 2011
1180 RY 4 3.5 3 0 0 3 3.5 4
*
*
1181 KK 66B
1182 KM BASIN 66B
1183 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
1184 KM L= 1.6 Lca= 1.0 S= 43.3 Kn= .050 LAG= 42.8
1185 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
1186 BA .67
1187 LG .25 5.00 .33 30.00
1188 UI 53. 56. 185. 248. 297. 352. 426. 590. 636. 496.
1189 UI 419. 346. 286. 232. 152. 93. 66. 58. 53. 21.
1190 UI 16. 16. 16. 16. 16. 0. 0. 0. 0. 0.
1191 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
*
*

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HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1192 KK R66B
1193 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
1194 DI D66B 48
1195 DI 0 10000
1196 DQ 0 10000
*
*
1197 KK CP66B
1198 KM COMBINE S66A AND S66B
1199 HC 2
*
*
1200 KK 66BTC
1201 KM ROUTE 66B TO 66C VIA WASH

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1202	RS	4	FLOW	-1						
1203	RC	.045	.04	.045	6000	.0150				
1204	RX	0	500	995	1003	1007	1016	1511	2011	
1205	RY	4	3.5	3	0	0	3	3.5	4	

1206 KK 66C  
 1207 KM BASIN 66C  
 1208 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 1209 KM L= 1.1 Lca= .7 S= 46.5 Kn= .039 LAG= 24.3  
 1210 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 1211 BA .50  
 1212 LG .19 .25 5.40 .29 48.00  
 1213 UI 69. 243. 385. 528. 817. 635. 463. 325. 159. 103.  
 1214 UI 63. 21. 21. 21. 0. 0. 0. 0. 0. 0.  
 1215 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

1216 KK R66C  
 1217 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 1218 DT D66C 42  
 1219 DI 0 10000  
 1220 DQ 0 10000

1221 KK CP66C1  
 1222 KM Split up hydrograph combination in order to separate flows.  
 1223 KM Combine Hydrographs 66BIC (from Sub. 66A) and R66C (from Sub. 66C)  
 \* KO 2 2  
 1224 HC 2

HEC-1 INPUT

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1225 KK CP66C2  
 1226 KM Combine Hydrograph CP66C1 (from Subbasins 66A, 66B and 66C)  
 1227 KM plus hydrograph 67DI66 ( C67D )  
 \* KO 1 2  
 1228 HC 2

1229 KK DI66  
 1230 KM DIVERT FLOW TO DETENTION BASIN WA  
 1231 KM By-pass Flow Reduced to 410 cfs from 450, S2, 5-17-99  
 \* KO 1 2  
 1232 DT DB66  
 1233 DI 0 150 363 411.0 456.0 513 577 643 712 1000  
 1234 DQ 0 0 0 32.0 71. 122 179 239 302 590

1235 KK 66C1T2  
 1236 KM ROUTE FLOW FROM DIVERSION STRUCTURE TO ELLIOT CHANNEL at ELLIOT ROAD.  
 1237 KM REACH CN-1 plus culvert CNC-1  
 1238 KM A single pipe size and an overall slope are used to represent this  
 1239 KM 1,070 ft long reach which has pipe sizes of 76", 84" and 90", and  
 1240 KM about 250' long sideweir and transition open channel.  
 1241 KM RD card used for routing (Sta. 9+30 to Sta. 20+00)  
 1242 ED 1070 0.0130 0.012 CIRC 7

1243 KK CP66C  
 1244 KM COMBINE FLOWS FROM ELLIOT CHANNEL AND CRISMON BYPASS CHANNEL  
 \* KO 1 2  
 1245 HC 2

1246 KK 66C1D  
 \* KM REACH ET-7  
 1247 KM ROUTE FLOWS FROM INTERSECTION OF CRISMON AND ELLIOT CHANNELS  
 1248 KM AT THE INTERSECTION OF ELLIOT ROAD and CRISMON ROAD TO THE ELLIOT BASIN  
 1249 KM WA Bleed-off Outlet, WHICH IS ABOUT 390 ft WEST OF CRISMON ROAD.  
 \* RD card used for routing (Sta. 57+35 to Sta. 61+25)  
 1250 RD 390 0.0052 0.012 CIRC 9.5

1251 KK DR66  
 \* KO 1  
 1252 KM RETURN DIVERT TO DETENTION BASIN FROM DIVERSION STRUCTURE  
 1253 DR DB66

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1254 KK RS66D1  
 \* KO 1

```

1255 KM ELLIOT BASIN, WEST A
1256 KM TWO PONDS OPERATING IN SERIES.
1257 KM Bottom Elevation Lowered to 1415.0 ft from 1420, and 18" Bleed-off
1258 KM Pipe Added from WA to Elliot Channel
* Since the bleed-off pipe length is short, no routing is provided.
* Existing SS - 1423 20 2.5 1.5, S2, 5-18-99
RS 1 STOR 0
1259 SV 0 1.60 10.00 25.50 34.70 44.20 54.10 64.40 75.10 86.00
1261 SE 1415.0 1417 1419 1421 1422 1423 1424 1425 1426 1427
1262 SL 1416.0 1.7672 .62 .5
1263 SS 1423.5 20 3.0 1.5
*
*
1264 KK B-WA
1265 KM Bleed-off Flow from WA to Elliot Channel - 18" Pipe, S2, 6-15-99
1266 KM Divert Flow to WB by Weir Spillover ($S card on RS66D1)
* RS66D1 is the total routed flow - SL + $S
* This operation is designed to separate weir flow from pipe flow
1267 DT D-WB
1268 DI 0 5 10 15 17.59 40.87 80.62 131.76 192.12 260.43
1269 DQ 0 0 0 0 0 21.2 60.0 110.2 169.7 237.2
*
*
1270 KK C-WA
1271 KM Combine Bleed-off Flow from WA with Flow in Elliot Channel
* Added by S2, 5-17-99
1272 HC 2
*
*
1273 KK RC-WA
1274 KM Route Flow from WA Outlet to WB Outlet in Elliot Channel
* Added by S2, 5-17-99
* RS FLOW -1
* RC .025 .015 .025 800 .0017
* RK 0 8 16 28 44 56 64 72
* RY 5.7 5.8 6.0 0 0 6.0 5.8 5.7
1275 KM WA Bleed-off Outlet to WB Bleed-off Outlet.
* RD card used for routing (Sta. 48+80 to Sta. 57+35)
1276 RD 855 0.0052 0.012 CIRC 9.5
*
*
1277 KK DR-WA
1278 KM Return Diverted Flow (Spillway) to WB from WA, S2 5-7-99
* KO 1
1279 DR D-WB
*
*

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1

HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1280 KK RS66D2
* KO 1
1281 KM ELLIOT BASIN, WEST B
1282 KM TWO PONDS OPERATING IN SERIES.
1283 KM Bottom Elevation Lowered to 1413.5 ft from 1414, and 36" Bleed-off
1284 KM Pipe Reduced to 18" from WB to Elliot Channel
* Since the bleed-off pipe length is short, no routing is provided.
* Existing SS - 1420.5 80 2.5 1.5, S2, 5-18-99
RS 1 STOR 0
1285 SV 0 4.40 8.80 14.50 21.00 28.00 35.30 42.90 50.90 59.20
1287 SE 1412.0 1415 1416 1417 1418 1419 1420 1421 1422 1423
1288 SL 1413.0 1.7672 .62 .5
1289 SS 1422.6 50 2.5 1.5
*
*
1290 KK CP66D
1291 KM COMBINE FLOWS FROM WEST ELLIOT BASIN AND ELLIOT CHANNEL
1292 KM AT THE OUTLET PIPE.
* KO 1
1293 HC 2
*
*
1294 KK 66T66D
* KM REACH ET-6
1295 KM ROUTE FROM DETENTION BASIN WB OUTLET TO ELLSWORTH RD
1296 KM 2350 -> 3200, S2, 5-17-99
* First portion
* RD card used for routing (Sta. 36+44 to 48+80)
1297 RD 1236 0.0052 0.012 CIRC 9.5
*
*
1298 KK 66-66D
* KM REACH ET-5
1299 KM ROUTE FROM DETENTION BASIN WB OUTLET TO ELLSWORTH RD
1300 KM 2350 -> 3200, S2, 5-17-99
* Second portion
* RD card used for routing (Sta. 12+46 to Sta. 36+44)
1301 RD 2398 0.0040 0.012 CIRC 9.5
*
*

```

1302 KK 66D  
 1303 KM BASIN 66D  
 1304 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 1305 KM L= 1.0 Lca= .7 S= 28.6 Kn= .020 LAG= 13.2  
 1306 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 1307 BA .31  
 1308 LG .10 .17 6.80 .19 80.00  
 1309 UI 162. 480. 845. 540. 232. 89. 24. 0. 0. 0.  
 1310 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 \*

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1  
 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1311 KK R66D  
 1312 KM RETENTION REDUCED BY 77% FROM 31 TO 7 AC-FT  
 1313 KM DUE TO DEVELOPMENT USING DETENTION BASIN  
 \* The developer does not participate in the basin so the retention volume  
 \* increased to 31 A-F  
 1314 DT D66D 31  
 1315 DI 0 10000  
 1316 DQ 0 10000  
 \*

1317 KK 61A  
 1318 KM BASIN 61A  
 1319 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 1320 KM L= .9 Lca= .4 S= 36.8 Kn= .037 LAG= 19.1  
 1321 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 1322 BA .52  
 1323 LG .19 .25 4.20 .56 52.00  
 1324 UI 117. 412. 628. 1037. 786. 517. 261. 132. 62. 28.  
 1325 UI 28. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 1326 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 \*

1327 KK R61A  
 1328 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 1329 DT D61A 42  
 1330 DI 0 10000  
 1331 DQ 0 10000  
 \*

1332 KK 61ATB  
 1333 KM ROUTING 61A TO 61B VIA ELLSWORTH ROAD  
 1334 RS 10 FLOW -1  
 1335 RC .035 .024 .035 5280 .005  
 1336 RX 0 500 750 752 802 852 1102 1602  
 1337 RY 3 2 1.5 1.2 1.2 1.5 2 3  
 \*

1338 KK 61B  
 1339 KM BASIN 61B  
 1340 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 1341 KM L= 1.4 Lca= .7 S= 39.7 Kn= .047 LAG= 33.6  
 1342 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 1343 BA 1.09  
 1344 LG .24 .25 4.80 .37 35.00  
 1345 UI 109. 223. 475. 615. 765. 1049. 1335. 1025. 822. 643.  
 1346 UI 495. 288. 187. 143. 109. 36. 34. 34. 34. 34.  
 1347 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 1348 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 \*

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1  
 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1349 KK R61B  
 1350 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 1351 DT D61B 81  
 1352 DI 0 10000  
 1353 DQ 0 10000  
 \*

1354 KK CP61B  
 1355 KM COMBINE FLOWS FROM S61A AND S61B  
 1356 HC 2  
 \*

1357 KK 61T66D  
 1358 KM ROUTE CP61B TO SUBBASIN 66D ALONG ELLSWORTH ROAD. ROUTING WILL BE  
 1359 KM THE SAME AS WAS GIVEN FOR SUBBASIN 61A  
 1360 RS 9 FLOW -1  
 1361 RC .035 .024 .035 5280 .008  
 1362 RX 0 500 750 752 802 852 1102 1602  
 1363 RY 3 2 1.5 1.2 1.2 1.5 2 3  
 \*

1364 KK 67E  
 1365 KM BASIN 67E  
 1366 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 1367 KM L= 1.2 Lca= .7 S= 32.3 Kn=.038 LAG= 26.9  
 1368 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 1369 BA .58  
 1370 LG .19 .25 5.40 .30 50.00  
 1371 UI 73. 219. 378. 496. 732. 830. 597. 443. 315. 157.  
 1372 UI 110. 73. 24. 22. 22. 22. 0. 0. 0. 0.  
 1373 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

1374 KK R67E  
 1375 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 1376 KM DUE TO DEVELOPMENT USING DETENTION BASIN  
 1377 DT D67E 50  
 1378 DI 0 10000  
 1379 DQ 0 10000

1380 KK C67E  
 1381 KM COMBINE FLOWS FROM ELLSWORTH ROAD JUST NORTH OF ELLIOT ROAD  
 1382 HC 2

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1  
 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1383 KK C66D  
 1384 KM COMBINE ELLIOT CHANNEL FLOW WITH HYDROGRAPH C67E @ ELLIOT RD & ELLSWORTH RD.  
 1385 HC 3

1386 KK 66T23A  
 1387 KM NAME WAS CHANGED FROM 66T70A TO 66T23A  
 1388 KM Pipe Routing, Reach ET-5  
 \* SE corner curve of Elliot Rd. & Ellsworth Rd.  
 \* RD card used for routing (Junction Structure to Sta. 12+46)  
 1389 RD 253 0.0015 0.012 CIRC 9.5

1390 KK 66T23B  
 1391 KM NAME WAS CHANGED FROM 66T70B TO 66T23B  
 1392 KM Pipe Routing, Reach ET-5  
 \* Elliot Rd. to Culvert along Ellsworth Rd. 2-102" pipe = 144" pipe  
 \* RD card used for routing (Sta. 85+65 to Sta. 97+51)  
 1393 RD 1186 0.0015 0.012 CIRC 12

1394 KK CULVT  
 1395 KM Pipe Routing, Culvert  
 \* 2-102" pipe culvert crossing Ellsworth Rd.  
 \* RD card used for routing  
 1396 RD 196 0.0008 0.012 CIRC 12  
 \* KK66T70C  
 \* KM REACH ET-4, ET-5 ( COMPRISED OF ET-5A AND ET-5B).  
 \* KM ROUTE FROM ELLSWORTH Culvert TO SANTAN FREEWAY.  
 \* RS 2 FLOW -1  
 \* RC .032 .032 .032 2490 .0008  
 \* RX 0 10 20 56 78 112 120 130  
 \* RY 6.2 6.1 6.0 0 0 6.0 6.1 6.2

1397 KK 66T23C  
 1398 KM ROUTE ELLIOT STORM DRAIN FLOW SOUTH TO MESQUITE ROAD ALIGNMENT ALONG THE  
 1399 KM WEST SIDE OF ELLSWORTH ROAD VIA ENGINEERED CHANNEL.  
 1400 RS 1 FLOW -1  
 1401 RC .032 .032 .032 1000 .0021  
 1402 RX 0 5 10 46 56 92 97 102  
 1403 RY 10 9.5 9 0 0 9 9.5 10

\* \*\*\*\*\*UPDATED MPG PLANNING \*\*\*\*\*  
 \* \*\*\*\*\*DWM 09/07/2011\*\*\*\*\*

HEC-1 INPUT

1  
 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1404 KK 04 BASIN  
 1405 KM BASIN 04  
 1406 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 1407 KM L=1.39 Lca=0.44 S=22.3 Kn=0.033 LAG=21.9  
 1408 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 1409 BA 0.309  
 1410 LG 0.13 0.15 8.80 0.08 68  
 1411 UI 0 48 191 287 437 533 361 251 127 73  
 1412 UI 41 15 15 15 0 0 0 0 0 0

EMD089.txt  
 1413 UI 0 0 0 0 0 0 0 0 0 0  
 1414 UI 0 0 0 0 0 0 0 0 0 0  
 1415 UI 0 0 0 0 0 0 0 0 0 0  
 \*

1416 KK RET04 DIVERT  
 1417 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 \* KO 2  
 1418 DT 04RET 30.4 0.0  
 1419 DI 0 10000  
 1420 DQ 0 10000  
 \*

1421 KK CP23  
 1422 KM COMBINE FLOW FROM 66T23C AND BASIN 04.  
 \* KO 2  
 1423 MC 2  
 \*

1424 KK 66T23D  
 1425 KM ROUTE ELLIOT STORM DRAIN FLOW WEST TO LOOP 202 EAST CHANNEL ALONG THE NORTH  
 1426 KM SIDE OF THE MESQUITE ROAD ALIGNMENT VIA ENGINEERED CHANNEL.  
 1427 RS 1 FLOW -1  
 1428 RC .032 .032 .032 2300 .0021  
 1429 RX 0 5 10 46 56 92 97 102  
 1430 RI 10 9.5 9 0 0 9 9.5 10  
 \*

1431 KK 62C  
 1432 KM BASIN 62C  
 1433 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 1434 KM L= .6 Lca= .3 S= 24.2 Km= .049 LAG= 19.8  
 1435 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 1436 BA .55  
 1437 LG .25 .25 4.65 .40 47.00  
 1438 UI 112. 406. 615. 1024. 853. 571. 330. 154. 83. 28.  
 1439 UI 20. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 1440 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 \*

1

HEC-1 INPUT

PAGE 39

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1441 KK R62C  
 1442 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 1443 DI D62C 31  
 1444 DI 0 10000  
 1445 DQ 0 10000  
 \*

1446 KK 62CTE  
 1447 KM ROUTE BASIN 62C TO BASIN 62E BY CHANNEL ON EAST SIDE OF PROPOSED SANTAN  
 1448 KM FREEWAY ALIGNMENT  
 1449 RS 3 FLOW -1  
 1450 RC .030 .030 .030 2000 .0003  
 1451 RX 0 5 10 25 45 55 60 65  
 1452 RY 8 7 6.5 0 0 6.5 7 8  
 \*

1453 KK 62E  
 1454 KM BASIN 62E  
 1455 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 1456 KM L= .6 Lca= .3 S= 31.8 Km= .050 LNG= 20.4  
 1457 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 1458 BA .15  
 1459 LG .25 .25 4.65 .39 45.00  
 1460 UI 29. 108. 163. 268. 246. 167. 104. 46. 28. 10.  
 1461 UI 8. 8. 0. 0. 0. 0. 0. 0. 0. 0.  
 1462 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 \*

1463 KK R62E  
 1464 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 1465 DT D62E 12  
 1466 DI 0 10000  
 1467 DQ 0 10000  
 \*

1468 KK CP62E  
 1469 KM COMBINE FLOWS FROM SUBBASIN 62C AND SUBBASIN 62E  
 1470 HC 2  
 \*

1471 KK 62T68A  
 1472 KM ROUTE FLOW FROM CP62E TO SUBBASIN 68A BY CHANNEL ALONG PROPOSED ALIGNMENT  
 1473 KM OF THE SANTAN FREEWAY  
 \* ZW A-62T68A B-NORTH OF ELLIOT C-FLOW F-100YR FUTURE  
 1474 RS 5 FLOW -1



```

EMDU89.txt
1475 RC .030 .030 .030 3280 .00015
1476 RX 0 5 10 20 30 40 45 50
1477 RY 20 15 15 0 0 15 15 20

```

```

*
* BASIN 68A WAS SEPERATED INTO 2 BASINS TO CALCULATE OFFSITE FLOW IMPACTS
* TO BASIN 23.
*
* KK 68A
* KM BASIN 68A
* KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
* KM L=.7 Lca=.4 S= 37.7 Kn=.032 LAG= 13.7
* KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
* BA .35
* LG .16 .25 5.70 .27 66.00
* UI 168. 506. 914. 635. 301. 114. 34. 26. 0.
* UI 0. 0. 0. 0. 0. 0. 0. 0. 0.
*

```

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

1478 KK 68A1 BASIN
1479 KM BASIN 68A1
1480 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
1481 KM L=0.92 Lca=0.46 S=37.7 Kn=0.030 LAG=15.6
1482 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
1483 BA 0.297
1484 LG 0.16 0.25 5.70 0.27 3
1485 UI 106 340 612 604 368 151 73 20 20 0
1486 UI 0 0 0 0 0 0 0 0 0 0

```

```

1487 KK 68A2 BASIN
1488 KM BASIN 68A2
1489 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
1490 KM L=0.50 Lca=0.25 S=37.8 Kn=0.030 LAG=9.8
1491 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
1492 BA 0.048
1493 LG 0.18 0.25 5.70 0.27 80
1494 UI 46 147 124 41 9 0 0 0 0 0
1495 UI 0 0 0 0 0 0 0 0 0 0

```

```

1496 KK CP68A1
1497 KM COMBINE FLOWS FROM BASINS 68A1 AND 68A2
1498 HC 2

```

```

1499 KK R68A
1500 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
1501 DT 968A 31
1502 DI 0 10000
1503 DQ 0 10000

```

```

1504 KK CP68A2
1505 KM COMBINE FLOW FROM BASINS 68A1 AND 68A2 WITH THE ROUTED FLOW FROM CP62E
* KO 3 2
* ZW A=COMBINED FLOW, CP68A B=FROM BASIN AND ROUTE C FLOW F=100YR FUTURE
1506 HC 2

```

```

1507 KK 68T70A
1508 KM ROUTE FLOW FROM CP68A AT ELLIOT AND SANTAN FREEWAY ALIGNMENT TO SUBBASIN
1509 KM 70A, AT THE POINT WHERE SIPHON DRAW INTERSECS THE FREEWAY ALIGNMENT
1510 KM CHANNEL IS NATURAL AND ONLY APPROXIMATE IN ROUTING PARAMETERS
1511 RS 5 FLOW -1
1512 RC .030 .030 .030 3960 .0006
1513 RX 0 5 10 20 30 40 45 50
1514 RY 10 5 4 0 0 4 5 10

```

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

1515 KK 70A1 BASIN
1516 KM BASIN 70A1
1517 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
1518 KM L=0.52 Lca=0.26 S=3.8 Kn=0.030 LAG=15.7
1519 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
1520 BA 0.053
1521 LG 0.10 0.25 5.70 0.29 80
1522 UI 18 61 106 110 65 29 13 3 4 0
1523 UI 0 0 0 0 0 0 0 0 0 0

```

```

1524 KK 23 BASIN
1525 KM BASIN 23
1526 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN

```

EMDUR9.txt  
 1527 KM L=0.78 Lca=0.36 S=17.9 Kn=0.036 LAG=18.5  
 1528 KM PHOENIX VALLEY S-GGRAPH WAS USED FOR THIS BASIN  
 1529 BA 0.218  
 1530 LG 0.10 0.21 6.40 0.22 76  
 1531 UI 53 183 283 450 325 206 93 51 19 12  
 1532 UI 12 0 0 0 0 0 0 0 0 0  
 1533 UI 0 0 0 0 0 0 0 0 0 0

1534 KK RET23 DIVERT  
 1535 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 1536 DT 23RET 22.7 0.0  
 1537 DI 0 10000  
 1538 DQ 0 10000

1539 KK C70A1  
 1540 KM COMBINE FLOWS FROM 68T70A, 66T23D, BASIN 70A1, AND BASIN 23.  
 \* KO 2  
 1541 HC 4

1542 KK 70A1T2  
 1543 KM ROUTE FLOW ALONG LOOP 202 WITHIN THE EAST CHANNEL FROM MESQUITE ROAD TO  
 1544 KM WARNER ROAD.  
 1545 RS 2 FLOW -1  
 1546 RC .025 .025 .025 2675 0.0005  
 1547 RX 0 8 16 59 91 134 142 150  
 1548 RY 7.4 7.3 7.2 0 0 7.2 7.1 7.0

1

HEC-1 INPUT

PAGE 42

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1549 KK 24 BASIN  
 1550 KM BASIN 24  
 1551 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 1552 KM L=0.83 Lca=0.38 S=24.1 Kn=0.035 LAG=17.8  
 1553 KM PHOENIX VALLEY S-GGRAPH WAS USED FOR THIS BASIN  
 1554 BA 0.252  
 1555 LG 0.10 0.15 8.80 0.09 79  
 1556 UI 67 229 357 538 359 221 91 51 14 15  
 1557 UI 0 0 0 0 0 0 0 0 0 0

1558 KK RET24 DIVERT  
 1559 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 1560 DT 24RET 26.5 0.0  
 1561 DI 0 10000  
 1562 DQ 0 10000

1563 KK 70A2 BASIN  
 1564 KM BASIN 70A2  
 1565 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 1566 KM L=0.51 Lca=0.26 S=19.6 Kn=0.030 LAG=11.4  
 1567 KM PHOENIX VALLEY S-GGRAPH WAS USED FOR THIS BASIN  
 1568 BA 0.036  
 1569 LG 0.10 0.15 8.40 0.10 80  
 1570 UI 25 77 106 49 15 4 0 0 0 0  
 1571 UI 0 0 0 0 0 0 0 0 0 0

1572 KK CP70A2  
 1573 KM COMBINE FLOWS FROM 70A1T2 AND BASINS 24 AND 70A2  
 1574 KM  
 1575 HC 3

1576 KK 70T76A  
 1577 KM DIBBLE DRAINAGE FACILITY  
 1578 KM ROUTE FLOW ALONG NEW SANTAN FREEWAY ALIGNMENT  
 1579 KM REACH ET-3A, ET-3B  
 1580 RS 4 FLOW -1  
 1581 RC .025 .025 .025 4500 0.0005  
 1582 RX 0 8 16 59 91 134 142 150  
 1583 RY 7.4 7.3 7.2 0 0 7.2 7.1 7.0

1

HEC-1 INPUT

PAGE 43

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1584 KK 76A  
 1585 KM BASIN 76A  
 1586 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 1587 KM L= 2.9 Lca= 1.7 S= 24.1 Kn= .030 LAG= 42.9  
 1588 KM PHOENIX VALLEY S-GGRAPH WAS USED FOR THIS BASIN  
 1589 BA 1.91

EMDUS9.txt  
 1590 LG .15 .15 8.80 .08 56.00  
 1591 UI 150. 159. 528. 707. 847. 1004. 1213. 1673. 1826. 1424.  
 1592 UI 1201. 992. 822. 667. 444. 266. 247. 171. 150. 65.  
 1593 UI 46. 46. 46. 46. 46. 0. 0. 0. 0. 0.  
 1594 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

1595 KK R75A  
 1596 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 1597 DT D75A 185  
 1598 DI 0 10000  
 1599 DQ 0 10000

1600 KK C76A  
 1601 KM COMBINE HYDROGRAPHS 70T76A (SANTAN FREEWAY CHANNEL FLOWS) WITH SUBBASIN 76A  
 \* KO 2  
 1602 HC 2  
 \*  
 \* KK\*DESAN  
 \* KM DIVERT FROM SANTAN CHANNEL INTO THE RAY DEIENTION BASIN  
 \* KM ADDED BY CPE IN JUNE 2000.  
 \* KM USES A REALISTIC SIDE-WEIR EQUATION TO FORM POWER CURVE  
 \* KM WEIR CREST = 4.5 FT; WEIR LENGTH = 200FT; 4.0 FT DIV STRUCTURE.  
 \* KO 3  
 \* DT SANDB 1537  
 \* DI 0 750 772 819 892 999 1356 8138  
 \* DQ 0 0 7 31 78 154 439 6509

1603 KK 76ATER  
 1604 KM DIBBLE DRAINAGE FACILITY  
 1605 KM ROUTE FLOW ALONG NEW SANTAN FREEWAY ALIGNMENT TO NEW POWERLINE FLOODWAY ALGN.  
 1606 KM REACH ET-2A, ET-2B  
 1607 RS 5 FLOW -1  
 1608 RC .025 .025 .025 5750 0.0005  
 1609 RX 0 8 16 61 93 138 146 154  
 1610 RY 7.7 7.6 7.5 0 0 7.5 7.4 7.3

1 MEC-1 INPUT PAGE 44

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1611 KK 73A  
 1612 KM BASIN 73A  
 1613 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 1614 KM L= 2.3 Lca= 1.0 S= 34.9 Kn= .093 LAG= 94.5  
 1615 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 1616 BA .95  
 1617 LG .35 .36 5.00 .27 .00  
 1618 UI 34. 34. 34. 34. 84. 117. 134. 156. 171. 185.  
 1619 UI 197. 214. 232. 254. 274. 317. 381. 429. 424. 369.  
 1620 UI 332. 303. 282. 263. 240. 220. 202. 185. 169. 157.  
 1621 UI 134. 107. 90. 60. 60. 57. 55. 54. 34. 34.  
 1622 UI 34. 34. 16. 10. 10. 10. 10. 10. 10. 10.  
 1623 UI 10. 10. 10. 10. 10. 10. 0. 0. 0. 0.  
 1624 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

1625 KK 73ATB ROUTE  
 1626 KM ROUTE FLOW FROM BASIN 73A THROUGH THE MOUNTAIN HEIGHTS DEVELOPEMENT FROM  
 1627 KM MERIDIAN ROAD TO MOUNTAIN ROAD.  
 1628 RS 2 FLOW -1  
 1629 KC 0.045 0.040 0.045 2030 0.0050 0.00  
 1630 KX 0.00 5.00 10.00 20.00 120.00 130.00 135.00 140.00  
 1631 KY 4.00 3.00 2.50 0.00 0.00 2.50 3.00 4.00

1632 KK 73B BASIN  
 1633 KM BASIN 73B  
 1634 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 1635 KM L=0.56 Lca=0.28 S=30.4 Kn=0.040 LAG=14.9  
 1636 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 1637 BA 0.425  
 1638 LG 0.25 0.25 5.40 0.27 30  
 1639 UI 169 530 973 829 461 180 73 30 0 0  
 1640 UI 0 0 0 0 0 0 0 0 0 0

1641 KK REIT3B DIVERT  
 1642 KM RETAIN 80% OF THE 100 YR 2 HR RUNOFF VOLUME  
 1643 DT 73BRET 39.5 0.0  
 1644 DI 0 10000  
 1645 DQ 0 10000

1646 KK CP73B COMBINE  
 1647 KM COMBINE HYDROGRAPHS 73ATB AND BASIN 73B  
 1648 HC 2

## HEC-1 INPUT

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1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1649 KK 73BTC ROUTE  
 1650 KM ROUTE FLOW THROUGH THE NOVA VISTA DEVELOPEMENT FROM MOUNTAIN ROAD TO  
 1651 KM SIGNAL BUTTE ROAD.  
 1652 RS 4 FLOW -1  
 1653 RC 0.045 0.040 0.045 4500 0.0050 0.00 0.00  
 1654 RX 0.00 5.00 10.00 22.00 122.00 134.00 139.00 144.00  
 1655 RY 4.00 3.50 3.00 0.00 0.00 3.00 3.50 4.00

\*  
 \*  
 1656 KK 73C BASIN  
 1657 KM BASIN 73C  
 1658 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 1659 KM L=1.33 Lca=0.30 S=22.6 Kn=0.040 LAG=22.5  
 1660 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 1661 BA 0.585  
 1662 LG 0.25 0.25 5.40 0.27 30  
 1663 UI 88 344 512 764 1019 695 488 287 149 88  
 1664 UI 31 27 26 0 0 0 0 0 0 0  
 1665 UI 0 0 0 0 0 0 0 0 0 0

\*  
 \*  
 1666 KK RET73C DIVERT  
 1667 KM RETAIN 80% OF THE 100 YR 2 HR RUNOFF VOLUME  
 1668 DT 73CRET 37.2 0.0  
 1669 DI 0 10000  
 1670 DQ 0 10000

\*  
 \*  
 1671 KK CP73C COMBINE  
 1672 KM COMBINE HYDROGRAPHS 73BTC AND BASIN 73C  
 1673 HC 2

\* \*\*\*\*\*UPDATED CHANNEL SECTION\*\*\*\*\*  
 \* \*\*\*\*\*DWM 09/07/2011\*\*\*\*\*  
 \* 6:1 SS EAST BANK, 3:1 SS WEST BANK, 38FT BOTTOM, 3.5 FT DEEP  
 \*

1674 KK 73T74C ROUTE  
 1675 KM ROUTE FLOW SOUTH ALONG THE WEST SIDE OF SIGNAL BUTTE ROAD IN AN  
 1676 KM ENGINEERED CHANNEL FROM WARNER ROAD TO THE POWERLINE FLOODWAY.  
 1677 RS 20 FLOW -1  
 1678 RC 0.032 0.032 0.032 4670 .0024  
 1679 RX 0 5 10 31 69 79.5 84.5 89.5  
 1680 RY 3.5 3.5 3.5 0 0 3.5 3.5 3.5

## HEC-1 INPUT

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1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1681 KK 74A  
 1682 KM BASIN 74A  
 1683 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 1684 KM L= 2.4 Lca= 1.0 S= 42.2 Kn= .095 LAG= 92.9  
 1685 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN

\* KO 2 2  
 1686 BA .75  
 1687 LG .35 .36 5.00 .27 .00  
 1688 UI 27. 27. 27. 27. 73. 96. 111. 129. 140. 151.  
 1689 UI 163. 175. 193. 208. 228. 268. 317. 362. 327. 287.  
 1690 UI 260. 239. 222. 206. 187. 171. 160. 142. 132. 118.  
 1691 UI 99. 79. 56. 48. 47. 45. 45. 32. 27. 27.  
 1692 UI 27. 19. 8. 8. 8. 8. 8. 8. 8. 8.  
 1693 UI 8. 8. 8. 8. 8. 8. 8. 8. 8. 8.  
 1694 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

\*  
 \*  
 1695 KK 74ATB ROUTE  
 1696 KM ROUTE FLOW FROM BASIN 74A VIA THE POWERLINE FLOODWAY FROM MERIDIAN ROAD TO  
 1697 KM MOUNTAIN ROAD. FLOW ENTERS THE POWERLINE FLOODWAY VIA A 75FT WEIR ON THE  
 1698 KM NORTHWEST CORNER OF THE MERIDIAN ROAD AND POWERLINE FLOODWAY INTERSECTION.  
 1699 RS 1 FLOW -1  
 1700 RC 0.013 0.013 0.013 3200 0.0060 0.00  
 1701 RX 0.00 7.00 21.50 30.00 36.00 44.50 59.00 66.00  
 1702 RY 6.00 5.50 5.50 0.00 0.00 5.50 5.50 6.00

\*  
 \*  
 1703 KK 74B BASIN  
 1704 KM BASIN 74B  
 1705 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 1706 KM L=1.31 Lca=0.41 S=23.7 Kn=0.040 LAG=24.9  
 1707 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 1708 BA 0.333  
 1709 LG 0.25 0.25 5.80 0.22 30  
 1710 UI 45 154 245 330 528 430 318 229 122 76  
 1711 UI 44 18 14 14 0 0 0 0 0 0

1712 UI 0 0 0 0 0 0 0 0 0 0  
 \*  
 \*  
 1713 KK RET74B DIVERT  
 1714 KM RETAIN 80% OF THE 100 YR 2 HR RUNOFF VOLUME  
 1715 DT 74BRET 17.8 0.0  
 1716 DI 0 10000  
 1717 DQ 0 10000  
 \*

1

HEC-1 INPUT

PAGE 47

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1718 KK CP74B COMBINE  
 1719 KM COMBINE HYDROGRAPHS 74ATB AND BASIN 74B  
 1720 HC 2  
 \*

1721 KK 74BTC ROUTE  
 1722 KM ROUTE FLOW VIA THE POWERLINE FLOODWAY FROM MOUNTAIN ROAD TO SIGNAL BUTTE  
 1723 KM ROAD.  
 1724 RS 1 FLOW -1  
 1725 RC 0.013 0.013 0.013 3100 0.0055 0.00  
 1726 RX 0.00 7.00 21.50 30.00 36.00 44.50 59.00 66.00  
 1727 RY 6.00 5.50 5.50 0.00 0.00 5.50 5.50 6.00  
 \*

1728 KK 74C BASIN  
 1729 KM BASIN 74C  
 1730 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 1731 KM L=1.22 Lca=0.40 S=25.4 Kn=0.040 LAG=23.7  
 1732 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 1733 BA 0.345  
 1734 LG 0.25 0.17 6.80 0.15 30  
 1735 UI 48 180 276 386 588 428 310 211 97 65  
 1736 UI 35 15 15 16 0 0 0 0 0 0  
 1737 UI 0 0 0 0 0 0 0 0 0 0  
 \*

1738 KK RET74C DIVERT  
 1739 KM RETAIN 80% OF THE 100 YR 2 HR RUNOFF VOLUME  
 1740 DT 74CRET 22.6 0.0  
 1741 DI 0 10000  
 1742 DQ 0 10000  
 \*

1743 KK CP74C COMBINE  
 1744 KM COMBINE HYDROGRAPHS 73T74C, 74BTC, AND BASIN 74C  
 1745 \* KO 2  
 HC 3  
 \*

\*\*\*\*\*UPDATED MPG PLANNING \*\*\*\*\*  
 \*\*\*\*\*DWN 09/07/2011\*\*\*\*\*

1746 KK 74CT10  
 1747 KM ROUTE FLOW FROM IN THE POWERLINE FLOODWAY FROM CP74C TO CP10  
 1748 RS 2 FLOW -1  
 1749 RC 0.030 0.013 0.030 4250 .0036  
 1750 RX 0 15 16.5 25 33 41.5 43 58  
 1751 RY 6.6 6.6 5.6 0 0 5.6 6.6 6.6  
 \*

1

HEC-1 INPUT

PAGE 48

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1752 KK 10 BASIN  
 1753 KM BASIN 10  
 1754 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 1755 KM L=1.01 Lca=0.56 S=20.8 Kn=0.045 LAG=29.3  
 1756 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 1757 BA 0.171  
 1758 LG 0.25 0.19 6.60 0.17 31  
 1759 UI 0 20 52 95 123 165 239 187 144 109  
 1760 UI 75 38 29 20 7 6 6 6 0 0  
 1761 UI 0 0 0 0 0 0 0 0 0 0  
 1762 UI 0 0 0 0 0 0 0 0 0 0  
 1763 UI 0 0 0 0 0 0 0 0 0 0  
 \*

1764 KK RET10 DIVERT  
 1765 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 1766 DT 10RET 12.7 0.0  
 1767 DI 0 10000  
 1768 DQ 0 10000  
 \*

1769 KK CP10  
 1770 KM COMBINE HYDROGRAPHS 74CT10 AND RET10  
 1771 HC 2

```

1772 KK 10T11
1773 KM ROUTE FLOW FROM IN THE POWERLINE FLOODWAY FROM CP10 TO CP11
1774 RS 1 FLOW -1
1775 RC 0.030 0.013 0.030 1080 .0060
1776 RX 0 15 16.5 26 34 43.5 45 60
1777 RY 7.3 7.3 6.3 0 0 6.3 7.3 7.3
    
```

```

1778 KK 11 BASIN
1779 KM BASIN 11
1780 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
1781 KM L=0.25 Lca=0.11 S=20.0 Kn=0.031 LAG=6.4
1782 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
1783 BA 0.061
1784 LG 0.21 0.25 4.20 0.54 53
1785 UI 0 134 275 55 0 0 0 0 0 0
1786 UI 0 0 0 0 0 0 0 0 0 0
1787 UI 0 0 0 0 0 0 0 0 0 0
1788 UI 0 0 0 0 0 0 0 0 0 0
1789 UI 0 0 0 0 0 0 0 0 0 0
    
```

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

1790 KK RET11 DIVERT
1791 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
1792 DT 11RET 5.1 0.0
1793 DI 0 10000
1794 DQ 0 10000
    
```

```

1795 KK CP11
1796 KM COMBINE HYDROGRAPHS 10T11 AND RET11
1797 HC 2
    
```

```

1798 KK 11Y75
1799 KM ROUTE FLOW FROM IN THE POWERLINE FLOODWAY FROM CP11 TO CP75
1800 RS 1 FLOW -1
1801 RC 0.030 0.013 0.030 5250 .0056
1802 RX 0 15 16.5 26 34 43.5 45 60
1803 RY 7.3 7.3 6.3 0 0 6.3 7.3 7.3
    
```

```

1804 KK 02B BASIN
1805 KM BASIN 02B
1806 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
1807 KM L=0.53 Lca=0.26 S=28.3 Kn=0.040 LAG=14.4
1808 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
1809 BA 0.242
1810 LG 0.15 0.25 5.40 0.30 56
1811 UI 0 104 319 589 460 251 88 35 17 0
1812 UI 0 0 0 0 0 0 0 0 0 0
1813 UI 0 0 0 0 0 0 0 0 0 0
1814 UI 0 0 0 0 0 0 0 0 0 0
1815 UI 0 0 0 0 0 0 0 0 0 0
    
```

```

1816 KK RET02B DIVERT
1817 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
1818 DT 02BRET 22.7 0
1819 DI 0 10000
1820 DQ 0 10000
    
```

```

1821 KK 2BT1 ROUTE
1822 KM ROUTE FLOW IN EXCESS OF THE 100-YEAR, 2-HR STORM OVERLAND TO
1823 KM COLF COURSE IN SUBBASIN 1
1824 RS 1 FLOW -1
1825 RC 0.032 0.032 0.032 50 0.0050
1826 RX 0.00 1 2 3 2003 2004 2005 2006
1827 RY 1.00 0.75 0.50 0.00 0.00 0.50 0.75 1.00
    
```

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

1828 KK 01 BASIN
1829 KM BASIN 01
1830 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
1831 KM L=0.94 Lca=0.29 S=27.7 Kn=0.057 LAG=26.6
1832 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
1833 BA 0.316
1834 LG 0.10 0.15 8.00 0.13 11
1835 UI 0 40 122 209 275 413 444 320 237 164
    
```

```

1836      UI      91      57      37      12      12      12      0      0      0      0
1837      UI      0      0      0      0      0      0      0      0      0      0
1838      UI      0      0      0      0      0      0      0      0      0      0
1839      UI      0      0      0      0      0      0      0      0      0      0

```

```

1840      KK      03      BASIN
1841      KM      BASIN 03
1842      KM      THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
1843      KM      L=0.35 Lca=0.03 S=34.3 Kn=0.030 LAG=3.9
1844      KM      PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
1845      BA      0.159
1846      LG      0.10      0.15      8.40      0.10      80
1847      UI      0      892      325      0      0      0      0      0      0
1848      UI      0      0      0      0      0      0      0      0      0
1849      UI      0      0      0      0      0      0      0      0      0
1850      UI      0      0      0      0      0      0      0      0      0
1851      UI      0      0      0      0      0      0      0      0      0

```

```

1852      KK      CP1
1853      KM      COMBINE HYDROGRAPHS 2BT1, 01, AND 03
1854      HC      3

```

```

1855      KK      RET01      DIVERT
1856      KM      RETAIN 100 YR 2 HR RUNOFF VOLUME
1857      DT      01RET      46.9      0.0
1858      DI      0      10000
1859      DQ      0      10000

```

```

1860      KK      1T6      ROUTE
1861      KM      ROUTE FLOW FROM BASIN 1 TO BASIN 6
1862      RS      1      FLOW      -1
1863      RC      0.013      0.013      0.013      1560      .0030
1864      RX      0      15      17      36      112      130      132      146
1865      RY      4      0      0      3      3      0      0      4

```

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

1866      KK      02A      BASIN
1867      KM      BASIN 02A
1868      KM      THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
1869      KM      L=0.45 Lca=0.15 S=31.1 Kn=0.041 LAG=11.0
1870      KM      PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
1871      BA      0.182
1872      LG      0.21      0.19      5.60      0.19      46
1873      UI      0      139      422      528      226      68      17      0      0      0
1874      UI      0      0      0      0      0      0      0      0      0      0
1875      UI      0      0      0      0      0      0      0      0      0      0
1876      UI      0      0      0      0      0      0      0      0      0      0
1877      UI      0      0      0      0      0      0      0      0      0      0

```

```

1878      KK      RET02A      DIVERT
1879      KM      RETAIN 100 YR 2 HR RUNOFF VOLUME
1880      DT      02ARET      15.9      0.0
1881      DI      0      10000
1882      DQ      0      10000

```

```

1883      KK      02C      BASIN
1884      KM      BASIN 02C
1885      KM      THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
1886      KM      L=0.53 Lca=0.26 S=23.3 Kn=0.040 LAG=14.4
1887      KM      PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
1888      BA      0.189
1889      LG      0.15      0.25      5.60      0.28      56
1890      UI      0      82      249      460      360      196      68      27      14      0
1891      UI      0      0      0      0      0      0      0      0      0      0
1892      UI      0      0      0      0      0      0      0      0      0      0
1893      UI      0      0      0      0      0      0      0      0      0      0
1894      UI      0      0      0      0      0      0      0      0      0      0

```

```

1895      KK      RET02C      DIVERT
1896      KM      RETAIN 100 YR 2 HR RUNOFF VOLUME
1897      DT      02CRET      28.9      0.0
1898      DI      0      10000
1899      DQ      0      10000

```

```

1900      KK      CP2
1901      KM      COMBINE HYDROGRAPHS RET02A AND RET02C
1902      HC      2

```

1

LINE	ID	1	2	3	4	5	6	7	8	9	10	
1903	KK	2AT6 ROUTE										
1904	KM	ROUTE FLOW FROM BASIN 2A TO BASIN 6										
1905	RS	2 FLOW -1										
1906	RC	0.013	0.013	0.013	3440	.0050						
1907	RX	0	15	17	36	112	130	132	146			
1908	RY	4	0	0	3	3	0	0	4			
1909	KK	06 BASIN										
1910	KM	BASIN 06										
1911	KM	THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN										
1912	KM	L=0.88 Lca=0.36 S=22.7 Kn=0.034 LAG=17.5										
1913	KM	PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN										
1914	BA	0.282										
1915	LG	0.22	0.25	4.70	0.39	46						
1916	UI	0	78	261	417	610	395	215	97	48	17	
1917	UI	17	0	0	0	0	0	0	0	0	0	
1918	UI	0	0	0	0	0	0	0	0	0	0	
1919	UI	0	0	0	0	0	0	0	0	0	0	
1920	UI	0	0	0	0	0	0	0	0	0	0	
1921	KK	RET06 DIVERT										
1922	KM	RETAIN 100 YR 2 HR RUNOFF VOLUME										
1923	DT	06RET 26.5 0.0										
1924	DI	0 10000										
1925	DQ	0 10000										

1926	KK	CP6										
1927	KM	COMBINE HYDROGRAPHS 116, 2AT6, AND RET06										
1928	MC	3										
1929	KK	6T5 ROUTE										
1930	KM	ROUTE FLOW FROM BASIN 6 TO BASIN 5										
1931	RS	1 FLOW -1										
1932	RC	0.013	0.013	0.013	2160	.0040						
1933	RX	0	15	17	36	112	130	132	146			
1934	RY	4	0	0	3	3	0	0	4			

1935	KK	05 BASIN										
1936	KM	BASIN 05										
1937	KM	THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN										
1938	KM	L=0.70 Lca=0.23 S=22.9 Kn=0.039 LAG=15.5										
1939	KM	PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN										
1940	BA	0.145										
1941	LG	0.19	0.13	10.10	0.05	36						

1

LINE	ID	1	2	3	4	5	6	7	8	9	10	
1942	UI	0	53	167	302	297	175	72	34	10	10	
1943	UI	0	0	0	0	0	0	0	0	0	0	
1944	UI	0	0	0	0	0	0	0	0	0	0	
1945	UI	0	0	0	0	0	0	0	0	0	0	
1946	UI	0	0	0	0	0	0	0	0	0	0	
1947	KK	RET05 DIVERT										
1948	KM	RETAIN 100 YR 2 HR RUNOFF VOLUME										
1949	DT	05RET 13.2 0.0										
1950	DI	0 10000										
1951	DQ	0 10000										
1952	KK	07 BASIN										
1953	KM	BASIN 07										
1954	KM	THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN										
1955	KM	L=0.77 Lca=0.39 S=23.4 Kn=0.045 LAG=22.5										
1956	KM	PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN										
1957	BA	0.243										
1958	LG	0.15	0.15	7.30	0.15	20						
1959	UI	0	36	142	215	320	418	291	204	118	62	
1960	UI	37	13	11	11	0	0	0	0	0	0	
1961	UI	0	0	0	0	0	0	0	0	0	0	
1962	UI	0	0	0	0	0	0	0	0	0	0	
1963	UI	0	0	0	0	0	0	0	0	0	0	
1964	KK	RET07 DIVERT										
1965	KM	RETAIN 100 YR 2 HR RUNOFF VOLUME										
1966	DT	07RET 20.1 0.0										
1967	DI	0 10000										



```

1968 DQ 0 10000
*
*
1969 KK CP5
1970 KM COMBINE HYDROGRAPHS 6T5, RET05, AND RET07
1971 HC 3
*
*
1972 KK 5T12
1973 KM ROUTE FLOW FROM BASIN 5 TO BASIN 12
1974 RS 1 FLOW -1
1975 RC 0.013 0.013 0.013 1630 .0040
1976 RX 0 15 17 36 112 130 132 146
1977 RY 4 0 0 3 3 0 0 4
*
*

```

1

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

1978 KK 12 BASIN
1979 KM BASIN 12
1980 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
1981 KM L=0.76 Lca=0.16 S=19.7 Kn=0.032 LAG=11.7
1982 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
1983 BA 0.269
1984 LG 0.22 0.15 7.60 0.12 36
1985 UI 0 182 535 792 394 121 36 0 0 0
1986 UI 0 0 0 0 0 0 0 0 0 0
1987 UI 0 0 0 0 0 0 0 0 0 0
1988 UI 0 0 0 0 0 0 0 0 0 0
1989 UI 0 0 0 0 0 0 0 0 0 0
*
*

```

```

1990 KK RET12 DIVERT
1991 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
1992 DT 12RET 24.6 0.0
1993 DI 0 10000
1994 DQ 0 10000
*
*

```

```

1995 KK CP12
1996 KM COMBINE HYDROGRAPHS 5T12 AND RET12
1997 HC 2
*
*

```

```

1998 KK 12T13
1999 KM ROUTE FLOW FROM BASIN 12 TO BASIN 13
2000 RS 4 FLOW -1
2001 RC 0.032 0.016 0.032 4520 .0020
2002 RX 0 5 10 15 125 130 135 140
2003 RY 2 1.5 1 0 0 1 1.5 2
*
*

```

```

2004 KK 08 BASIN
2005 KM BASIN 08
2006 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
2007 KM L=1.37 Lca=0.94 S=21.2 Kn=0.042 LAG=37.3
2008 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
2009 BA 0.769
2010 LG 0.22 0.21 5.40 0.20 32
2011 UI 0 69 115 274 362 437 541 769 817 625
2012 UI 515 412 329 219 122 112 69 54 21 21
2013 UI 21 21 21 0 0 0 0 0 0 0
2014 UI 0 0 0 0 0 0 0 0 0 0
2015 UI 0 0 0 0 0 0 0 0 0 0
*
*

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1

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

2016 KK RET08 DIVERT
2017 KM RETAIN 100 YR 2 HR RUNOFF VOLUME
2018 DT 08RET 64.7 0.0
2019 DI 0 10000
2020 DQ 0 10000
*
*

```

```

2021 KK 8T9
2022 KM ROUTE FLOW FROM BASIN 8 TO BASIN 9
2023 RS 1 FLOW -1
2024 RC 0.013 0.013 0.013 1550 .0050
2025 RX 0 15 17 36 112 130 132 146
2026 RY 4 0 0 3 3 0 0 4
*
*

```

```

2027 KK 09 BASIN
2028 KM BASIN 09

```

EMDUS9.txt

2029 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 2030 KM L=0.59 Lca=0.14 S=27.1 Kn=0.042 LAG=12.5  
 2031 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 2032 BA 0.155  
 2033 LG 0.24 0.25 5.70 0.26 40  
 2034 UI 0 91 267 444 255 93 30 13 0 0  
 2035 UI 0 0 0 0 0 0 0 0 0 0  
 2036 UI 0 0 0 0 0 0 0 0 0 0  
 2037 UI 0 0 0 0 0 0 0 0 0 0  
 2038 UI 0 0 0 0 0 0 0 0 0 0

2039 KK RET09 DIVERT  
 2040 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 2041 DT 09RET 13.5 0.0  
 2042 DI 0 10000  
 2043 DQ 0 10000

2044 KK CP9  
 2045 KM COMBINE HYDROGRAPHS BT9 AND RET09  
 2046 HC 2

2047 KK 9T13  
 2048 KM ROUTE FLOW FROM BASIN 9 TO BASIN 13  
 2049 RS 1 FLOW -1  
 2050 RC 0.013 0.013 0.013 4240 .0050  
 2051 RX 0 15 17 36 112 130 132 146  
 2052 RY 4 0 0 3 3 0 0 4

1

HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

2053 KK 13 BASIN  
 2054 KM BASIN 13  
 2055 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 2056 KM L=0.91 Lca=0.30 S=22.0 Kn=0.039 LAG=19.1  
 2057 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 2058 BA 0.307  
 2059 LG 0.18 0.25 5.00 0.37 45  
 2060 UI 0 69 243 371 613 465 306 154 78 37  
 2061 UI 17 17 0 0 0 0 0 0 0 0  
 2062 UI 0 0 0 0 0 0 0 0 0 0  
 2063 UI 0 0 0 0 0 0 0 0 0 0  
 2064 UI 0 0 0 0 0 0 0 0 0 0

2065 KK RET13 DIVERT  
 2066 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 2067 DT 13RET 27.6 0.0  
 2068 DI 0 10000  
 2069 DQ 0 10000

2070 KK CP13  
 2071 KM COMBINE HYDROGRAPHS 12T13, 9T13, AND RET13  
 2072 HC 3

2073 KK 13T75  
 2074 KM ROUTE FLOW FROM BASIN 13 TO BASIN CP75  
 2075 RS 1 FLOW -1  
 2076 RC 0.032 0.016 0.032 600 .0020  
 2077 RX 0 5 10 15 125 130 135 140  
 2078 RY 2 1.5 1 0 0 1 1.5 2

2079 KK 14 BASIN  
 2080 KM BASIN 14  
 2081 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 2082 KM L=1.12 Lca=0.63 S=25.9 Kn=0.042 LAG=28.5  
 2083 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 2084 BA 0.192  
 2085 LG 0.22 0.25 4.40 0.48 42  
 2086 UI 0 23 62 113 146 261 275 205 157 118  
 2087 UI 74 39 29 19 7 7 7 7 0 0  
 2088 UI 0 0 0 0 0 0 0 0 0 0  
 2089 UI 0 0 0 0 0 0 0 0 0 0  
 2090 UI 0 0 0 0 0 0 0 0 0 0

1

HEC-1 INPUT

PAGE 57

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

2091 KK RET14 DIVERT  
 2092 KM RETAIN 100 YR 2 HR RUNOFF VOLUME

2093 DT 14RET 15.8 0.0  
 2094 DI 0 10000  
 2095 DQ 0 10000  
 \*  
 \*

2096 KK CP75 COMBINE  
 2097 KM COMBINE HYDROGRAPHS 11T7B, 13T75, AND RET14  
 \* KO 2  
 2098 HC 3  
 \*  
 \*

2099 KK 75TPC  
 2100 KM ROUTE 75 THROUGH POWERLINE FLOODWAY TO AIR FORCE CHANNEL  
 2101 RS 1 FLOW -1  
 2102 RC .03 .013 .03 3900 .0641  
 2103 RX 0 1005 1023 1030.5 1036.5 1044 1062 2067  
 2104 RY 6 5 5 0 0 5 5 6  
 \*  
 \*

2105 KK 77A  
 2106 KM BASIN 77A  
 2107 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 2108 KM L= 2.9 Lca= 1.5 S= 31.1 Kn=.092 LAG= 119.0  
 2109 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 2110 BA 1.74  
 2111 LG .35 .36 5.00 .27 .00  
 2112 UI 49. 49. 49. 49. 49. 108. 162. 185. 205. 230.  
 2113 UI 244. 264. 278. 293. 311. 333. 358. 380. 406. 462.  
 2114 UI 537. 584. 659. 601. 541. 496. 461. 430. 407. 385.  
 2115 UI 362. 334. 311. 293. 273. 252. 238. 226. 189. 161.  
 2116 UI 141. 104. 87. 87. 83. 81. 81. 73. 49. 49.  
 2117 UI 49. 49. 49. 22. 15. 15. 15. 15. 15. 15.  
 2118 UI 15. 15. 15. 15. 15. 15. 15. 15. 15. 15.  
 2119 UI 15. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 2120 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 \*  
 \*

2121 KK 77AIB ROUTE  
 2122 KM ROUTE BASIN 77A THROUGH THE KEIGHLEY PLACE SUBDIVISION FROM MERIDIAN ROAD TO  
 2123 KM TO MOUNTAIN ROAD.  
 2124 RS 1 FLOW -1  
 2125 RC 0.045 0.040 0.045 3000 0.0050 0.00  
 2126 RX 0.00 5.00 10.00 37.00 47.00 74.00 79.00 84.00  
 2127 RY 5.50 5.00 4.50 0.00 0.00 4.50 5.00 5.50  
 \*  
 \*

1

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

2128 KK 77B BASIN  
 2129 KM BASIN 77B  
 2130 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 2131 KM L=0.58 Lca=0.26 S=28.6 Kn=0.047 LAG=17.2  
 2132 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 2133 BA 0.349  
 2134 LG 0.19 0.25 5.40 0.30 18  
 2135 UI 100 337 536 757 486 273 113 54 20 21  
 2136 UI 0 0 0 0 0 0 0 0 0 0  
 \*  
 \*

2137 KK RET77B DIVERT  
 2138 KM RETAIN 80% OF THE 100 YR 2 HR RUNOFF VOLUME  
 2139 DT 77BRET 16.5 0.0  
 2140 DI 0 10000  
 2141 DQ 0 10000  
 \*  
 \*

2142 KK CP77B COMBINE  
 2143 KM COMBINE HYDROGRAPHS 77ATD AND 77B.  
 2144 HC 2  
 \*  
 \*

2145 KK 77BIC ROUTE  
 2146 KM ROUTE FLOW THROUGH THE MOUNTAIN HORIZONS (SOUTH) DEVELOPEMENT FROM MOUNTAIN  
 2147 KM ROAD TO SIGNAL BUTTE ROAD.  
 2148 RS 3 FLOW -1  
 2149 RC 0.045 0.040 0.045 4750 0.0042 0.00  
 2150 RX 0.00 5.00 10.00 20.00 85.00 105.00 110.00 115.00  
 2151 RY 5.00 4.00 3.00 0.00 0.00 3.00 4.00 5.00  
 \*  
 \*

2152 KK 77C BASIN  
 2153 KM BASIN 77C  
 2154 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 2155 KM L=0.76 Lca=0.51 S=23.7 Kn=0.040 LAG=22.0  
 2156 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 2157 BA 0.279  
 2158 LG 0.25 0.25 6.00 0.21 30  
 \*  
 \*

2159 UI 42 172 257 388 ENDU89.txt 425 324 228 119 66 38  
 2160 UI 14 13 13 0 0 0 0 0 0 0  
 \*  
 \*  
 2161 KK FET77C DIVERT  
 2162 KM RETAIN 80% OF THE 100 YR 2 HR RUNOFF VOLUME  
 2163 DT 77CRET 16.8 0.0  
 2164 DI 0 10000  
 2165 DQ 0 10000  
 \*  
 \*

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HEC-1 INPUT

PAGE 59

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

2166 KK C77C COMBINE  
 2167 KM COMBINE HYDROGRAPHS 77BTC AND 77C  
 2168 HC 2  
 \*  
 \*

2169 KK 77CT78 ROUTE  
 2170 KM ROUTE FLOW SOUTH ALONG THE WEST SIDE OF SIGNAL BUTTE ROAD IN AN ENGINEERED  
 2171 KM CHANNEL FROM RAY ROAD TO WILLIAMS FIELD ROAD.  
 2172 RS 4 FLOW -1  
 2173 RC 0.032 0.032 0.032 4435 0.0020 0.00  
 2174 RX 0.00 5.00 10.00 24.00 124.00 138.00 143.00 148.00  
 2175 RY 4.50 4.00 3.50 0.00 0.00 3.50 4.00 4.50  
 \*  
 \*

2176 KK 78A  
 2177 KM BASIN 78A  
 2178 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 2179 KM L= 3.3 Lca= 1.3 S= 30.2 Kn= .090 LAG= 118.0  
 2180 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 2181 BA 1.88  
 2182 LG .35 .36 5.00 .27 .00  
 2183 UI 54. 54. 54. 54. 54. 124. 176. 203. 227. 252.  
 2184 UI 268. 290. 305. 322. 342. 366. 396. 417. 451. 515.  
 2185 UI 612. 641. 716. 643. 579. 531. 494. 464. 437. 417.  
 2186 UI 385. 356. 334. 315. 290. 270. 255. 233. 206. 159.  
 2187 UI 153. 95. 95. 95. 88. 88. 88. 88. 88. 54. 54.  
 2188 UI 54. 54. 45. 16. 16. 16. 16. 16. 16. 16. 16.  
 2189 UI 16. 16. 16. 16. 16. 16. 16. 16. 16. 16. 16.  
 2190 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 2191 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 \*  
 \*

2192 KK 78ATB ROUTE  
 2193 KM ROUTE FLOW FROM 78A TO 78B VIA WASH CROSSING COUNTY LINE  
 2194 RS 7 FLOW -1  
 2195 RC 0.045 0.040 0.045 3500 0.0042 0.00  
 2196 RX 0.00 500.00 980.00 1063.00 1007.00 1031.00 1511.00 2011.00  
 2197 RY 4.50 3.50 3.00 0.00 0.00 3.00 3.50 4.50  
 \*  
 \*

2198 KK 78B BASIN  
 2199 KM BASIN 78B  
 2200 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 2201 KM L=0.60 Lca=0.40 S=31.7 Kn=0.050 LAG=21.7  
 2202 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 2203 BA 0.396  
 2204 LG 0.30 0.17 6.80 0.15 15  
 2205 UI 61 254 371 576 682 457 315 156 90 48  
 2206 UI 20 19 0 0 0 0 0 0 0 0  
 \*  
 \*

\* CURRENTLY THERE IS NO EXISTING RETENTION OR PLANNED RETENTION FOR BASIN 78B  
 \* DUE TO THE CURRENT LAND USE OF LARGE LOT RESIDENTIAL.  
 \*

1

HEC-1 INPUT

PAGE 60

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

2207 KK C78B COMBINE  
 2208 KM COMBINE HYDROGRAPHS 78ATB AND 78B  
 2209 HC 2  
 \*  
 \*

2210 KK 78BTC ROUTE  
 2211 KM ROUTE 78B TO 78C VIA WASH CROSSING MOUNTAIN ROAD, THEN SOUTH ALONG  
 2212 KM WESTERN EDGE OF 78C.  
 2213 RS 3 FLOW -1  
 2214 RC 0.035 0.022 0.035 4500 0.0033 0.00  
 2215 RX 0.00 100.00 110.00 115.00 120.00 125.00 130.00 135.00  
 2216 RY 5.00 4.00 3.50 0.00 0.00 3.50 8.00 9.00  
 \*  
 \*

2217 KK 78C BASIN  
 2218 KM BASIN 78C  
 2219 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 2220 KM L=0.50 Lca=0.30 S=31.8 Kn=0.048 LAG=17.4

EMDU89.txt  
 2221 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 2222 BA 0.288  
 2223 LG 0.18 0.15 7.60 0.14 6  
 2224 UI 80 273 428 624 405 236 96 48 17 16  
 2225 UI 0 0 0 0 0 0 0 0 0 0

2226 KK REF78C DIVERT  
 2227 KM RETAIN 80% OF THE 100 YR 2 HR RUNOFF VOLUME  
 2228 DT 78CRET 2.2 0.0  
 2229 DI 0 10000  
 2230 DQ 0 10000

2231 KK C78C COMBINE  
 2232 KM COMBINE HYDROGRAPHS 78BTC AND 78C.  
 2233 HC 2

2234 KK C78C2 COMBINE  
 2235 KM COMBINE HYDROGRAPHS 77CT78 AND C78C.  
 \* KO 2  
 2236 HC 2

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

2237 KK 78CT79 ROUTE  
 2238 KM ROUTE 78C TO 79A FROM SIGNAL BUTTE ROAD TO THE PROPERTY BOUNDARY APPROXIMATELY  
 2239 KM 1/4 MILE TO THE WEST OF SIGNAL BUTTE ROAD VIA ENGINEERED CHANNEL.  
 2240 RS 2 FLOW -1  
 2241 RC 0.032 0.032 0.032 4215 0.0033 0.00  
 2242 RX 0.00 5.00 10.00 26.00 81.00 97.00 102.00 107.00  
 2243 RY 5.00 4.50 4.00 0.00 0.00 4.00 4.50 5.00

\* \*\*\*\*\*UPDATED EM PLANNING \*\*\*\*\*  
 \* \*\*\*\*\*DES 01/18/2013\*\*\*\*\*  
 \*

2244 KK 20 BASIN  
 2245 KM BASIN 20  
 2246 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 2247 KM L=1.02 Lca=0.45 S=17.6 Kn=0.041 LAG=25.5  
 2248 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 2249 BA 0.306  
 2250 LG 0.21 0.15 8.00 0.11 31  
 2251 UI 0 40 132 217 290 456 408 299 217 131  
 2252 UI 69 46 24 12 12 12 0 0 0 0  
 2253 UI 0 0 0 0 0 0 0 0 0 0  
 2254 UI 0 0 0 0 0 0 0 0 0 0  
 2255 UI 0 0 0 0 0 0 0 0 0 0

2256 KK RET20 DIVERT  
 2257 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 2258 DT 20CRET 24.3 0.0  
 2259 DI 0 10000  
 2260 DQ 0 10000

2261 KK CP22B COMBINE  
 2262 KM COMBINE HYDROGRAPHS 78CT79 AND RET20  
 \* KO 2  
 2263 HC 2

\* \*\*\*\*\*UPDATED EM PLANNING \*\*\*\*\*  
 \* \*\*\*\*\*DES 01/18/2013\*\*\*\*\*  
 \*

2264 KK 16 BASIN  
 2265 KM BASIN 16  
 2266 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 2267 KM L=0.44 Lca=0.21 S=34.1 Kn=0.044 LAG=13.1  
 2268 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 2269 BA 0.106  
 2270 LG 0.22 0.17 6.80 0.17 33  
 2271 UI 0 56 167 292 184 77 29 8 0 0  
 2272 UI 0 0 0 0 0 0 0 0 0 0

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

2273 UI 0 0 0 0 0 0 0 0 0 0  
 2274 UI 0 0 0 0 0 0 0 0 0 0  
 2275 UI 0 0 0 0 0 0 0 0 0 0

2276 KK RET16 DIVERT  
 2277 KM RETAIN 100 YR 2 HR RUNOFF VOLUME

ENDU89.txt

2278 DT 16RET 8.4 0.0  
 2279 DI 0 10000  
 2280 DQ 0 10000  
 \*  
 \*

2281 KK 18 BASIN  
 2282 KM BASIN 18  
 2283 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 2284 KM L=0.72 Lca=0.33 S=20.8 Kn=0.041 LAG=19.2  
 2285 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 2286 BA 0.320  
 2287 LG 0.21 0.25 5.80 0.25 31  
 2288 UI 0 71 251 383 634 486 321 165 83 40  
 2289 UI 17 17 0 0 0 0 0 0 0 0  
 2290 UI 0 0 0 0 0 0 0 0 0 0  
 2291 UI 0 0 0 0 0 0 0 0 0 0  
 2292 UI 0 0 0 0 0 0 0 0 0 0  
 \*  
 \*

2293 KK RET18 DIVERT  
 2294 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 2295 DT 18RET 25.4 0.0  
 2296 DI 0 10000  
 2297 DQ 0 10000  
 \*  
 \*

2298 KK 18T19 ROUTE  
 2299 KM ROUTE FLOW FROM BASIN 18 TO BASIN 19  
 2300 RS 4 FLOW -1  
 2301 RC 0.013 0.013 0.013 1150 .0020  
 2302 RX 0 15 17 38 112 130 132 146  
 2303 RY 4 0 0 3 3 0 0 4  
 \*  
 \*

2304 KK CP19A COMBINE  
 2305 KM COMBINE HYDROGRAPHS RET16 AND 18T19  
 \* KO 2  
 2306 HC 2  
 \*  
 \*

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1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

2307 KK 19 BASIN  
 2308 KM BASIN 19  
 2309 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 2310 KM L=0.50 Lca=0.20 S=20.0 Kn=0.041 LAG=13.9  
 2311 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 2312 BA 0.102  
 2313 LG 0.21 0.15 8.40 0.09 30  
 2314 UI 0 47 144 262 188 93 34 11 8 0  
 2315 UI 0 0 0 0 0 0 0 0 0 0  
 2316 UI 0 0 0 0 0 0 0 0 0 0  
 2317 UI 0 0 0 0 0 0 0 0 0 0  
 2318 UI 0 0 0 0 0 0 0 0 0 0  
 \*  
 \*

2319 KK RET19 DIVERT  
 2320 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 \* KO 2  
 2321 DT 19RET 8.1 0.0  
 2322 DI 0 10000  
 2323 DQ 0 10000  
 \*  
 \*

2324 KK CP19B COMBINE  
 2325 KM COMBINE HYDROGRAPHS CP19A AND RET20.  
 2326 HC 2  
 \*  
 \*

2327 KK 17 BASIN  
 2328 KM BASIN 17  
 2329 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 2330 KM L=0.92 Lca=0.47 S=19.6 Kn=0.045 LAG=26.8  
 2331 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 2332 BA 0.134  
 2333 LG 0.25 0.25 4.10 0.55 30  
 2334 UI 0 17 51 88 115 171 190 137 101 71  
 2335 UI 35 25 17 5 5 5 5 0 0 0  
 2336 UI 0 0 0 0 0 0 0 0 0 0  
 2337 UI 0 0 0 0 0 0 0 0 0 0  
 2338 UI 0 0 0 0 0 0 0 0 0 0  
 \*  
 \*

2339 KK RET17 DIVERT  
 2340 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 \* KO 2  
 2341 DT 17RET 10.1 0.0  
 2342 DI 0 10000

2343 DQ 0 10000

\* .....  
\*

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

2344 KK 79A BASIN  
2345 KM BASIN 79A  
2346 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
2347 KM L=1.43 Lca=0.82 S=14.7 Kn=0.090 LAG=82.6  
2348 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
2349 BA 1.067  
2350 LG 0.10 0.15 7.60 0.14 0 63 146 167 203 224 240  
2351 UI 0 43 44 43 63 146 167 203 224 240  
2352 UI 262 288 316 346 408 489 565 524 446 400  
2353 UI 372 340 305 278 255 223 209 178 140 110  
2354 UI 76 76 72 71 47 44 44 42 13 13  
2355 UI 14 13 13 14 13 13 14 13 13 14

2356 KK CP79A1 COMBINE  
2357 KM COMBINE HYDROGRAPHS RET17, 79A, CP22B, AND CP19B.  
\* KO 2  
2358 HC 4

2359 KK 78F  
2360 KM BASIN 78F  
2361 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
2362 KM L= 3.7 Lca= 2.1 S= 29.8 Kn= .090 LAG= 147.0  
2363 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
2364 BA 4.19  
2365 LG .35 .36 5.00 .27 .00 96. 148. 315. 325. 368.  
2366 UI 96. 96. 96. 96. 96. 541. 566. 595. 626. 663.  
2367 UI 399. 442. 468. 495. 521. 541. 566. 595. 626. 663.  
2368 UI 708. 737. 774. 845. 936. 1085. 1119. 1276. 1239. 1127.  
2369 UI 1033. 968. 907. 862. 820. 782. 751. 712. 667. 629.  
2370 UI 597. 570. 541. 502. 472. 454. 436. 368. 339. 276.  
2371 UI 276. 174. 170. 170. 168. 158. 158. 156. 96.  
2372 UI 96. 96. 96. 96. 96. 79. 29. 29. 29.  
2373 UI 29. 29. 29. 29. 29. 29. 29. 29. 29.  
2374 UI 29. 29. 29. 29. 29. 29. 29. 0. 0.  
2375 UI 0. 0. 0. 0. 0. 0. 0. 0. 0.

2376 KK 82A1  
2377 KM BASIN 82A1  
2378 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
2379 KM L= 3.6 Lca= .9 S= 33.9 Kn= .090 LAG= 103.0  
2380 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
2381 BA 3.12  
2382 LG .35 .36 5.00 .27 .00 335. 381. 436. 485. 526.  
2383 UI 102. 102. 102. 102. 163. 335. 381. 436. 485. 526.  
2384 UI 564. 599. 644. 693. 757. 801. 901. 1048. 1222. 1356.  
2385 UI 1223. 1084. 987. 913. 852. 800. 742. 675. 629. 589.  
2386 UI 529. 494. 459. 391. 304. 266. 180. 180. 173. 167.  
2387 UI 167. 123. 102. 102. 102. 36. 31. 31. 31.

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

2388 UI 31. 31. 31. 31. 31. 31. 31. 31. 31. 31.  
2389 UI 31. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
2390 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

2391 KK C02A1  
2392 KM COMBINE FLOWS FROM SUBBASINS 78F AND 82A1 NORTH OF PECOS ROAD AT  
2393 KM NEW DETENTION BASIN  
2394 HC 2

2395 KK DBB2A1  
2396 KM PECOS NORTH BASIN  
2397 KM NEW DETENTION BASIN LOCATED EAST OF MERIDIAN ROAD & 660' NORTH OF PECOS RD.  
2398 KM WITH 1'-42" RCP OUTLET & 86' SPILLWAY AT ELEV=44  
\* KO 2 2  
2399 RS 1 STOR 0  
2400 SV 0 8 34 73 113 153 195 237 280 346  
2401 SE 36 37 38 39 40 41 42 43 44 46.1  
2402 SL 33.5 9.6 .62 .5  
2403 SS 44 195 3 1.5

2404 KK PS-9  
2405 KM REACH PS-9  
2406 KM OUTFLOW CHANNEL FROM NEW DETENTION BASIN 82A TO MAIN CHANNEL @ PECOS  
2407 RS 1 FLOW -1  
2408 RC .025 .025 .025 500 .0005 72 80 88  
2409 RX 0 8 16 42 46

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EMDU89.txt
2410 RY 4.1 4.2 4.3 0 0 4.3 4.2 4.1
*
*
2411 KK CAP2
2412 KM INFLOW FROM EAST OF THE CAP THROUGH 1 - 36" PIPE OVERCRUTE
2413 KM STATION #536+00 SALT-GILA AQUEDUCT REACH 2
2414 KM QI CARDS BASED ON OVERCRUTE CAPACITY OF 64 CFS
2415 IN 60
2416 BA .01
2417 QI 0 20 64 64 64 64 64 64 64 64
2418 QI 64 64 64 64 64 64 64 64 64 64
2419 QI 64 64 64 64 64 64 64 64 64 64
*
*
2420 KK RCAP2
2421 KM ROUTE CAP2 THROUGH 82A2 VIA WASH TO SUBBASIN 82A2
2422 IN 15
2423 RS 11 FLOW -1
2424 RC .045 .04 .045 24000 .05
2425 RX 0 500 1000 1010 1020 1030 1530 2030
2426 RY 8 5 3 0 0 3 5 8
*
*

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HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

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2427 KK 82A2
2428 KM BASIN 82A2
2429 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
2430 KM L= 4.6 Lca= 2.9 S= 27.2 Kn= .089 LAG= 183.0
2431 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
2432 BA 4.13
2433 LG .35 .36 5.00 .27 1.00
2434 UI 76. 76. 76. 76. 76. 76. 76. 76. 177. 249.
2435 UI 250. 291. 291. 339. 349. 371. 381. 406. 420. 430.
2436 UI 448. 466. 483. 503. 529. 561. 578. 596. 631. 680.
2437 UI 743. 805. 911. 923. 1027. 979. 901. 845. 794. 751.
2438 UI 717. 686. 659. 635. 613. 594. 570. 540. 514. 487.
2439 UI 470. 455. 437. 407. 393. 372. 362. 349. 315. 291.
2440 UI 263. 218. 218. 179. 134. 134. 134. 134. 128. 125.
2441 UI 125. 125. 125. 96. 76. 76. 76. 76. 76. 76.
2442 UI 76. 67. 23. 23. 23. 23. 23. 23. 23. 23.
2443 UI 23. 23. 23. 23. 23. 23. 23. 23. 23. 23.
2444 UI 23. 23. 23. 23. 23. 23. 23. 23. 23. 0.
2445 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
2446 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
*
*

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2447 XX C82A2
2448 KM COMBINE FLOW FROM ROUTED CAP2 AND SUBBASIN 82A2
2449 HC 2
*
*

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2450 KK 82A4
2451 KM BASIN 82A4
2452 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
2453 KM L= 3.5 Lca= 1.5 S= 29.1 Kn= .090 LAG= 128.0
2454 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
2455 BA 2.13
2456 LG .35 .36 5.00 .27 .00
2457 UI 56. 56. 56. 56. 56. 70. 184. 193. 214. 248.
2458 UI 265. 281. 302. 316. 332. 352. 374. 401. 424. 447.
2459 UI 494. 557. 650. 691. 742. 667. 606. 560. 523. 490.
2460 UI 466. 443. 419. 389. 364. 343. 325. 299. 281. 267.
2461 UI 251. 214. 182. 161. 126. 99. 99. 97. 92. 92.
2462 UI 92. 69. 56. 56. 56. 56. 56. 27. 17. 17.
2463 UI 17. 17. 17. 17. 17. 17. 17. 17. 17. 17.
2464 UI 17. 17. 17. 17. 17. 17. 0. 0. 0. 0.
2465 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
*
*

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2466 KK 82A4T3
2467 KM REACH MN-2
2468 KM ROUTE FLOW FROM SUBBASIN 82A4 TO DETENTION BASIN 82A3
2469 RS 2 FLOW -1
2470 RC .025 .025 .025 1050 .0005
2471 RX 0 8 16 47 107 138 146 154
*
*

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HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

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2472 RY 5.0 5.1 5.2 0 0 5.2 5.1 5.0
*
*
2473 KK 82A3
2474 KM BASIN 82A3
2475 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN
2476 KM L= 3.6 Lca= 2.0 S= 28.3 Kn= .090 LAG= 145.0
2477 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN
2478 BA 2.02
2479 LG .35 .36 5.00 .27 .00

```



	UI	47.	47.	47.	47.	47.	47.	82.	154.	162.	180.
2480	UI	200.	218.	230.	246.	257.	268.	280.	294.	310.	330.
2481	UI	351.	365.	387.	425.	477.	553.	566.	633.	583.	529.
2482	UI	492.	460.	434.	410.	392.	376.	361.	336.	316.	299.
2483	UI	284.	271.	250.	237.	228.	216.	190.	180.	136.	135.
2484	UI	101.	83.	83.	83.	78.	77.	77.	77.	52.	47.
2485	UI	47.	47.	47.	47.	42.	14.	14.	14.	14.	14.
2486	UI	14.	14.	14.	14.	14.	14.	14.	14.	14.	14.
2487	UI	14.	14.	14.	14.	14.	14.	14.	14.	14.	14.
2488	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
2489	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.

EMD089.txt

2490 KK CP82A3  
 2491 KM COMBINE FLOW FROM SUBBASIN 82A4 AND SUBBASIN 82A3 BEFORE DETENTION BASIN  
 2492 KC 2

2493 KK CP82A5  
 2494 KM COMBINE FLOWS FROM CAP OVERCHUTE AND SUBBASIN 82A  
 2495 KC 2

2496 KK DB82B  
 2497 KM PECOS SOUTH BASIN  
 2498 KM NEW DETENTION BASIN LOCATED EAST OF MERIDIAN ROAD & 660' SOUTH OF PECOS RD.  
 2499 KM WITH 1-66" RCP OUTLET & 80' SPILLWAY AT ELEV 41

	NO	2	2	0	0	25.7	62.7	110.5	158.7	207.5	257.3	320
2500	RS	1	STOR	0								
2501	SV	0	3.5	9.6	25.7	62.7	110.5	158.7	207.5	257.3	320	
2502	SE	31.5	33	34	35	36	37	38	39	40	42.1	
2503	SL	33	23.7	.62	.5							
2504	SS	41	80	3	1.5							

2505 KK MN-1  
 2506 KM REACH MN-1 plus culvert PSC-7  
 2507 KM ROUTE FLOW FROM NEW DETENTION BASIN 82B TO MAIN LINE CHANNEL @ PECOS

	RS	2	FLOW	-1	1030	.0005					
2508	RC	.025	.025	.025	1030	.0005					
2509	RX	0	8	16	47	55	87	95	103		
2510	RX										

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

2511 RY 5.1 5.2 5.3 0 0 5.3 5.2 5.1

2512 KK CP82A6  
 2513 KM COMBINE FLOWS AFTER DETENTION BASINS.  
 2514 KC 2

2515 KK 82TBOX  
 2516 KM REACH PS-9  
 2517 KM ROUTE FLOW FROM DETENTION BASIN DB82B TO 1000' FOOT LONG BOX CULVERT (PSC-6).

	RS	1	FLOW	-1	750	.0005				
2518	RC	.025	.025	.025	750	.0005				
2519	RX	0	8	16	47.2	67	98	106	114	
2520	RY	5.0	5.1	5.2	0	0	5.2	5.1	5.0	

2522 KK BOXCLV  
 2523 KM REACH PSC-5  
 2524 KM ROUTE FLOW THROUGH BOX CULVERT

	RS	1	FLOW	-1	1000	.0020				
2525	RC	.015	.012	.015	1000	.0020				
2526	RX	0	8	16	16.01	28.01	28.02	36	44	
2527	RY	4.8	4.9	5	0	0	5	4.9	4.8	

2529 KK BOXT78  
 2530 KM REACH PS-5, PS-6, PS-7 plus culverts PSC-5 & PSC-4  
 2531 KM ROUTE FLOW FROM 1000' BOX CULVERT TO C78D (SIGNAL BUTTE ROAD)

	RS	3	FLOW	-1	3400	.0005				
2532	RC	.025	.025	.025	3400	.0005				
2533	RX	0	8	16	47	67	98	106	114	
2534	RY	5.4	5.3	5.2	0	0	5.2	5.3	5.4	

2536 KK 78D  
 2537 KM BASIN 78D  
 2538 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 2539 KM L= 1.2 Lca= .5 S= .21.7 Ka= .030 LAG= 19.5  
 2540 KM PHOENIX VALLEY S-GRAFH WAS USED FOR THIS BASIN

	BA	.89	.15	8.00	.11	55.00					
2541	LG	.89	.15	8.00	.11	55.00					
2542	UI	189.	678.	1029.	1713.	1367.	909.	496.	240.	123.	47.
2543	UI	47.	0.	0.	0.	0.	0.	0.	0.	0.	0.
2544	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
2545	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

2546 KK R78D  
 2547 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 2548 DT D78D 84  
 2549 DI 0 10000  
 2550 DQ 0 10000  
 \*

2551 KK 82B  
 2552 KM BASIN 82B  
 2553 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 2554 KM L= .9 Lca= .4 S= 21.2 Kn= .030 LAG= 17.2  
 2555 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 2556 BA .92  
 2557 LG .15 .25 5.00 .36 55.00  
 2558 UI 266. 879. 1420. 2004. 1268. 727. 294. 142. 55. 55.  
 2559 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 2560 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 \*

2561 KK R82  
 2562 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 2563 DT D82 1  
 2564 DI 0 10000  
 2565 DQ 0 10000  
 \*

2566 KK DITRM  
 2567 KM DIVERTING 110.7 ACRE-FEET DUE TO ON-SITE RETENTION  
 2568 KM VOLUMES WERE DERIVED FROM DRAINAGE REPORT - REFERENCE 7.  
 2569 DT TRM 110.7  
 2570 DI 0 10000  
 2571 DQ 0 10000  
 \*

2572 KK C78D  
 2573 KM COMBINE FLOWS FROM 78D, 82B AND ROUTED FLOW 82T78D  
 2574 KM @ PECOS ROAD AND SIGNAL BUTTE ROAD.  
 2575 HC 3  
 \*

2576 KK 78DTE  
 2577 KM REACH PS-2, PS-3, PS-4 plus culverts PSC-3 AND PSC-2.  
 2578 KM ROUTE FLOWS FROM 78D (PECOS RD AND SIGNAL BUTTE RD) TO 78E (PECOS AND CRISMON  
 2579 RS 11 FLOW -1  
 2580 RC .025 0.025 0.025 5100 .0005  
 2581 RX 0 8 16 53.2 93.2 130.4 138.4 146.4  
 2582 RY 6.0 6.1 6.2 0 0 6.3 6.1 6.0  
 \*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

2583 KK 78E  
 2584 KM BASIN 78E  
 2585 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 2586 KM L= 1.1 Lca= .5 S= 17.4 Kn= .087 LAG= 57.4  
 2587 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 2588 BA 1.01  
 2589 LG .35 .26 8.80 .06 1.00  
 2590 UI 59. 59. 108. 212. 271. 313. 351. 402. 460. 565.  
 2591 UI 741. 698. 577. 502. 447. 382. 335. 289. 241. 166.  
 2592 UI 104. 100. 97. 60. 59. 45. 18. 18. 18. 18.  
 2593 UI 18. 18. 18. 18. 0. 0. 0. 0. 0. 0.  
 2594 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 \*

2595 KK 83  
 2596 KM BASIN 83  
 2597 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 2598 KM L= 2.0 Lca= .5 S= 15.0 Kn= .030 LAG= 25.8  
 2599 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 2600 BA 1.01  
 2601 LG .15 .25 5.00 .36 55.00  
 2602 UI 131. 423. 700. 931. 1454. 1362. 993. 726. 464. 226.  
 2603 UI 161. 89. 40. 40. 40. 0. 0. 0. 0. 0.  
 2604 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 \*

2605 KK R83  
 2606 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 2607 DT D83 83  
 2608 DI 0 10000  
 2609 DQ 0 10000  
 \*

2610 KK C78E  
 2611 KM COMBINE FLOWS FROM 78D AND 78E (CRISMON ROAD)  
 2612 HC 3  
 \*

2613 KK 78ET84  
 2614 KM REACH PS-1  
 2615 KM ROUTE FLOWS WEST ALONG PECOS IN A PROPOSED CHANNEL  
 2616 KM ROUTE FLOWS FROM CRISMON ROAD TO ELLSWORTH ROAD.  
 2617 RS 4 FLOW -1  
 2618 RC .025 0.025 0.025 4840 .0005  
 2619 RX 0 8 16 53.2 93.2 130.4 138.4 146.4  
 2620 RY 6.0 6.1 6.2 0 0 6.2 6.1 6.0  
 \*

HEC-1 INPUT

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

2621 KK 84  
 2622 KM BASIN 84  
 2623 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 2624 KM L= 2.0 Lca= .5 S= 12.5 Kn= .030 LAG= 26.7  
 2625 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 2626 BA .99  
 2627 LG .15 .25 4.70 .40 55.00  
 2628 UI 125. 380. 651. 855. 1278. 1399. 1008. 747. 520. 258.  
 2629 UI 182. 120. 38. 38. 38. 0. 0. 0. 0. 0.  
 2630 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 \*

2631 KK R84  
 2632 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 2633 DT D84 85  
 2634 D1 0 10000  
 2635 DQ 0 10000  
 \*

2636 KK C84  
 2637 KM COMBINE FLOWS FROM 78E AND 84 AT ELLSWORTH AND PECOS ROAD  
 2638 KM CHANNEL EAST SIDE OF GATEWAY WILLIAMS FLOWING TO THE NORTH  
 2639 HC 2  
 \*

2640 KK 84T79B  
 2641 KM REACH EH-3B  
 2642 KM ROUTE FLOWS FROM THE CORNER OF PECOS AND ELLSWORTH ROADS TO  
 2643 KM THE SOUTH OF WILLIAMS FIELD ROAD AND ELLSWORTH ROAD  
 2644 KM (THIS IS THE COMBINE POINT FROM BASIN 79B)  
 2645 RS 1 FLOW -1  
 2646 RC .015 .015 .015 3383 .0010  
 2647 RX 0 8 16 30 55 69 77 85  
 2648 RY 6.7 6.8 6.9 0 0 6.9 6.8 6.7  
 \*

2649 KK 79B  
 2650 KM BASIN 79B  
 2651 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 2652 KM L= 1.4 Lca= .6 S= 9.0 Kn= .090 LAG= 77.7  
 2653 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 2654 BA 1.00  
 2655 LG .35 .25 9.70 .05 .00  
 2656 UI 43. 43. 43. 85. 150. 179. 207. 232. 250. 275.  
 2657 UI 306. 335. 388. 479. 557. 515. 444. 396. 360. 331.  
 2658 UI 295. 267. 240. 216. 192. 155. 123. 76. 76. 71.  
 2659 UI 71. 48. 43. 43. 36. 13. 13. 13. 13. 13.  
 2660 UI 13. 13. 13. 13. 13. 13. 0. 0. 0. 0.  
 2661 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 \*

HEC-1 INPUT

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

2662 KK C79B1  
 2663 KM FLOWS FROM SOUTH CHANNEL ALONG ELLSWORTH ROAD.  
 2664 HC 2  
 \*

2665 KK 79BT82  
 2666 KM REACH EH-3A  
 2667 KM ROUTE FLOWS FROM THE COMBINE POINT OF SUB-BASIN 79B TO  
 2668 KM WILLIAMS FIELD ROAD AND ELLSWORTH ROAD  
 2669 RS 1 FLOW -1  
 2670 RC .025 .015 .025 5000 .0010  
 2671 RX 0 8 16 30 55 69 77 85  
 2672 RY 6.7 6.8 6.9 0 0 6.9 6.8 6.7  
 \*

2673 KK C79B2  
 2674 KM COMBINE 79A AND ROUTED 79B (WHICH IS HYDROGRAPH C79B1)  
 2675 HC 2  
 \*  
 \*  
 2676 KK 79FPC2  
 2677 KM REACH EH-1, EH-2, plus culvert EHC-1  
 2678 KM ROUTE FLOWS THROUGH WILLIAMS-GATEWAY (SUBBASIN 80A) BY WAY OF NEW NORTH  
 2679 KM PERIMETER CHANNEL ABOUT 1/2 MILE WEST OF ELLSWORTH ROAD  
 2680 RS 2 FLOW -1  
 2681 RC .025 .015 .025 4760 .0014  
 2682 RX 0 8 16 33 61 78 86 94  
 2683 RY 8.4 8.5 8.4 0 0 8.4 8.5 8.4  
 \*  
 \*

2684 KK CFPWR  
 2685 KM COMBINE FLOWS FROM 75 AND 79 IN THE POWERLINE FLOODWAY ALONG RAY ROAD  
 2686 KM AT ABOUT 1/2 MILE WEST OF ELLSWORTH ROAD  
 \* KO 2  
 \* HC 2  
 \*  
 \*  
 \* KK\*DBPWR  
 \* KM DIVERT FROM POWERLINE CHANNEL INTO THE RAY DETENTION BASIN  
 \* KM USES A REALISTIC SIDE-WEIR EQUATION TO FORM POWER CURVE  
 \* KM WEIR CREST = 3.25FT; WEIR LENGTH = 750; 4.0FT DIV STRUCTURE.  
 \* KO 3  
 \* DI PWRDB 1537  
 \* DI 0 528 544 576 628 704 955 5730  
 \* DQ 0 0 11 40 89 162 410 5174  
 \*

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

2688 KK PWR180  
 2689 KM REACH FR-3, FR-4, plus culvert PRC-2  
 2690 KM ROUTE FLOWS FROM PLF COMBINE TO CATCH POINT AT 80A VIA PLF IMPROVEMENT  
 2691 RS 1 FLOW -1  
 2692 RC .025 .015 .025 3680 .0014  
 2693 RX 0 8 16 34 62 79 87 95  
 2694 RY 8.5 8.6 8.7 0 0 8.7 8.6 8.5  
 \*  
 \*

2695 KK 80A  
 2696 KM BASIN 80A  
 2697 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 2698 KM L= 3.8 Lca= 2.2 S= 14.2 Kn= .030 LAG= 58.2  
 2699 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 2700 BA 2.64  
 2701 LG .15 .15 9.70 .06 55.00  
 2702 UI 153. 153. 265. 544. 690. 802. 899. 1020. 1171. 1421.  
 2703 UI 1851. 1871. 1534. 1330. 1185. 1024. 896. 770. 653. 479.  
 2704 UI 305. 265. 251. 187. 153. 153. 50. 47. 47. 47.  
 2705 UI 47. 47. 47. 47. 47. 0. 0. 0. 0. 0.  
 2706 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 \*  
 \*

2707 KK R80A  
 2708 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 2709 DI D80A 33  
 2710 DI 0 10000  
 2711 DQ 0 10000  
 \*  
 \* THE SECTION BELOW COMBINES THE ROUTED FLOW FROM SANTAN AND POWER  
 \* THEN ROUTES THRU THE RAY DB  
 \* KKDRSND  
 \* KM RETURNS THE DIVERSION FROM THE SANTAN CHANNEL  
 \* DR SANDB  
 \*  
 \* KKDRPWB  
 \* KM RETURNS THE DIVERSION FROM THE POWERLINE CHANNEL  
 \* DR PNRDB  
 \*  
 \* KK CPRAY  
 \* KM COMBINES THE TWO ROUTED FLOWS IN THE RAY DETENTION BASIN  
 \* HC 2 1  
 \*  
 \* KK\*TRAY  
 \* KM ROUTES THE HYDROGRAPH OUT OF THE RAY BASIN  
 \* KM Currently incorporates a dummy set of outflow data  
 \* KO 1  
 \* RS 1 STOR -1  
 \* SV 0 244 493 747 1005 1269 1537  
 \* SE 0 1.54 3.08 4.63 6.17 7.71 9.25  
 \* SQ 0 1 2 3 4 5 6  
 \*  
 \*

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

2712 KK CFS0A  
 2713 KM COMBINE FLOW IN THE POWERLINE FLOODWAY WITH FLOW COMING FROM SUBBASIN 80A  
 2714 KM THE LOCATION FOR THIS COMBINATION AT THE NW CORNER OF SUBBASIN 80A  
 \* KO 2  
 2715 HC 2  
 \* HC 3  
 \*

2716 KK FWRASN  
 2717 KM REACH PR-1, PR-2 plus culvert PRC-1  
 2718 KM ROUTE FLOWS FROM COMBINE POINT AT 80A VIA PLF RE-ALIGNMENT.  
 2719 RS 1 FLOW -1  
 2720 RC .015 .015 .015 3500 .0014  
 2721 RX 0 8 16 74 154 212 220 228  
 2722 RY 9.5 9.6 9.7 0 0 9.7 9.6 9.5  
 \*

2723 KK CFSAN  
 2724 KM COMBINE FLOWS FROM THE POWERLINE FLOODWAY AND THE PROPOSED SANTAN FREEWAY  
 2725 KM DRAINAGE CHANNEL  
 \* KO 2  
 2726 HC 2  
 \*

\* \*\*\*\*REMOVED BY CPE IN JUNE 2000 IN FAVOR OF SANDS AND PWRDB  
 \* \*\*\*\*DIVERSIONS LOCATED UPSTREAM.  
 \* KK\*DBRAY  
 \* KM BASIN TO WITHDRAW FLOW FROM POWERLINE FLOODWAY  
 \* KO 3  
 \* DI RAYDB 1200  
 \* DI 0 1000 1001 1500 4500 14500  
 \* DQ 0 0 1 500 3500 13500  
 \*

2727 KK PWRMF  
 2728 KM ROUTE FLOW FROM RE-ALIGNED POWERLINE FLOODWAY TO EMF VIA  
 2729 KM NEW CHANNEL ALONG SANTAN FREEWAY ALIGNMENT  
 2730 KM REACH ET-1  
 \* KO 21  
 2731 RS 3 FLOW -1  
 2732 RC .025 .025 .025 3850 .0005  
 2733 RX 0 8 16 74 154 212 220 228  
 2734 RY 9.5 9.6 9.7 0 0 9.7 9.6 9.5  
 \*

HEC-1 INPUT

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

2735 KK EMFP0W  
 2736 KM COMBINE FLOW FROM THE POWERLINE FLOODWAY WITH FLOW IN THE EMF  
 \* KO 2  
 2737 HC 2  
 \*

2738 KK F0WTH1  
 2739 KM ROUTE EMF FLOW TO WILLIAMS FIELD ROAD VIA THE EMF  
 2740 KM THIS SECTION IS CONCRETE LINED TO PAST POWER ROAD BRIDGE  
 2741 RS 2 FLOW -1  
 2742 RC .03 .012 .03 4750 .0003  
 2743 RX 0 500 520 553 693 726 740 742  
 2744 RY 14 12 11 0 0 11 11 12  
 \*

2745 KK 80B  
 2746 KM BASIN 80B  
 2747 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 2748 KM L= 1.5 Lca= .9 S= 18.4 Km= .044 LAG= 41.9  
 2749 KM PHOENIX VALLEY S-CURVE WAS USED FOR THIS BASIN  
 \* KO 21  
 2750 BA 1.12  
 2751 LG .13 .17 6.80 .18 48.00  
 2752 UI 90. 105. 319. 433. 515. 614. 754. 1063. 1032. 814.  
 2753 UI 687. 563. 464. 359. 213. 155. 136. 90. 73. 28.  
 2754 UI 28. 28. 28. 28. 28. 0. 0. 0. 0. 0.  
 2755 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 \*

2756 KK R80B  
 2757 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 \* KO 21  
 2758 DI D80B 4  
 2759 DI 0 10000  
 2760 DQ 0 10000  
 \*

\* Subbasin 81B routed to EMFWL per discussions with the FCOMC as part of the  
 \* Chandler Heights/Rittenhouse Basin Design Project. QAZ  
 \*

2761 KK 81B

EMD089.txt

2762 KM BASIN 81B  
 2763 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 2764 KM L= 1.1 Lca= .4 S= 6.9 Kn= .033 LAG= 24.7  
 2765 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 2766 BA .84  
 2767 LG .10 .25 4.70 .45 67.00  
 2768 UI 115. 393. 631. 857. 1343. 1088. 796. 566. 296. 185.  
 2769 UI 115. 39. 35. 35. 0. 0. 0. 0. 0. 0.

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HEC-1 INPUT  
 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10  
 2770 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

2771 KK R81B  
 2772 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 2773 KO 3 21  
 2774 DI D81B 35  
 2775 DI 0 10000  
 2776 DQ 0 10000  
 \*  
 \* B1B to be combined with 80B and exported to EMF routing model by 80B81B  
 \* EMFWIL to combine 80B81B with flow from POWTWI for this model  
 \* Q42  
 \*  
 \*

2777 KK 80B81B  
 2778 KM COMBINE FLOWS FROM 80B & 81B AND EXPORT TO ROUTING MODEL  
 2779 KO 21  
 2780 HC 2  
 \*  
 \*

2781 KK EMFWIL  
 2782 KM COMBINE FLOWS INTO THE EMF WEST OF WILLIAMS AFB FROM 80B, 81B, EMF POWERLINE  
 \* KO 2  
 2783 HC 2  
 \*  
 \*

2784 KK WILTSP  
 2785 KM ROUTE EMF FLOW FROM WILLIAMS FIELD ROAD TO THE SOUTHERN PACIFIC RAILROAD  
 2786 KM (AT RITTENHOUSE ROAD)  
 2787 RS 3 FLOW -1  
 2788 RC .03 .022 .03 5000 .0003  
 2789 RX 0 500 520 553 693 726 740 742  
 2790 RY 14 12 11 0 0 11 11 12

\*  
 \* KKEMFRT1 Hydrograph name changed by Dibble & Associates to avoid two  
 \* KM different hydrographs with the same name.  
 \* KM COMBINE 81A & 81B AND RITTENHOUSE(HYDROGRAPH WILTSP, FROM EMFWIL)  
 \* HC 2  
 \*  
 \*  
 \* THE NEXT KK BLOCKS COME FROM THE QUEEN CREEK ADMS  
 \*  
 \* \*\*\*\*\* UPDATED TO GREEN-AMPT \*\*\*\*\*  
 \*

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HEC-1 INPUT  
 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

2791 KK SUB258  
 2792 KM PINAL COUNTY BASIN. PARAMETERS BASED ON EXISTING LAND-USE  
 2793 KM TO MODEL PINAL COUNTY'S PRE .V.G. POST DEVELOPMENT DRAINAGE CRITERIA  
 2794 KM BASIN 258  
 2795 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 2796 KM L= 4.6 Lca= 2.5 S= 24.9 Kn= .062 LAG= 122.0  
 2797 KM AGRICULTURAL S-GRAPH WAS USED FOR THIS BASIN  
 2798 BA 3.65  
 2799 LG .34 .17 4.70 .42 18.00  
 2800 UI 110. 110. 110. 110. 110. 217. 232. 299. 386. 574.  
 2801 UI 583. 717. 579. 644. 772. 916. 802. 927. 927. 927.  
 2802 UI 927. 927. 927. 927. 834. 772. 772. 772. 866. 865.  
 2803 UI 583. 579. 583. 650. 605. 456. 421. 421. 381. 331.  
 2804 UI 315. 309. 352. 309. 211. 211. 190. 178. 178. 159.  
 2805 UI 136. 136. 136. 125. 101. 101. 101. 101. 101. 74.  
 2806 UI 63. 63. 63. 63. 63. 63. 63. 63. 29. 14.  
 2807 UI 14. 14. 14. 14. 14. 14. 14. 14. 14. 14.  
 2808 UI 14. 14. 14. 14. 14. 14. 14. 14. 14. 14.  
 2809 UI 14. 14. 14. 14. 14. 14. 14. 14. 14. 14.  
 2810 UI 14. 14. 14. 14. 14. 14. 14. 14. 14. 0.  
 2811 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 2812 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

2813 KK R0259  
 2814 KM ROUTE SUB258 TO C0262  
 2815 KM 11 1.57 0.20  
 \*  
 \*  
 \* \*\*\*\*\* UPDATED TO GREEN-AMPT \*\*\*\*\*  
 \*

2816 KK SUB250  
 2817 KM MARICOPA COUNTY BASIN. PARAMETERS BASED ON FUTURE LAND-USE  
 2818 KM BASIN 250  
 2819 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 2820 KM L= 1.0 Lca= .5 S= 23.2 Kn= .045 LAG= 27.3  
 2821 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 2822 BA .98  
 2823 LG .27 .25 4.80 .36 24.00  
 2824 UI 121. 355. 623. 813. 1175. 1413. 1018. 764. 555. 284.  
 2825 UI 199. 121. 55. 37. 37. 37. 0. 0. 0. 0.  
 2826 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

\*  
 \* \*\*\*\*\* UPDATED \*\*\*\*\*  
 \*  
 \* NAME OF DIVERSION WAS CHANGED BECAUSE IT HAS THE SAME NAME AS ANOTHER  
 \* DIVERSION  
 \*  
 \* KKRETAIN

1

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

2827 KK R250  
 2828 KM 100-YR, 2HR RETENTION VOLUME FOR SUBBASIN LOCATED IN MARICOPA COUNTY  
 2829 DI RETDIV 69  
 2830 DI 0 10000  
 2831 DQ 0 10000

\*  
 \* \*\*\*\*\* PRESERVED \*\*\*\*\*  
 \*

2832 KK CO252  
 2833 KM COMBINE SUB250 AND RO259  
 2834 HC 2

2835 KK RO263  
 2836 KM ROUTE CO262 TO CO266  
 2837 RM 11 1.56 0.20

\*  
 \* \*\*\*\*\* UPDATED TO GREEN-AMPT \*\*\*\*\*  
 \*

2838 KK SUB264  
 2839 KM MARICOPA COUNTY BASIN. PARAMETERS BASED ON FUTURE LAND-USE  
 2840 KM BASIN 264  
 2841 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 2842 KM L= 1.0 Lca= .6 S= 20.0 Kn= .050 LAG= 32.9  
 2843 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 2844 BA 1.00  
 2845 LG .25 .25 4.70 .38 31.00  
 2846 UI 102. 217. 450. 584. 730. 1015. 1235. 921. 734. 572.  
 2847 UI 426. 228. 171. 115. 82. 31. 31. 31. 31. 0.  
 2848 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 2849 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

\*  
 \* \*\*\*\*\* UPDATED \*\*\*\*\*  
 \*  
 \* NAME OF DIVERSION WAS CHANGED BECAUSE IT HAS THE SAME NAME AS ANOTHER  
 \* DIVERSION  
 \*  
 \* KKRETAIN

2850 KK R264  
 2851 KM 100-YR, 2HR RETENTION VOLUME FOR SUBBASIN LOCATED IN MARICOPA COUNTY  
 2852 DI RETDIV 73  
 2853 DI 0 10000  
 2854 DQ 0 10000

\*  
 \* \*\*\*\*\* PRESERVED \*\*\*\*\*  
 \*

1

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

2855 KK CO266  
 2856 KM COMBINE SUB264 AND RO263  
 2857 HC 2

2858 KK RO267  
 2859 KM ROUTE CO266 TO CO270  
 2860 RM 11 3.31 0.20

\*  
 \* \*\*\*\*\* UPDATED TO GREEN-AMPT \*\*\*\*\*  
 \*

2861 KK SUB268  
 2862 KM MARICOPA COUNTY BASIN. PARAMETERS BASED ON FUTURE LAND-USE  
 2863 KM BASIN 268

EMDUB9.cxt  
 2864 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 2865 KM L= 2.0 Lca= 1.1 S= 13.4 Kn= .046 LAG= 55.1  
 2866 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 2867 BA .97  
 2868 LG .23 .25 4.65 .39 34.00  
 2869 UI 59. 59. 122. 219. 281. 324. 365. 422. 489. 637.  
 2870 UI 762. 634. 536. 474. 408. 355. 301. 255. 185. 113.  
 2871 UI 102. 97. 63. 59. 45. 18. 18. 18. 18. 18.  
 2872 UI 18. 18. 0. 0. 0. 0. 0. 0. 0. 0.  
 2873 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

\* \*\*\*\*\* UPDATED \*\*\*\*\*  
 \* DWM  
 \* NAME OF DIVERSION WAS CHANGED BECAUSE IT HAS THE SAME NAME AS ANOTHER  
 \* DIVERSION  
 \*  
 \* KKRETAIN

2874 KK R268  
 2875 KM 100-YR, 2HR RETENTION VOLUME FOR SUBBASIN LOCATED IN MARICOPA COUNTY  
 2876 DT RETDIV 68  
 2877 DI 0 10000  
 2878 DQ 0 10000

\* \*\*\*\*\* PRESERVED \*\*\*\*\*  
 \* DWM

2879 KK CO270  
 2880 KM COMBINE RUNOFF FROM R0267 AND SUB268  
 2881 HC 2

HEC-1 INPUT

1  
 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

2882 KK R0283  
 2883 KM ROUTE CO282 TO CONCENTRATION POINT AT QUEEN CREEK ROAD  
 2884 RM 11 2.78 0.20

\* THIS IS THE END OF THE QUEEN CREEK ADMS INSERT  
 \*  
 \*

2885 KK 88A  
 2886 KM BASIN 88A  
 2887 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 2888 KM L= .8 Lca= .2 S= 13.2 Kn= .020 LAG= 9.2  
 2889 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 2890 BA .50  
 2891 LG .10 .25 5.00 .40 80.00  
 2892 UI 549. 1709. 1208. 323. 71. 0. 0. 0. 0. 0.  
 2893 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

2894 KK R88A  
 2895 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 2896 DT D88A 50  
 2897 DI 0 10000  
 2898 DQ 0 10000

2899 KK 88AT89  
 2900 KM REACH RH-2b,RH-2a,RH-1,EXISTING CHANNEL (FCD 97-34), plus culvert RMC-1  
 2901 KM ROUTE 88A TO 89A VIA THE PROPOSED CHANNEL ALONG QUEEN CREEK ROAD  
 2902 KM FROM CRISMON ROAD TO ELLSWORTH ROAD  
 2903 RS 5 FLOW -1  
 2904 RC .025 .025 .025 5135 .0010  
 2905 RX 0 8 18 45 55 85 93 101  
 2906 RY 4.7 4.8 4.9 0 0 4.9 4.8 4.7

2907 KK 89A  
 2908 KM BASIN 89A  
 2909 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 2910 KM L= 1.0 Lca= .6 S= 19.0 Kn= .020 LAG= 13.5  
 2911 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 2912 BA .50  
 2913 LG .10 .25 4.65 .47 80.00  
 2914 UI 247. 742. 1328. 891. 408. 158. 41. 38. 0. 0.  
 2915 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

HEC-1 INPUT

1  
 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

2916 KK R89A  
 2917 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 2918 DT D89A 50  
 2919 DI 0 10000  
 2920 DQ 0 10000



2921 KK C89A  
 2922 KM COMBINE FLOWS FROM 88A AND 89A AT QUEEN CREEK ROAD AND ELLSWORTH ROAD  
 2923 HC 2  
 \*

2924 KK 89AIRI  
 2925 KM ROUTE 89A TO RITTENHOUSE ROAD VIA THE PROPOSED CHANNEL ALONG QUEEN CREEK ROAD  
 2926 KM FROM ELLSWORTH ROAD TO RITTENHOUSE ROAD  
 2927 KM REVISED 02.24.98 WITH RITTENHOUSE CHANNEL DESIGN.  
 2928 RS 5 FLOW -1  
 2929 RC .025 .025 .025 3145 .0005  
 2930 RX 0 10 26 39 49 72 78 100  
 2931 RI 12.2 9.7 5.7 0 0 5.7 9.7 12.2  
 \*

2932 KK C283  
 2933 KM COMBINE FLOWS FROM QUEEN CREEK ADMS AND EC ADNP AT QUEEN CREEK ROAD AND  
 2934 KM RITTENHOUSE ROAD.  
 \* KO 2  
 2935 HC 2  
 \*

2936 KK 283T90  
 2937 KM ROUTE FLOWS FROM CONCENTRATION POINT 283 AT QUEEN CREEK ROAD NORTH IN  
 2938 KM RITTENHOUSE CHANNEL TO THE HALF MILE STREET BETWEEN QUEEN CREEK ROAD AND  
 2939 KM GERMANN ROAD (RYAN STREET)  
 2940 KM  
 2941 KM REVISED 02.24.98 WITH RITTENHOUSE CHANNEL DESIGN (TYPICAL SECTION #5).  
 2942 KM  
 \* KO 2  
 2943 RS 3 FLOW -1  
 2944 RC .025 .025 .025 4400 .0005  
 2945 RX 0 22 28 51 61 83 90 108  
 2946 RI 11.2 9.7 5.7 0 0 5.7 9.7 11.2  
 \*

2947 KK 90A  
 2948 KM BASIN 90A  
 2949 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 2950 KM L= .6 Lca= .2 S= 24.2 Kn= .038 LAG= 12.8  
 2951 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 2952 BA .48

HEC-1 INPUT

LINE	ID	1	2	3	4	5	6	7	8	9	10
2953	LG	.10	.25	4.60	.49	62.00					
2954	UI	269.	789.	1351.	812.	319.	113.	39.	0.	0.	0.
2955	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.

2956 KK 90A  
 2957 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 2958 DT 90A 42  
 2959 DI 0 10000  
 2960 DQ 0 10000  
 \*

2961 KK C90A  
 2962 KM COMBINE FLOWS FROM C283 AND SUBBASIN 90 AT RYAN STREET ALIGNMENT  
 \* KO 2  
 2963 HC 2  
 \*

2964 KK 90ATB  
 2965 KM ROUTE FLOWS FROM SUBBASIN 90A TO 90B VIA CHANNEL  
 2966 KM REVISED 02.24.98 WITH RITTENHOUSE CHANNEL DESIGN (TYPICAL SECTION #4).  
 2967 KM  
 \* KO 2  
 2968 RS 2 FLOW 1  
 2969 RC .025 .025 .025 4400 .0005  
 2970 RX 0 22 28 51 61 83 90 108  
 2971 RI 11.2 9.7 5.7 0 0 5.7 9.7 11.2  
 \*

2972 KK 87A  
 2973 KM BASIN 87A  
 2974 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 2975 KM L= 1.0 Lca= .5 S= 24.8 Kn= .020 LAG= 11.7  
 2976 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 2977 BA .49  
 2978 LG .10 .25 5.00 .40 80.00  
 2979 UI 333. 979. 1448. 720. 221. 66. 0. 0. 0. 0.  
 2980 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 \*

2981 KK 87A

2982 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 2983 DT D87A 49  
 2984 DI 0 10000  
 2985 DQ 0 10000  
 \*

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

2986 KK 87A7B  
 2987 KM ROUTE 87A TO 87B VIA SHEET FLOW  
 2988 RS 6 FLOW -1  
 2989 RC .040 .040 .040 2640 .0056  
 2990 RX 0 500 1000 1005 1006 1011 1511 2011  
 2991 RY 1 .5 0 0 0 .5 1 1.5  
 \*

2992 KK 87B  
 2993 KM BASIN 87B  
 2994 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 2995 KM L= .9 Lca= .5 S= 11.6 Km .020 LAG= 12.8  
 2996 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 2997 BA .49  
 2998 LG .10 .25 5.00 .40 80.00  
 2999 UI 275. 809. 1385. 833. 327. 116. 40. 0. 0.  
 3000 UI 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 \*

3001 KK 887B  
 3002 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 3003 DT D87B 49  
 3004 DI 0 10000  
 3005 DQ 0 10000  
 \*

3006 KK C87  
 3007 KM COMBINE FLOW FROM SUBBASINS 87A AND 87B  
 3008 HC 2  
 \*

3009 KK 8788B  
 3010 KM ROUTE 887 TO 888 VIA GERMANN ROAD  
 3011 RS 8 FLOW -1  
 3012 RC .045 .025 .045 5280 .002  
 3013 RX 0 1000 1005 1010 1050 1060 1560 2060  
 3014 RY 14 13 18 12 11 14 14.5 15  
 \*

3015 KK 88B  
 3016 KM BASIN 88B  
 3017 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 3018 KM L= .9 Lca= .6 S= 21.2 Km .020 LAG= 12.8  
 3019 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 3020 BA .50  
 3021 LG .10 .25 5.00 .40 80.00  
 3022 UI 279. 819. 1402. 843. 331. 117. 40. 0. 0.  
 3023 UI 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 \*

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

3024 KK 888B  
 3025 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 3026 DT D88B 50  
 3027 DI 0 10000  
 3028 DQ 0 10000  
 \*

3029 KK C88B  
 3030 KM COMBINE FLOWS FROM SUBBASINS 88A AND 88B  
 3031 HC 2  
 \*

3032 KK 88T89B  
 3033 KM ROUTE 888 (CRIEMON ROAD) TO 889B (ELLSWORTH ROAD) VIA GERMANN ROAD  
 3034 RS 11 FLOW -1  
 3035 RC .045 .025 .045 5280 .004  
 3036 RX 0 1000 1005 1010 1050 1060 1560 2060  
 3037 RY 14 13 18 12 11 14 14.5 15  
 \*

3038 KK 89B  
 3039 KM BASIN 89B  
 3040 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 3041 KM L= .9 Lca= .5 S= 23.2 Km .020 LAG= 11.7  
 3042 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 3043 BA .50

EMDU69.txt

3044	LG	.10	.25	4.80	.43	80.00					
3045	UI	336.	987.	1460.	726.	223.	67.	0.	0.	0.	0.
3046	UI	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
*											
*											
3047	KK	R89B									
3048	KM	RETAIN 100 YR 2 HR RUNOFF VOLUME									
3049	DT	D89B 49									
3050	DI	0 10000									
3051	DQ	0 10000									
*											
*											
3052	KK	C89B									
3053	KM	COMBINE FLOWS FROM SUBBASINS 89A AND 89B									
3054	HC	2									
*											
*											

1 HEC-1 INPUT PAGE 85

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

3055	KK	89TB90									
3056	KM	ROUTE S89B (ELLISWORTH ROAD) TO S90B (AT RITTENHOUSE ROAD) VIA GERMANN ROAD									
3057	RS	.11 FLOW -1									
3058	RC	.045 .025 .045 8818 .0045									
3059	RK	0 1000 1005 1010 1050 1060 1560 2060									
3060	RY	14 13 18 12 11 14 14.5 15									
*											
*											

3061	KK	90B									
3062	KM	Basin 90B									
3063	KM	THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN									
3064	KM	L= 2.0 Lca= 1.2 S= 15.3 Kn= .042 LAG= 49.8									
3065	KM	PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN									
3066	BA	.82									
3067	LG	.22 .25 4.65 .38 31.00									
3068	UI	56. 56. 147. 232. 283. 327. 379. 444. 588. 709.									
3069	UI	583. 490. 427. 359. 306. 259. 187. 117. 96. 91.									
3070	UI	56. 56. 28. 17. 17. 17. 17. 17. 17. 0.									
3071	UI	0. 0. 0. 0. 0. 0. 0. 0. 0. 0.									
3072	UI	0. 0. 0. 0. 0. 0. 0. 0. 0. 0.									
*											
*											

3073	KK	R90B									
3074	KM	RETAIN 100 YR 2 HR RUNOFF VOLUME									
3075	DT	D90B 23									
3076	DI	0 10000									
3077	DQ	0 10000									
*											
*											

3078	KK	C90									
3079	KM	COMBINE FLOWS FROM 90A AND 90B									
3080	HC	3									
*											
*											

3081	KK	90T91									
3082	KM	REVISED 02.24.98 WITH RITTENHOUSE CHANNEL DESIGN (TYPICAL SECTION 13).									
3083	KM										
3084	RS	5 FLOW -1									
3085	RC	.025 .025 .025 6400 .0005									
3086	RK	0 30 36 60 88 108 114 132									
3087	RY	9.7 7.5 6 0 0 6 7.5 9.7									
*											
*											

3088	KK	85									
3089	KM	Basin 85									
3090	KM	THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN									
3091	KM	L= 2.0 Lca= .5 S= 15.0 Kn= .030 LAG= 25.8									
3092	KM	PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN									
3093	BA	1.00									

1 HEC-1 INPUT PAGE 86

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

3094	LG	.15 .25 4.35 .51 55.00									
3095	UI	131. 422. 698. 929. 1452. 1359. 991. 725. 463. 226.									
3096	UI	161. 89. 40. 40. 40. 0. 0. 0. 0. 0.									
3097	UI	0. 0. 0. 0. 0. 0. 0. 0. 0. 0.									
*											
*											
3098	KK	R85									
3099	KM	RETAIN 100 YR 2 HR RUNOFF VOLUME									
3100	DT	D85 84									
3101	DI	0 10000									
3102	DQ	0 10000									
*											
*											
3103	KK	85T86									
3104	KM	ROUTE S85 TO S86 VIA WAFB SOUTH PERIMETER CHANNEL									

EMDU89.txt

3105	RS	4	FLOW	-1						
3106	RC	0.055	0.035	0.055	5280	.0039				
3107	RK	0	500	1000	1013	1028	1041	1541	2041	
3108	RY	5.5	5	4.5	0	0	4.5	5	5.5	

3109 KK 86  
 3110 KM BASIN 86  
 3111 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 3112 KM L- 2.0 Lca- .5 S- 15.0 Kn- .030 LAG- 25.8  
 3113 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 3114 BA 1.00  
 3115 LG .15 .25 4.55 .45 55.00  
 3116 UI 131. 420. 695. 925. 1446. 1354. 987. 722. 461. 226.  
 3117 UI 160. 89. 40. 40. 40. 0. 0. 0. 0. 0.  
 3118 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

3119 KK R86  
 3120 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 3121 DT D86 85  
 3122 DI 0 10000  
 3123 DQ 0 10000

3124 KK C86  
 3125 KM COMBINE 85 AND 86 AT PECOS ROAD AND SOSSAMAN ROAD  
 3126 NC 2

1

HEC-1 INPUT

PAGE 87

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

3127 KK 86T91  
 3128 KM ROUTE 886 TO 891 VIA WAFB SOUTH PERIMETER CHANNEL. Grassy v=3ft/sec  
 3129 RS 7 FLOW -1  
 3130 RC 0.05 0.035 0.05 5500 .0025  
 3131 RK 0 500 1000 1013 1028 1041 1541 2041  
 3132 RY 5.5 5 4.5 0 0 4.5 5 5.5

3133 KK 91  
 3134 KM BASIN 91  
 3135 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 3136 KM L- 1.4 Lca- .6 S- 18.4 Kn- .030 LAG- 22.7  
 3137 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 3138 BA .46  
 3139 LG .15 .25 4.65 .42 55.00  
 3140 UI 68. 242. 399. 546. 792. 554. 390. 234. 116. 73.  
 3141 UI 28. 21. 21. 0. 0. 0. 0. 0. 0. 0.  
 3142 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

3143 KK RET91  
 3144 KM RETAIN 100YR 2HR VOLUME  
 3145 DT D91 38  
 3146 DI 0 10000  
 3147 DQ 0 10000

3148 KK 81A  
 3149 KM BASIN 81A  
 3150 KM THE FOLLOWING PARAMETERS WERE PROVIDED FOR THIS BASIN  
 3151 KM L- 3.3 Lca- 1.9 S- 16.4 Kn- .029 LAG- 49.0  
 3152 KM PHOENIX VALLEY S-GRAPH WAS USED FOR THIS BASIN  
 3153 BA 1.81  
 3154 LG .14 .25 4.70 .41 58.00  
 3155 UI 125. 125. 341. 523. 642. 737. 863. 1019. 1359. 1576.  
 3156 UI 1258. 1066. 922. 777. 656. 545. 386. 222. 209. 177.  
 3157 UI 125. 117. 38. 38. 38. 38. 38. 38. 38. 0.  
 3158 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
 3159 UI 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

3160 KK R81A  
 3161 KM RETAIN 100 YR 2 HR RUNOFF VOLUME  
 3162 DT D81A 5  
 3163 DI 0 10000  
 3164 DQ 0 10000

1

HEC-1 INPUT

PAGE 88

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

3165 KK CP91  
 3166 KM COMBINE 91, 90, 86 81A AT RITTENHOUSE CHANNEL  
 3167 HC 4

```

3168 KK 9ITEMF
3169 KM ROUTE 91 TO EMP
3170 KO 21
3171 RS 1 FLOW -1
3172 RC 0.035 0.022 0.035 4000 0.003
3173 RX 0 200 230 240 270 280 310 410
3174 RY 9 7 6 0 0 6 7 9

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3175 KK EMFRIT Revised by Dibble & Associates to remove combination at "RITTEN"
3176 KM COMBINE HYDROGRAPHS EMFRIT AND 9ITEMF
* KO 2
3177 HC 2

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* THIS PORTION OF THE MODEL IS USED TO DEVELOPE HYDROGRAPHS FOR THE CAP1A AND
* CAP1B OVERCHUTES. ALL PARAMETERS ARE FROM THE SEMESA ADMS DATED 1997

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3178 KM ***** \ / \ / modifications by Dibble & Associates \ / \ / *****
3179 KM
3180 KM On 12.16.97 - made changes to sub-basins 62C & 62D per County
3181 KM On 12.17.97 - revised detention Basin DB22A1
3182 KM On 12.18.97 - Siphon Draw basin to be on-line facility, rearranged sequence
3183 KM On 12.19.97 - Basin MK4B near Powerline Floodway, data based on grading plan
3184 KM - Revised Channel Routing parameters for Area 1
3185 KM On 01.05.98 - Revised Detention Basin DB22A1 & DB22B.
3186 KM - Coordinated file with FB.
3187 KM On 01.12.98 - Revised Hydrology per County '97 Land Use Parameters
3188 KM - File Updated by DDMS
3189 KM - Attention revised per County '97 Land Use Parameters
3190 KM On 01.14.98 - Revised routing along the Santan Alignment
3191 KM On 01.19.98 - Revised channel routing parameters, Area 2 for channel design.
3192 KM On 01.26.98 - Revised channel routing slopes in non-ADMP design areas to
3193 KM match the MAG97 values found in file FUTSOUTH.DAT from FCDMC
3194 KM On 01.26.98 - Revised wording at CAP1A and CAP1B to reflect 217 cfs per
3195 KM overchute location, not per pipe. This per Valerie Swick.
3196 KM On 01.27.98 - Copied the KK 91 to KK EMFRIT sequence from FUTSOUTH.DAT
3197 KM and revised diagram sequence to add in Santan Fwy channel.
3198 KM On 01.27.98 - Slope and NSTEPS values for some natural channels input from
3199 KM the District-supplied file "FUTSOUTH.DAT".
3200 KM On 01.28.98 - Revised hydrograph names near EMP at Rittenhouse Area.
3201 KM On 02.12.98 - Revised minor station error in channel route 65AT66, RX record
3202 KM On 02.23.98 - Revised per
3203 KM FCDMC comments: Reworded KM record for route 65AT66.
3204 KM Length for route 65T66 revised to 2400 feet.
3205 KM KM added to 78B to explain why no retention.
3206 KM Added channel route for CP82A4 to CP82A5.

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
3207 KM Added 0.25 ratio @ Basin 75, removed retention
3208 KM On 02.24.98 - Revised Rittenhouse Channel routing to reflect plans
3209 KM per FCD Contract No. 97-34 (Phase 2)
3210 KM
3211 KM On 03.03.98 - Received This File as FINAL HYDROLOGY from the FCDMC
3212 KM On 03.04.98 - Revised flow routing to show the PLF connecting to
3213 KM the Santan Channel. Also re-ordered subbasin 80a
3214 KM to enter the system at the proper location.
3215 KM On 03.06.98 - Revised KM record for EMPFNX to delete reference to C76A.
3216 KM On 03.10.98 - Revised preliminary channel sizes from design data.
3217 KM Added routing reaches represented in design. (Area 1).
3218 KM On 03.12.98 - Regraded Crismon Basin to avoid ADWR jurisdictional dam.
3219 KM On 03.13.98 - Revised Siphon Draw Basin to avoid ADWR jurisdictional dam.
3220 KM On 03.24.98 - Revised normal depth channel routing per Area 2 channel design.
3221 KM On 04.22.98 - Submitted Hydrology to County.
3222 KM On 05.20.98 - Revised Hydrology per County Comments
3223 KM On 06.11.98 - Revised channel properties to reflect earth channels
3224 KM On 06.12.98 - Submitted file to County (This is a pre-final submittal)
3225 KM
3226 KM On 06.18.98 - Added combine at Knox Road for better flowrate resolution.
3227 KM
3228 KM On 07.17.98 - Revised per FCD review comments. Channel routings revised to
3229 KM follow the ADMP Preliminary Design Plans. More verbal
3230 KM descriptions for channel routings added.
3231 KM
3232 KM On 07.24.98 - This is the final submittal HEC-1 input file.
3233 KM
3234 KM On 08.06.98 -Back checked entire file against FCDMC review comments printout.
3235 KM
3236 KM On 10.21.98 -Revised routing at Elliot basin for prelim. 30% design.
3237 KM
3238 KM On 10.27.98 -Revised divert for retention at Elliot Basin.
3239 KM
3240 KM On 01.15.99 -File submitted as part of Elliot Basin Addendum to the ADMP.
3241 KM
3242 KM ***** ***** modifications by Dibble & Associates *****
*
3243 ZZ

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SCHEMATIC DIAGRAM OF STREAM NETWORK

```

INPUT LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW
NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW

```

```
383 SOSS
    V
388 RSOSS
    .
394 .      59A
    .
406 .      .-----> D59A
404 .      R59A
    .
409 .      C59A-----
    V
412 59A59B
    .
419 .      59B
    .
433 .      .-----> D59B
431 .      R59B
    .
436 .      C59B-----
    V
439 59BT60
    .
445 .      60
    .
462 .      .-----> D60
460 .      R60
    .
465 .      EMFGUA-----
    V
469 GUATEL
    .
475 .      64
    .
492 .      .-----> D64
486 .      R64
    .
495 .      EMPELL-----
    V
498 ELTWAR
    .
504 .      62A
    .
515 .      .-----> D62A
513 .      R62A
    V
518 62ATB
    .
524 .      62B
    .
535 .      .-----> D62B
533 .      R62B
    .
538 .      C62B-----
    V
541 62BTD
    .
547 .      62D
    .
560 .      .-----> D62D
558 .      R62D
    .
563 .      CP62D-----
    V
566 62DIF
    .
572 .      62F
    .
584 .      .-----> D62F
582 .      R62F
```

```
587 . CP62F .....  
    . V  
    . V  
590 . 62T63  
    .  
597 . 63  
    .  
609 . -----> D63  
607 . R63  
    .  
612 . CP63 .....  
    . V  
    . V  
615 . 63T71  
    .  
622 . 68B1  
    .  
631 . 68B2  
    .  
640 . 68B3  
    .  
649 . CP68 .....  
    .  
654 . -----> D68B  
652 . R68  
    . V  
    . V  
657 . 68BT69  
    .  
663 . 69  
    .  
674 . -----> D69  
672 . R69  
    .  
677 . C69 .....  
    . V  
    . V  
680 . 69T71  
    .  
686 . 25  
    .  
700 . -----> 25RET  
698 . RET25  
    . V  
    . V  
703 . 25T71  
    .  
709 . 71  
    .  
724 . -----> D71  
721 . R71  
    .  
729 . C71 .....  
    . V  
    . V  
733 . 71T72  
    .  
740 . 72  
    .  
752 . -----> D72  
750 . R72  
    .  
755 . CPKNOX .....  
    .  
758 . EMFWAR .....  
    . V  
    . V  
761 . WARTKN  
    .  
767 . 26  
    .  
778 . -----> 26RET  
776 . RET26  
    . V  
    . V
```







```

1172 . . . . .
1170 . . . . . R66A -----> D66A
      . . . . . V
      . . . . . V
1175 . . . . . 66ATB
      . . . . .
1181 . . . . . 66B
      . . . . .
1194 . . . . . R66B -----> D66B
1192 . . . . .
      . . . . .
1197 . . . . . CP66B ----->
      . . . . . V
1200 . . . . . 66BTC
      . . . . .
1206 . . . . . 66C
      . . . . .
1218 . . . . . R66C -----> D66C
1216 . . . . .
      . . . . .
1221 . . . . . CP66C1 ----->
      . . . . .
1225 . . . . . CP66C2 ----->
      . . . . .
1232 . . . . . DI66 -----> D66
1229 . . . . . V
      . . . . . V
1235 . . . . . 66C1T2
      . . . . .
1243 . . . . . CP66C ----->
      . . . . . V
1246 . . . . . 66CTD
      . . . . .
1253 . . . . . DR66 -----> D66
1251 . . . . . V
      . . . . . V
1254 . . . . . RS66D1
      . . . . .
1267 . . . . . B-WA -----> D-WB
1264 . . . . .
      . . . . .
1270 . . . . . C-WA ----->
      . . . . . V
      . . . . . V
1273 . . . . . RC-WA
      . . . . .
1279 . . . . . DR-WA -----> D-WB
1277 . . . . . V
      . . . . . V
1280 . . . . . RS66D2
      . . . . .
1290 . . . . . CP66D ----->
      . . . . . V
      . . . . . V
1294 . . . . . 66T66D
      . . . . . V
      . . . . . V
1298 . . . . . 66-66D
      . . . . .
1302 . . . . . 66D
      . . . . .
1314 . . . . . R66D -----> D66D
1311 . . . . .
      . . . . .
1317 . . . . . 61A
      . . . . .
1329 . . . . . R61A -----> D61A
1327 . . . . . V
      . . . . . V
1332 . . . . . 61ATE
      . . . . .
1338 . . . . . 61B
      . . . . .

```



```

1539 C70A1 .....
      V
1542 70A1T2
      V
1549      24
      V
1560      -----> 24REI
1558 RET24
      V
1563      70A2
      V
1572 CP70A2 .....
      V
1576 70T76A
      V
1584      76A
      V
1597      -----> D76A
1595 R76A
      V
1600 C76A .....
      V
1603 76ATER
      V
1611      73A
      V
1625 73ATB
      V
1632      73B
      V
1643      -----> 73BRET
1641 RET73B
      V
1646 CP73B .....
      V
1649 73BTC
      V
1656      73C
      V
1668      -----> 73CRET
1666 RET73C
      V
1671 CP73C .....
      V
1674 73T74C
      V
1681      74A
      V
1695 74ATE
      V
1703      74B
      V
1715      -----> 74BRET
1713 RET74B
      V
1718 CP74B .....
      V
1721 74BTC
      V
1728      74C
      V
1740      -----> 74CRET
1738 RET74C
      V
1743 CP74C .....
      V
1746 74CT10

```



```

EMD089.txt
.
.
.
1969 CP5-----
.   V
.   V
1972 5T12
.
.
.
1978      12
.
.
.
1992      -----> 12RET
1990 RET12
.
.
.
1995 CP12-----
.   V
.   V
1998 12T13
.
.
.
2004      08
.
.
.
2018      -----> 08RET
2016 RET08
.   V
.   V
2021 8T9
.
.
.
2027      09
.
.
.
2041      -----> 09RET
2039 RET09
.
.
.
2044 CP9-----
.   V
.   V
2047 9T13
.
.
.
2053      13
.
.
.
2067      -----> 13RET
2065 RET13
.
.
.
2070 CP13-----
.   V
.   V
2073 13T75
.
.
.
2079      14
.
.
.
2093      -----> 14RET
2091 RET14
.
.
.
2096 CP75-----
.   V
.   V
2099 75T7C
.
.
.
2105 77A
.   V
.   V
2121 77ATB
.
.
.
2128      77B
.
.
.
2139      -----> 77BRET
2137 RET77B
.
.
.
2142 CP77B-----
.   V
.   V
2145 77BTC
.
.
.
2152      77C
.
.
.
2163      -----> 77CRET
2161 RET77C
.
.
.
2166 C77C-----
.   V
.   V

```









```

2858      V
          RG267
2861      .
          SUB268
          .
2876      .-----> RETDIV
2874      .
          R268
          .
2879      CQ270.....
          V
          V
2882      RQ283
          .
          .
          B8A
          .
2896      .-----> D88A
2894      .
          RB8A
          V
          V
2899      BBAT89
          .
          .
          B9A
          .
2907      .
          .
          R89A-----> D89A
          .
          .
          C89A.....
          V
          V
2924      B9ATRI
          .
          .
          C283.....
          V
          V
2936      283T90
          .
          .
          90A
          .
2958      .-----> D90A
2956      .
          R90A
          .
          .
          C90A.....
          V
          V
2964      90ATB
          .
          .
          87A
          .
2983      .-----> D87A
2981      .
          R87A
          V
          V
2986      87ATB
          .
          .
          87B
          .
2992      .
          .
          R87B-----> D87B
          .
          .
          C87.....
          V
          V
3009      87T88B
          .
          .
          88B
          .
3026      .-----> D88B
3024      .
          R88B
          .
          .
          C88B.....
          V
          V
3032      88T89B
          .
          .
          89B
          .
3038      .
          .
          .-----> D89B
3048      .
    
```



EMDU89.txt

WOOD, PATEL & ASSOCIATES, INC.  
DARREN E. SMITH, P.E.

FILE PATH:  
R:\MESA PROVING GROUNDS\2012\123835\PROJECT SUPPORT\REPORTS\  
DRAINAGE\HYDROLOGY\PROPOSED\EMDU89.DAT

\*\*\*\*\*  
FILE: MFGDU7.DAT

MODEL REVISED: 09-07-2011

PROJECT: MESA PROVING GROUNDS

THIS MODEL SHOULD REPLACE WS4-SEM.DAT IN THE HEC-1 RUN SEQUENCE SPECIFIED BELOW. REFERENCING WS2-NEM.DSS IS STILL REQUIRED.

THIS IS A POST DEVELOPED MODEL REVISION TO REFLECT UPDATED PLANNING FOR DEVELOPMENT UNIT 7 (DU7) PROVIDED BY ARIZONA LAND DESIGN ON 09/02/2011.

MODEL REVISION DESCRIPTION:

THIS MODEL IS AN EXERPT OF THE MODEL PROVIDED BY THE FLOOD CONTROL DISTRICT OF MARICOPA COUNTY (WS4-SEM.DAT). ONSITE WATERSHEDS WERE UPDATED TO REFLECT A GRADING PLAN PROVIDED BY LD TEAM ON 8/30/2011. MODELLING OF THE POWERLINE FLOODWAY HAS BEEN UPDATED TO REFLECT THE EXISTING SECTIONS AND SLOPE PER AS-BUILT DRAWINGS ACROSS THE MFG SITE.

MODEL REVISED BY:  
WOOD, PATEL & ASSOCIATES, INC.  
DANIEL W. MATTHEWS, E.I.T.

FILE PATH:  
R:\MESA PROVING GROUNDS\2011\113697\PROJECT SUPPORT\REPORTS\  
DRAINAGE\HYDROLOGY\MFGDU7.DAT

\*\*\*\*\*  
FILE: MFG20RT2.DAT

MODEL REVISED: 04-25-2011

PROJECT: MESA PROVING GROUNDS

THIS MODEL SHOULD REPLACE WS4-SEM.DAT IN THE HEC-1 RUN SEQUENCE SPECIFIED BELOW. REFERENCING WS2-NEM.DSS IS STILL REQUIRED.

THIS IS A 100-YEAR, 2-HOUR RETENTION SCENARIO MODEL USING THE 20MSF COMMERCIAL SPACE AND 15K DU LAND PLAN PROVIDED BY SHABACK PARTNERS ON 12/12/07.

MODEL REVISION DESCRIPTION:

THIS MODEL IS AN EXERPT OF THE MODEL PROVIDED BY THE FLOOD CONTROL DISTRICT OF MARICOPA COUNTY (WS4-SEM.DAT). ONSITE WATERSHEDS 01 AND 20 WERE UPDATED TO REFLECT THE INCORPORATION OF THE FIRST SOLAR SITE IN THE NORTHEAST CORNER OF DU-6. WATERSHED 02 WAS SPLIT INTO 02A AND 02B. LAND USE WAS CHANGED TO INDUSTRIAL FOR 02B AND ENTIRELY RESIDENTIAL FOR 02A. THE FIRST SOLAR SITE RUNOFF WILL NOW BE RETAINED ENTIRELY ONSITE.

MODEL REVISED BY:  
WOOD, PATEL & ASSOCIATES, INC.  
STEPHEN M. SCINTO, P.E.

FILE PATH:  
R:\MESA PROVING GROUNDS\2010\103564.04\PROJECT SUPPORT\REPORTS\  
DRAINAGE\HYDROLOGY\POST-DEVELOPED 100YR2HR RETENTION MODEL\  
MFG20RT2.DAT

\*\*\*\*\*  
FILE: MFG20RT2.DAT

MODEL REVISED: 09-16-08

PROJECT: MESA PROVING GROUNDS

THIS MODEL SHOULD REPLACE WS4-SEM.DAT IN THE HEC-1 RUN SEQUENCE SPECIFIED BELOW. REFERENCING WS2-NEM.DSS IS STILL REQUIRED.

THIS IS A 100-YEAR, 2-HOUR RETENTION SCENARIO MODEL USING THE 20MSF COMMERCIAL SPACE AND 15K DU LAND PLAN PROVIDED BY SHABACK PARTNERS ON 12/12/07.

MODEL REVISION DESCRIPTION:

THIS MODEL IS AN EXERPT OF THE MODEL PROVIDED BY THE FLOOD CONTROL DISTRICT OF MARICOPA COUNTY (WS4-SEM.DAT). ONSITE WATERSHEDS 01, 02, 03, AND 05 WERE UPDATED TO REFLECT THE CURRENT GOLF COURSE CONFIGURATION.

MODEL REVISED BY:  
WOOD, PATEL & ASSOCIATES, INC.  
DANIEL W. MATTHEWS, E.I.T.

EMD089.txt

FILE PATH:  
R:\MESA PROVING GROUNDS\2006\062753\PROJECT SUPPORT\HYDRO\MDR-20-15 LAND  
PLAN\2ND SUBMITTAL\COM\HYDROLOGY\MFG20RT2.DAT

\*\*\*\*\*  
FILE: MFG20RT2.DAT

MODEL REVISED: 05-15-08

PROJECT: MESA PROVING GROUNDS

MODEL REVISION DESCRIPTION:

THIS MODEL SHOULD REPLACE WS4-SEM.DAT IN THE HEC-1 RUN SEQUENCE SPECIFIC  
BELOW. REFERENCING WS2-NEM.DSS IS STILL REQUIRED.

THIS IS A 100-YEAR, 2-HOUR RETENTION SCENARIO MODEL USING  
THE 20MSF COMMERCIAL SPACE AND 15K DU LAND PLAN PROVIDED  
BY SWABACK PARTNERS ON 12/12/07.

THIS MODEL IS AN EXERPT OF THE MODEL PROVIDED BY THE FLOOD CONTROL  
DISTRICT OF MARICOPA COUNTY (WS4-SEM.DAT). WATERSHED 79A WAS UPDATED  
AS REQUESTED BY FLOOD CONTROL DISTRICT OF MARICOPA COUNTY TO REDUCE THE  
PERCENT IMPERVIOUS VALUE FROM 80% TO 0% TO MATCH THE LAND USE AS MODELED  
WITHIN THE EAST MESA ADMF.

MODEL REVISED BY:  
WOOD, PATEL & ASSOCIATES, INC.  
DANIEL W. MATTHEWS, E.I.T.

FILE PATH:  
R:\MESA PROVING GROUNDS\2006\062753\PROJECT SUPPORT\HYDRO\MDR-20-15 LAND  
PLAN\2ND SUBMITTAL\POST-DEVELOPED 100YR2HR RETENTION MODEL (MFG20RT2)\  
MFG20RT2.DAT

\*\*\*\*\*  
FILE: MFG20RT2.DAT

MODEL REVISED: 01-08-08

PROJECT: MESA PROVING GROUNDS

MODEL REVISION DESCRIPTION:

THIS MODEL SHOULD REPLACE WS4-SEM.DAT IN THE HEC-1 RUN SEQUENCE SPECIFIC  
BELOW. REFERENCING WS2-NEM.DSS IS STILL REQUIRED.

THIS IS A 100-YEAR, 2-HOUR RETENTION SCENARIO MODEL USING  
THE 20MSF COMMERCIAL SPACE AND 15K DU LAND PLAN PROVIDED  
BY SWABACK PARTNERS ON 12/12/07.

THIS MODEL IS AN EXERPT OF THE MODEL PROVIDED BY THE FLOOD CONTROL  
DISTRICT OF MARICOPA COUNTY (WS4-SEM.DAT). WATERSHEDS 68A, 68B,  
70A, 70B, 71, 73B, 73C, 74B, 74C, 75, 77B, 77C, 78B, 78C, AND 79A  
HAVE ALL BEEN UPDATED TO REFLECT CURRENT WATERSHED DELINEATIONS,  
NEW DEVELOPMENT, CURRENT RETENTION, AND FLOOD ROUTING. BASIN 75  
HAS BEEN UPDATED TO REFLECT PLANNED DEVELOPEMENT FOR THE MESA  
PROVING GROUNDS SITE.

MODEL REVISED BY:  
WOOD, PATEL & ASSOCIATES, INC.  
DANIEL W. MATTHEWS, E.I.T.

FILE PATH:  
R:\MESA PROVING GROUNDS\2006\062753\PROJECT SUPPORT\HYDRO\MDR-20-15 LAND  
PLAN\HYDROLOGY\POST-DEVELOPED 100YR2HR RETENTION MODEL (MFG20RT2)\  
MFG20RT2.DAT

\*\*\*\*\*  
ID Kirkham Michael:  
Last Revised Date: 1/22/03  
Filename: WS4-SEM.DAT

Comments Dated 1/22/03 (CJ)

This model should be used ONLY for the Rittenhouse and Chendler Heights  
Basin Design Project - Final Design Analyses.

This model is one of several models that represent the EMF watershed.  
This model covers the Southeast Mesa Area and should reference as a DSS  
the watershed model for the Northeast Mesa Area (Filename WS2-NEM.DAT).

This model is necessary to determine the input hydrographs for the  
Rittenhouse Basin Design HEC-RAS Unsteady State analysis. To develop  
the necessary input hydrographs the following models should be run in order.  
Because the files utilize a TAPE21 file to export import hydrographs  
between models, prior to running the FIRST model (WS1-NEM.DAT) any existing  
TAPE21 file in the directory should be deleted. The run procedure order is:

1) WS1-NEM.DAT

EMDU89.txt

- 2) WS2-NEM.DAT
- 3) WS3-QCSW.DAT
- 4) WS4-SEM.DAT (referencing WS2-NEM.DSS for the DSS file)
- 5) RT1-BASE.DAT

The necessary input hydrographs for the Rittenhouse Basin analysis are determined in RT1-BASE. In that output file, the hydrograph at RWFLD1 should be exported and used as the input hydrograph at the EMF Reach 4 Cross Section 17.082. And the hydrograph at RITTEN should be exported and used as the input hydrograph for the Rittenhouse Main Channel at Cross Section B20.00

\*\*\*\*\*  
 \*\*\*\* NOTE BY PRIMATECH ENGINEERS: \*\*\*\*  
 \*\*\*\* DATE: 06/12/2001 \*\*\*\*  
 \*\*\*\* THE NEW FILE NAME IS: SEBTALT2.DAT \*\*\*\*  
 \*\*\*\* THE FILE WAS RENAMED AS <<RTBTALT2.DAT>> FOR THE EAST MARICOPA \*\*\*\*  
 \*\*\*\* FLOODWAY CAPACITY MITIGATION PROJECT, BY FLOOD CONTROL DISTRICT OF \*\*\*\*  
 \*\*\*\* MARICOPA COUNTY. \*\*\*\*  
 \*\*\*\* THE FILE WAS RENAMED <<RTBTALT3.DAT>> AND UPDATED USING GREEN AND \*\*\*\*  
 \*\*\*\* AMPT FUTURE CONDITIONS FOR BASINS 258 TO 268. \*\*\*\*  
 \*\*\*\*\*

THIS MODEL WAS ORIGINALLY MIDDOUT.DAT  
 IT HAS BEEN MODIFIED BY CPE (7/2000)  
 FOR ALTERNATIVE 2 FOR THE EAST MARICOPA FLOWWAY  
 CAPACITY MITIGATION AND MULTI-USE CORRIDOR STUDY  
 TO ROUTE BOTH THE POWERLINE FLOWWAY  
 AND THE SANTAN FREEWAY CHANNEL INTO THE RAY BASIN PRIOR THEIR OUTFALL  
 INTO THE EMF

Model files changed by Collins/Pins Engineering  
 to reflect multi-use design concepts (recreation  
 and environment) proposed throughout the entire  
 EMF Corridor. July 2000

VERSION 8.06 CPE 7/31/00

\*\*\*\*\*  
 FILENAME: MIDDOUT.DAT

ALL CIP INFRASTRUCTURE IS IN PLACE, FUTURE CONDITIONS LANDUSE IS IN PLACE  
 FLOW IS ROUTED UP ELLSWORTH ROAD IN A EARTH LINED CHANNEL

\*\*\*\*\*  
 PRODUCED BY DIBBLE AND ASSOCIATES AND HOSKIN ENGINEERING CONSULTANTS.  
 File Name: Final8.Dat  
 Revised - Jan. 2000 by SZ (Wood/Patel) From Final7.dat - new 2-V & Sideweir  
 Revised - Jan. 2000 by SZ (Wood/Patel) from Final6.dat - 604 review comments  
 Revised - Dec. 1999 by SZ (Wood/Patel) from Final5.dat  
 Revised - Dec. 1999 by SZ (Wood/Patel) from Final4.dat  
 Revised - Nov. 1999 by SZ (Wood/Patel) from Final3.dat  
 Revised - June 1999 by SZ (Wood/Patel) for Final Model from Opt1.dat.  
 Revised - May 1999 by SZ (Wood/Patel) for Option 1, Based on Model SDB.DAT  
 REVISED - MAY, 1999 BY VAS TO INCORPORATE INCREASE OF SUBBASIN RETENTION AND  
 REVISIONS TO THE REGIONAL DETENTION BASIN STORAGE  
 REVISED - FEB, 1999 BY VALERIE SWICK, PCD OF MARICOPA COUNTY  
 REVISED - MAY, 1998 BY D&A

REVISED BY VALERIE SWICK, FEB. 26, 1998

FLOWS FROM DETENTION BASIN LOCATED AT NE CORNER OF ELLIOT AND ELLSWORTH ROADS  
 IS ROUTED TO THE SOUTHWEST BY SIPHON DRAW TO SUBBASIN 70A. FROM THERE THEY  
 WILL BE ROUTED BY A CHANNEL TO THE EMF. FLOWS FROM SUBBASINS ADJACENT TO  
 SANTAN FREEWAY ALIGNMENT WILL BE ROUTED SOUTH TO SUBBASIN 70A WHERE THEY WILL  
 BE COMBINED WITH FLOW IN SIPHON DRAW.

EAST MESA AREA DRAINAGE MASTER PLAN  
 AREA SOUTH OF SUPERSTITION (U.S. HWY 60)  
 AUGUST 1997  
 SOUTHEAST MESA HIGH RESOLUTION MODEL

\*\*\*\*\*FUTURE CONDITION MODEL OF THE WATERSHED\*\*\*\*\*

\*\*\*\*\*ATTENTION\*\*\*\*\*  
 SUBBASINS 75, 79A, 79B, 78E, LANDUSES WERE NOT  
 CHANGED BECAUSE IT WAS FELT THAT THEIR FUTURE CONDITIONS LANDUSES WOULD BE  
 SIMILAR TO THE EXISTING CONDITIONS LANDUSES.  
 RETENTION VOLUMES WILL ALSO NOT BE UTILIZED FOR SUBBASINS 75, 79A, 79B, 78E  
 SOME QUEEN CREEK SUBBASINS WILL ALSO NOT HAVE RETENTION VOLUMES, EITHER  
 BECAUSE THEY LIE IN PINAL COUNTY AND WE DONT KNOW PINAL COUNTIES PLANS OR  
 THEY LIE IN THE SANTAN MOUNTAINS AND WON'T GET DEVELOPED  
 WILLIAMS GATEWAY AIRPORT (SUBBASINS 80A, 80B, 81A, AND 81B) ARE MODELED AS  
 FUTURE CONDITIONS AND HAVE RETENTION VOLUMES FOR THE 100YR 2HR STORM  
 \*\*\*\*\*

FILENAME: SDBB.DAT

THIS MODEL REPRESENTS THE FUTURE CONDITION OF THE WATERSHED.  
 TOTAL DRAINAGE AREA IS APPROXIMATELY 213 SQ. MI.  
 THIS MODEL USES A K<sub>n</sub> VALUE OF 0.09 FOR DESERT LAND USE DUE TO SHEET FLOW











```

Pathname: /SOSSAMAN DRAIN/AT SUPERSTITIION/FLOW/31MAR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 50: /SOSSAMAN DRAIN/AT SUPERSTITIION/FLOW/31MAR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 1 NVALS: 1 JULS: 35519 IETIME: 1440
NLDATA: 288 JULSD: 35519
JULS: 31MAR97 JULSD: 31MAR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 288 NDATA: 288 NREAD: 1 ILIM: 1
---- Exiting ZRRTS, Number of data values: 1, Status: 0
Offset: 0, Units: CFS, Type:INST-VAL
---- Entering ZRRTSX for unit 71 ----
Pathname: /SOSSAMAN DRAIN/AT SUPERSTITIION/FLOW/5MIN/100YR/
Time Window set. Interval: 5 Number of data values: 601
Starting date and time: Mar 31, 1997 2400 ( 35519 1440)
Ending date and time: Apr 3, 1997 0200 ( 35522 120)
Input time offset: 0
After ZRDINF, Record found: T
Pathname: /SOSSAMAN DRAIN/AT SUPERSTITIION/FLOW/31MAR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 50: /SOSSAMAN DRAIN/AT SUPERSTITIION/FLOW/31MAR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 1 NVALS: 601 JULS: 35519 IETIME: 1440
NLDATA: 288 JULSD: 35519
JULS: 31MAR97 JULSD: 31MAR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 288 NDATA: 288 NREAD: 1 ILIM: 1
After ZRDINF, Record found: T
Pathname: /SOSSAMAN DRAIN/AT SUPERSTITIION/FLOW/01APR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 50: /SOSSAMAN DRAIN/AT SUPERSTITIION/FLOW/01APR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 2 NVALS: 601 JULS: 35519 IETIME: 1440
NLDATA: 288 JULSD: 35520
JULS: 31MAR97 JULSD: 01APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 288 ILIM: 289
After ZRDINF, Record found: T
Pathname: /SOSSAMAN DRAIN/AT SUPERSTITIION/FLOW/02APR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 50: /SOSSAMAN DRAIN/AT SUPERSTITIION/FLOW/02APR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 290 NVALS: 601 JULS: 35519 IETIME: 1440
NLDATA: 288 JULSD: 35521
JULS: 31MAR97 JULSD: 02APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 288 ILIM: 577
After ZRDINF, Record found: T
Pathname: /SOSSAMAN DRAIN/AT SUPERSTITIION/FLOW/03APR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 50: /SOSSAMAN DRAIN/AT SUPERSTITIION/FLOW/03APR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 578 NVALS: 601 JULS: 35519 IETIME: 1440
NLDATA: 288 JULSD: 35522
JULS: 31MAR97 JULSD: 03APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 24 ILIM: 601
---- Exiting ZRRTS, Number of data values: 601, Status: 0
Offset: 0, Units: CFS, Type:INST-VAL

```

\*\*\* \*\*

```

*****
* *
465 KK * EMFGUA *
* *
*****

```

```

467 KO OUTPUT CONTROL VARIABLES
IFRNT 5 PRINT CONTROL
IFLOT 0 PLOT CONTROL
QSCAL 0 HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 600 LAST ORDINATE PUNCHED OR SAVED
TIMINT .083 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
* *
486 KK * R64 *
* *
*****

```

```

491 KO OUTPUT CONTROL VARIABLES

```

```

IPRNT      5 PRINT CONTROL
IFLOT      0 PLOT CONTROL
QSCAL      0 HYDROGRAPH PLOT SCALE
IPNCH      0 PUNCH COMPUTED HYDROGRAPH
IOUT       21 SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2      600 LAST ORDINATE PUNCHED OR SAVED
TIMINT     .083 TIME INTERVAL IN HOURS
    
```

\*\*\*\*\*

```

*****
* *
755 KK * CPKNOX *
* *
*****
    
```

```

756 KO OUTPUT CONTROL VARIABLES
IPRNT      5 PRINT CONTROL
IFLOT      0 PLOT CONTROL
QSCAL      0 HYDROGRAPH PLOT SCALE
IPNCH      0 PUNCH COMPUTED HYDROGRAPH
IOUT       21 SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2      600 LAST ORDINATE PUNCHED OR SAVED
TIMINT     .083 TIME INTERVAL IN HOURS
    
```

\*\*\*\*\*

```

*****
* *
830 KK * KNOX *
* *
*****
    
```

```

832 KO OUTPUT CONTROL VARIABLES
IPRNT      5 PRINT CONTROL
IFLOT      0 PLOT CONTROL
QSCAL      0 HYDROGRAPH PLOT SCALE
IPNCH      0 PUNCH COMPUTED HYDROGRAPH
IOUT       21 SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2      600 LAST ORDINATE PUNCHED OR SAVED
TIMINT     .083 TIME INTERVAL IN HOURS
    
```

```

----- Entering ZRRISX for unit 71 -----
Pathname: /CAP1A/OVERCHUTE/FLOW//SMIN/100YEAR/
Time Window set. Interval: 5 Number of data values: 1
Starting date and time: Mar 31, 1997 2400 ( 35519 1440)
Ending date and time: Mar 31, 1997 2400 ( 35519 1440)
Input time offset: 0
After ZRDINF, Record found: T
Pathname: /CAP1A/OVERCHUTE/FLOW/31MAR1997/SMIN/100YEAR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 50: /CAP1A/OVERCHUTE/FLOW/31MAR1997/SMIN/100YEAR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 1 NVALS: 1 JULS: 35519 ISTE: 1440
NLDATA: 288 JULSD: 35519
JULS: 31MAR97 JULSD: 31MAR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 288 NDATA: 288 NREAD: 1 ILIM: 1
----- Exiting ZRRISX, Number of data values: 1, Status: 0
Offset: 0, Units: CFS, Type:INST-VAL
----- Entering ZRRISX for unit 71 -----
Pathname: /CAP1A/OVERCHUTE/FLOW//SMIN/100YEAR/
Time Window set. Interval: 5 Number of data values: 601
Starting date and time: Mar 31, 1997 2400 ( 35519 1440)
Ending date and time: Apr 3, 1997 0200 ( 35522 120)
Input time offset: 0
After ZRDINF, Record found: T
Pathname: /CAP1A/OVERCHUTE/FLOW/31MAR1997/SMIN/100YEAR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 50: /CAP1A/OVERCHUTE/FLOW/31MAR1997/SMIN/100YEAR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 1 NVALS: 601 JULS: 35519 ISTE: 1440
NLDATA: 288 JULSD: 35519
JULS: 31MAR97 JULSD: 31MAR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 288 NDATA: 288 NREAD: 1 ILIM: 1
After ZRDINF, Record found: T
Pathname: /CAP1A/OVERCHUTE/FLOW/01APR1997/SMIN/100YEAR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 50: /CAP1A/OVERCHUTE/FLOW/01APR1997/SMIN/100YEAR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 2 NVALS: 601 JULS: 35519 ISTE: 1440
NLDATA: 288 JULSD: 35520
JULS: 31MAR97 JULSD: 01APR97
Quality Read: F, Quality Requested: F
    
```

```

EMD089.txt
---ZRTS Calculations: NPOS: 1 NDATA: 288 NREAD: 288 ILIM: 289
After ZRDINF, Record found: 1
Pathname: /CAP1A/OVERCHUTE/FLOW/02APR1997/5MIN/100YEAR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers: 50; /CAP1A/OVERCHUTE/FLOW/02APR1997/5MIN/100YEAR/
-----DSS---Debug: Enter ZRTS; Unit: 71
NSTART: 290 NVALS: 601 JULS: 35519 ISTEIME: 1440
NLDATA: 288 JULSD: 35521
JULS: 31MAR97 JULSD: 02APR97
Quality Read: F, Quality Requested: F
---ZRTS Calculations: NPOS: 1 NDATA: 288 NREAD: 288 ILIM: 577
After ZRDINF, Record found: 1
Pathname: /CAP1A/OVERCHUTE/FLOW/03APR1997/5MIN/100YEAR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers: 50; /CAP1A/OVERCHUTE/FLOW/03APR1997/5MIN/100YEAR/
-----DSS---Debug: Enter ZRTS; Unit: 71
NSTART: 578 NVALS: 601 JULS: 35519 ISTEIME: 1440
NLDATA: 288 JULSD: 35522
JULS: 31MAR97 JULSD: 03APR97
Quality Read: F, Quality Requested: F
---ZRTS Calculations: NPOS: 1 NDATA: 288 NREAD: 24 ILIM: 601
---- Exiting ZRTS, Number of data values: 601, Status: 0
Offset: 0, Units: CFS, Type:INST-VAL
-----DSS---ZWRITE Unit 71; Vers: 155; /CAP1B/OVERCHUTE/FLOW/31MAR1997/5MIN/100YEAR/
-----DSS---ZWRITE Unit 71; Vers: 155; /CAP1B/OVERCHUTE/FLOW/01APR1997/5MIN/100YEAR/
-----DSS---ZWRITE Unit 71; Vers: 155; /CAP1B/OVERCHUTE/FLOW/02APR1997/5MIN/100YEAR/
-----DSS---ZWRITE Unit 71; Vers: 155; /CAP1B/OVERCHUTE/FLOW/03APR1997/5MIN/100YEAR/
---- Entering ZRTSX for unit 71 ----
Pathname: /ADOT EAST BASIN/AT SUPERSTITIION/FLOW//5MIN/100YR/
Time Window set. Interval: 5 Number of data values: 1
Starting date and time: Mar 31, 1997 2400 ( 35519 1440)
Ending date and time: Mar 31, 1997 2400 ( 35519 1440)
Input time offset: 0
After ZRDINF, Record found: 7
Pathname: /ADOT EAST BASIN/AT SUPERSTITIION/FLOW/31MAR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers: 25; /ADOT EAST BASIN/AT SUPERSTITIION/FLOW/31MAR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRTS; Unit: 71
NSTART: 1 NVALS: 1 JULS: 35519 ISTEIME: 1440
NLDATA: 288 JULSD: 35519
JULS: 31MAR97 JULSD: 31MAR97
Quality Read: F, Quality Requested: F
---ZRTS Calculations: NPOS: 288 NDATA: 288 NREAD: 1 ILIM: 1
---- Exiting ZRTS, Number of data values: 1, Status: 0
Offset: 0, Units: CFS, Type:INST-VAL
---- Entering ZRTSX for unit 71 ----
Pathname: /ADOT EAST BASIN/AT SUPERSTITIION/FLOW//5MIN/100YR/
Time Window set. Interval: 5 Number of data values: 601
Starting date and time: Mar 31, 1997 2400 ( 35519 1440)
Ending date and time: Apr 3, 1997 0200 ( 35522 120)
Input time offset: 0
After ZRDINF, Record found: 1
Pathname: /ADOT EAST BASIN/AT SUPERSTITIION/FLOW/31MAR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers: 25; /ADOT EAST BASIN/AT SUPERSTITIION/FLOW/31MAR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRTS; Unit: 71
NSTART: 1 NVALS: 601 JULS: 35519 ISTEIME: 1440
NLDATA: 288 JULSD: 35519
JULS: 31MAR97 JULSD: 31MAR97
Quality Read: F, Quality Requested: F
---ZRTS Calculations: NPOS: 288 NDATA: 288 NREAD: 1 ILIM: 1
After ZRDINF, Record found: 1
Pathname: /ADOT EAST BASIN/AT SUPERSTITIION/FLOW/01APR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers: 25; /ADOT EAST BASIN/AT SUPERSTITIION/FLOW/01APR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRTS; Unit: 71
NSTART: 2 NVALS: 601 JULS: 35519 ISTEIME: 1440
NLDATA: 288 JULSD: 35520
JULS: 31MAR97 JULSD: 01APR97
Quality Read: F, Quality Requested: F
---ZRTS Calculations: NPOS: 1 NDATA: 288 NREAD: 288 ILIM: 289
After ZRDINF, Record found: 1
Pathname: /ADOT EAST BASIN/AT SUPERSTITIION/FLOW/02APR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers: 25; /ADOT EAST BASIN/AT SUPERSTITIION/FLOW/02APR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRTS; Unit: 71
NSTART: 290 NVALS: 601 JULS: 35519 ISTEIME: 1440
NLDATA: 288 JULSD: 35521
JULS: 31MAR97 JULSD: 02APR97
Quality Read: F, Quality Requested: F
---ZRTS Calculations: NPOS: 1 NDATA: 288 NREAD: 288 ILIM: 577
After ZRDINF, Record found: 1
Pathname: /ADOT EAST BASIN/AT SUPERSTITIION/FLOW/03APR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers: 25; /ADOT EAST BASIN/AT SUPERSTITIION/FLOW/03APR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRTS; Unit: 71
NSTART: 578 NVALS: 601 JULS: 35519 ISTEIME: 1440
NLDATA: 288 JULSD: 35522
JULS: 31MAR97 JULSD: 03APR97
Quality Read: F, Quality Requested: F
---ZRTS Calculations: NPOS: 1 NDATA: 288 NREAD: 24 ILIM: 601
---- Exiting ZRTS, Number of data values: 601, Status: 0

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Offset:      0, Units: CFS      , Type:INST-VAL
----- Entering ZRRTSX for unit 71 -----
Pathname: /ADOT WEST BASIN/AT SUPERSTITION/FLOW//5MIN/100YR/
Time Window set. Interval:      5 Number of data values:      1
Starting date and time: Mar 31, 1997 2400 ( 35519 1440)
Ending date and time: Mar 31, 1997 2400 ( 35519 1440)
Input time offset:      0
After ZRDINF, Record found: I
Pathname: /ADOT WEST BASIN/AT SUPERSTITION/FLOW/31MAR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 25: /ADOT WEST BASIN/AT SUPERSTITION/FLOW/31MAR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 1 NVALS: 1 JULS: 35519 IETIME: 1440
NLDATA: 288 JULSD: 35519
JULS: 31MAR97 JULSD: 31MAR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 288 NDATA: 288 NREAD: 1 ILIM: 1
--- Exiting ZRRTS, Number of data values: 1, Status: 0
Offset:      0, Units: CFS      , Type:INST-VAL
----- Entering ZRRTSX for unit 71 -----
Pathname: /ADOT WEST BASIN/AT SUPERSTITION/FLOW//5MIN/100YR/
Time Window set. Interval:      5 Number of data values:     601
Starting date and time: Mar 31, 1997 2400 ( 35519 1440)
Ending date and time: Apr 3, 1997 0200 ( 35522 120)
Input time offset:      0
After ZRDINF, Record found: I
Pathname: /ADOT WEST BASIN/AT SUPERSTITION/FLOW/31MAR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 25: /ADOT WEST BASIN/AT SUPERSTITION/FLOW/31MAR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 1 NVALS: 601 JULS: 35519 IETIME: 1440
NLDATA: 288 JULSD: 35519
JULS: 31MAR97 JULSD: 31MAR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 288 NDATA: 288 NREAD: 1 ILIM: 1
After ZRDINF, Record found: I
Pathname: /ADOT WEST BASIN/AT SUPERSTITION/FLOW/01APR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 25: /ADOT WEST BASIN/AT SUPERSTITION/FLOW/01APR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 2 NVALS: 601 JULS: 35519 IETIME: 1440
NLDATA: 288 JULSD: 35520
JULS: 31MAR97 JULSD: 01APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 288 ILIM: 289
After ZRDINF, Record found: I
Pathname: /ADOT WEST BASIN/AT SUPERSTITION/FLOW/02APR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 25: /ADOT WEST BASIN/AT SUPERSTITION/FLOW/02APR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 290 NVALS: 601 JULS: 35519 IETIME: 1440
NLDATA: 288 JULSD: 35521
JULS: 31MAR97 JULSD: 02APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 288 ILIM: 577
After ZRDINF, Record found: I
Pathname: /ADOT WEST BASIN/AT SUPERSTITION/FLOW/03APR1997/5MIN/100YR/
Number of actual data: 288 Header length: 0
Compression: 0 Quality: 0
-----DSS--- ZREAD Unit 71; Vers. 25: /ADOT WEST BASIN/AT SUPERSTITION/FLOW/03APR1997/5MIN/100YR/
-----DSS---Debug: Enter ZRRTSB; Unit: 71
NSTART: 578 NVALS: 601 JULS: 35519 IETIME: 1440
NLDATA: 288 JULSD: 35522
JULS: 31MAR97 JULSD: 03APR97
Quality Read: F, Quality Requested: F
---ZRRTSB Calculations: NPOS: 1 NDATA: 288 NREAD: 24 ILIM: 601
--- Exiting ZRRTS, Number of data values: 601, Status: 0
Offset:      0, Units: CFS      , Type:INST-VAL
WARNING EXCESS AT PONDING LESS THAN ZERO FOR PERIOD. EXCESS SET TO ZERO
WARNING EXCESS AT PONDING LESS THAN ZERO FOR PERIOD. EXCESS SET TO ZERO

WARNING --- ROUTED OUTFLOW ( 316.) IS GREATER THAN MAXIMUM OUTFLOW ( 298.) IN STORAGE-OUTFLOW TABLE
WARNING --- ROUTED OUTFLOW ( 342.) IS GREATER THAN MAXIMUM OUTFLOW ( 298.) IN STORAGE-OUTFLOW TABLE
WARNING --- ROUTED OUTFLOW ( 359.) IS GREATER THAN MAXIMUM OUTFLOW ( 298.) IN STORAGE-OUTFLOW TABLE
WARNING --- ROUTED OUTFLOW ( 369.) IS GREATER THAN MAXIMUM OUTFLOW ( 298.) IN STORAGE-OUTFLOW TABLE
WARNING --- ROUTED OUTFLOW ( 371.) IS GREATER THAN MAXIMUM OUTFLOW ( 298.) IN STORAGE-OUTFLOW TABLE
WARNING --- ROUTED OUTFLOW ( 367.) IS GREATER THAN MAXIMUM OUTFLOW ( 298.) IN STORAGE-OUTFLOW TABLE
WARNING --- ROUTED OUTFLOW ( 359.) IS GREATER THAN MAXIMUM OUTFLOW ( 298.) IN STORAGE-OUTFLOW TABLE
WARNING --- ROUTED OUTFLOW ( 347.) IS GREATER THAN MAXIMUM OUTFLOW ( 298.) IN STORAGE-OUTFLOW TABLE
WARNING --- ROUTED OUTFLOW ( 332.) IS GREATER THAN MAXIMUM OUTFLOW ( 298.) IN STORAGE-OUTFLOW TABLE
WARNING --- ROUTED OUTFLOW ( 317.) IS GREATER THAN MAXIMUM OUTFLOW ( 298.) IN STORAGE-OUTFLOW TABLE
WARNING --- ROUTED OUTFLOW ( 302.) IS GREATER THAN MAXIMUM OUTFLOW ( 298.) IN STORAGE-OUTFLOW TABLE
WARNING --- ROUTED OUTFLOW ( 306.) IS GREATER THAN MAXIMUM OUTFLOW ( 298.) IN STORAGE-OUTFLOW TABLE

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WARNING --- ROUTED OUTFLOW ( 737.) IS GREATER THAN MAXIMUM OUTFLOW ( 654.) IN STORAGE-OUTFLOW TABLE  
 WARNING --- ROUTED OUTFLOW ( 760.) IS GREATER THAN MAXIMUM OUTFLOW ( 654.) IN STORAGE-OUTFLOW TABLE  
 WARNING --- ROUTED OUTFLOW ( 776.) IS GREATER THAN MAXIMUM OUTFLOW ( 654.) IN STORAGE-OUTFLOW TABLE  
 WARNING --- ROUTED OUTFLOW ( 785.) IS GREATER THAN MAXIMUM OUTFLOW ( 654.) IN STORAGE-OUTFLOW TABLE  
 WARNING --- ROUTED OUTFLOW ( 789.) IS GREATER THAN MAXIMUM OUTFLOW ( 654.) IN STORAGE-OUTFLOW TABLE  
 WARNING --- ROUTED OUTFLOW ( 787.) IS GREATER THAN MAXIMUM OUTFLOW ( 654.) IN STORAGE-OUTFLOW TABLE  
 WARNING --- ROUTED OUTFLOW ( 783.) IS GREATER THAN MAXIMUM OUTFLOW ( 654.) IN STORAGE-OUTFLOW TABLE  
 WARNING --- ROUTED OUTFLOW ( 778.) IS GREATER THAN MAXIMUM OUTFLOW ( 654.) IN STORAGE-OUTFLOW TABLE  
 WARNING --- ROUTED OUTFLOW ( 771.) IS GREATER THAN MAXIMUM OUTFLOW ( 654.) IN STORAGE-OUTFLOW TABLE  
 WARNING --- ROUTED OUTFLOW ( 764.) IS GREATER THAN MAXIMUM OUTFLOW ( 654.) IN STORAGE-OUTFLOW TABLE  
 WARNING --- ROUTED OUTFLOW ( 755.) IS GREATER THAN MAXIMUM OUTFLOW ( 654.) IN STORAGE-OUTFLOW TABLE  
 WARNING --- ROUTED OUTFLOW ( 743.) IS GREATER THAN MAXIMUM OUTFLOW ( 654.) IN STORAGE-OUTFLOW TABLE  
 WARNING --- ROUTED OUTFLOW ( 728.) IS GREATER THAN MAXIMUM OUTFLOW ( 654.) IN STORAGE-OUTFLOW TABLE  
 WARNING --- ROUTED OUTFLOW ( 710.) IS GREATER THAN MAXIMUM OUTFLOW ( 654.) IN STORAGE-OUTFLOW TABLE  
 WARNING --- ROUTED OUTFLOW ( 691.) IS GREATER THAN MAXIMUM OUTFLOW ( 654.) IN STORAGE-OUTFLOW TABLE  
 WARNING --- ROUTED OUTFLOW ( 671.) IS GREATER THAN MAXIMUM OUTFLOW ( 654.) IN STORAGE-OUTFLOW TABLE

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2771 KK \*\*\*\*\*  
 \* \*  
 \* R61B \*  
 \* \*  
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2773 KO OUTPUT CONTROL VARIABLES  
 IPRNT 3 PRINT CONTROL  
 IFLOT 0 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 PUNCH HYDROGRAPH ON THIS UNIT  
 ISAV1 1 SAVE HYDROGRAPH PUNCHED OR SAVED  
 ISAV2 600 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .083 TIME INTERVAL IN HOURS

DI DIVERSION IDENTIFICATION  
 DSIAD 081B DIVERSION HYDROGRAPH IDENTIFICATION  
 DSTIMX 35.00 MAXIMUM VOLUME TO BE DIVERTED  
 DI INFLOW .00 10000.00  
 DQ DIVERTED FLOW .00 10000.00

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DIVERSION HYDROGRAPH D81B  
 TRANSPOSITION AREA .0 SQ MI

PEAK FLOW + (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.92-HR
419.	11.92	55.	18.	8.	8.
		(INCHES) .605	.781	.781	.781
		(AC-FT) 27.	35.	35.	35.

CUMULATIVE AREA = .84 SQ MI

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HYDROGRAPH AT STATION R61B  
 TRANSPOSITION AREA .0 SQ MI

PEAK FLOW + (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.92-HR
1223.	12.25	155.	44.	21.	21.
		(INCHES) 1.713	1.927	1.927	1.927
		(AC-FT) 77.	86.	86.	86.

CUMULATIVE AREA = .84 SQ MI

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DIVERSION HYDROGRAPH  
 TRANSPOSITION AREA 1.0 SQ MI

PEAK FLOW (CFS)	TIME (HR)	6-HR	24-HR	72-HR	49.92-HR
445.	11.92	55.	18.	8.	8.
		(INCHES) .606	.781	.781	.781
		(AC-FT) 27.	35.	35.	35.
CUMULATIVE AREA =		.84 SQ MI			

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HYDROGRAPH AT STATION  
 TRANSPOSITION AREA 1.0 SQ MI

PEAK FLOW (CFS)	TIME (HR)	6-HR	24-HR	72-HR	49.92-HR
1215.	12.25	153.	43.	21.	21.
		(INCHES) 1.696	1.911	1.911	1.911
		(AC-FT) 76.	86.	86.	86.
CUMULATIVE AREA =		.84 SQ MI			

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DIVERSION HYDROGRAPH  
 TRANSPOSITION AREA 5.0 SQ MI

PEAK FLOW (CFS)	TIME (HR)	6-HR	24-HR	72-HR	49.92-HR
477.	11.92	55.	18.	8.	8.
		(INCHES) .607	.781	.781	.781
		(AC-FT) 27.	35.	35.	35.
CUMULATIVE AREA =		.84 SQ MI			

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HYDROGRAPH AT STATION  
 TRANSPOSITION AREA 5.0 SQ MI

PEAK FLOW (CFS)	TIME (HR)	6-HR	24-HR	72-HR	49.92-HR
1180.	12.25	148.	42.	20.	20.
		(INCHES) 1.634	1.838	1.838	1.838
		(AC-FT) 73.	82.	82.	82.
CUMULATIVE AREA =		.84 SQ MI			

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DIVERSION HYDROGRAPH  
 TRANSPOSITION AREA 10.0 SQ MI

PEAK FLOW (CFS)	TIME (HR)	6-HR	24-HR	72-HR	49.92-HR
458.	11.92	55.	18.	8.	8.
		(INCHES) .613	.781	.781	.781
		(AC-FT) 27.	35.	35.	35.
CUMULATIVE AREA =		.84 SQ MI			

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HYDROGRAPH AT STATION  
 TRANSPOSITION AREA 10.0 SQ MI

PEAK FLOW (CFS)	TIME (HR)	6-HR	24-HR	72-HR	49.92-HR
1138.	12.25	140.	39.	19.	19.
		(INCHES) 1.551	1.748	1.748	1.748
		(AC-FT) 69.	78.	78.	78.
CUMULATIVE AREA =		.84 SQ MI			

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DIVERSION HYDROGRAPH  
 TRANSPOSITION AREA 30.0 SQ MI

PEAK FLOW + (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.92-HR
+ 467.	12.00	(CFS) 56.	18.	8.	8.
		(INCHES) .620	.781	.781	.781
		(AC-FT) 28.	35.	35.	35.
		CUMULATIVE AREA = .84 SQ MI			

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HYDROGRAPH AT STATION R81B  
TRANSPPOSITION AREA 30.0 SQ MI

PEAK FLOW + (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.92-HR
+ 1083.	12.25	(CFS) 131.	37.	18.	18.
		(INCHES) 1.445	1.634	1.634	1.634
		(AC-FT) 65.	73.	73.	73.
		CUMULATIVE AREA = .84 SQ MI			

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DIVERSION HYDROGRAPH D81B  
TRANSPPOSITION AREA 60.0 SQ MI

PEAK FLOW + (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.92-HR
+ 591.	12.00	(CFS) 56.	18.	8.	8.
		(INCHES) .624	.781	.781	.781
		(AC-FT) 28.	35.	35.	35.
		CUMULATIVE AREA = .84 SQ MI			

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HYDROGRAPH AT STATION R81B  
TRANSPPOSITION AREA 60.0 SQ MI

PEAK FLOW + (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.92-HR
+ 1029.	12.25	(CFS) 121.	34.	17.	17.
		(INCHES) 1.342	1.520	1.520	1.520
		(AC-FT) 60.	68.	68.	68.
		CUMULATIVE AREA = .84 SQ MI			

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DIVERSION HYDROGRAPH D81B  
TRANSPPOSITION AREA 90.0 SQ MI

PEAK FLOW + (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.92-HR
+ 579.	12.00	(CFS) 57.	18.	8.	8.
		(INCHES) .627	.781	.781	.781
		(AC-FT) 28.	35.	35.	35.
		CUMULATIVE AREA = .84 SQ MI			

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HYDROGRAPH AT STATION R81B  
TRANSPPOSITION AREA 90.0 SQ MI

PEAK FLOW + (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.92-HR
+ 1009.	12.25	(CFS) 118.	33.	16.	16.
		(INCHES) 1.305	1.479	1.479	1.479
		(AC-FT) 58.	66.	66.	66.
		CUMULATIVE AREA = .84 SQ MI			

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DIVERSION HYDROGRAPH D81B  
TRANSPPOSITION AREA 120.0 SQ MI

PEAK FLOW + (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.92-HR

EMDUB9.txt

+ 566.	12.00	(CFS)	57.	18.	8.	8.
		(INCHES)	.629	.781	.781	.781
		(AC-FT)	28.	35.	35.	35.
		CUMULATIVE AREA =	.84 SQ MI			

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HYDROGRAPH AT STATION R81B  
 TRANSPOSITION AREA 120.0 SQ MI

PEAK FLOW	TIME		6-HR	24-HR	72-HR	49.92-HR
+ (CFS)	(HR)	(CFS)				
+ 990.	12.25	114.	32.	16.	16.	
		(INCHES)	1.267	1.438	1.438	1.438
		(AC-FT)	57.	64.	64.	64.
		CUMULATIVE AREA =	.84 SQ MI			

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DIVERSION HYDROGRAPH D81B  
 TRANSPOSITION AREA 150.0 SQ MI

PEAK FLOW	TIME		6-HR	24-HR	72-HR	49.92-HR
+ (CFS)	(HR)	(CFS)				
+ 559.	12.00	57.	18.	8.	8.	
		(INCHES)	.631	.781	.781	.781
		(AC-FT)	28.	35.	35.	35.
		CUMULATIVE AREA =	.84 SQ MI			

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HYDROGRAPH AT STATION R81B  
 TRANSPOSITION AREA 150.0 SQ MI

PEAK FLOW	TIME		6-HR	24-HR	72-HR	49.92-HR
+ (CFS)	(HR)	(CFS)				
+ 978.	12.25	112.	32.	15.	15.	
		(INCHES)	1.244	1.414	1.414	1.414
		(AC-FT)	56.	63.	63.	63.
		CUMULATIVE AREA =	.84 SQ MI			

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INTERPOLATED DIVERSION HYDROGRAPH AT D81B

PEAK FLOW	TIME		6-HR	24-HR	72-HR	49.92-HR
+ (CFS)	(HR)	(CFS)				
+ 444.	11.92	55.	18.	8.	8.	
		(INCHES)	.606	.781	.781	.781
		(AC-FT)	27.	35.	35.	35.
		CUMULATIVE AREA =	.84 SQ MI			

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INTERPOLATED HYDROGRAPH AT R81B

PEAK FLOW	TIME		6-HR	24-HR	72-HR	49.92-HR
+ (CFS)	(HR)	(CFS)				
+ 1215.	12.25	153.	43.	21.	21.	
		(INCHES)	1.698	1.911	1.911	1.911
		(AC-FT)	76.	86.	86.	86.
		CUMULATIVE AREA =	.84 SQ MI			

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2777 KK \*\*\*\*\*  
 \* 80B81B \*  
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					EMDU89.txt		
+		59B	637.	12.83	132.	39.	.94
+	DIVERSION TO	D59B	637.	12.83	119.	32.	.94
+	HYDROGRAPH AT	R59B	137.	13.67	24.	8.	.94
+	2 COMBINED AT	C59B	1708.	12.83	615.	219.	13.70
+	ROUTED TO	S9BT60	1616.	13.33	613.	219.	13.70
+	HYDROGRAPH AT	60	1069.	13.50	359.	112.	2.30
+	DIVERSION TO	D60	1069.	13.50	313.	86.	2.30
+	HYDROGRAPH AT	R60	355.	14.67	86.	27.	2.30
+	2 COMBINED AT	EMFGUA	1616.	13.33	658.	238.	16.00
+	ROUTED TO	GUATEL	1524.	13.58	629.	236.	16.00
+	HYDROGRAPH AT	64	924.	12.42	156.	51.	.81
+	DIVERSION TO	D64	924.	12.42	121.	34.	.81
+	HYDROGRAPH AT	R64	477.	12.75	57.	18.	.81
+	2 COMBINED AT	EMFELL	1595.	13.58	668.	249.	16.81
+	ROUTED TO	ELTWAR	1491.	14.00	646.	249.	16.81
+	HYDROGRAPH AT	62A	806.	12.00	89.	31.	.38
+	DIVERSION TO	D62A	806.	12.00	57.	17.	.38
+	HYDROGRAPH AT	R62A	544.	12.08	46.	14.	.38
+	ROUTED TO	62ATB	448.	12.17	46.	14.	.38
+	HYDROGRAPH AT	62B	544.	12.00	54.	19.	.23
+	DIVERSION TO	D62B	454.	11.92	33.	10.	.23
+	HYDROGRAPH AT	R62B	470.	12.08	30.	9.	.23
+	2 COMBINED AT	C62B	659.	12.17	76.	23.	.61
+	ROUTED TO	62BTD	394.	12.58	72.	23.	.61
+	HYDROGRAPH AT	62D	609.	12.17	86.	28.	.46
+	DIVERSION TO	D62D	609.	12.17	63.	18.	.46
+	HYDROGRAPH AT	R62D	275.	12.50	34.	10.	.46
+	2 COMBINED AT	CP62D	646.	12.50	106.	33.	1.07
+	ROUTED TO	62DTF	445.	13.25	105.	33.	1.07
+	HYDROGRAPH AT	62F	421.	12.17	49.	16.	.26
+	DIVERSION TO	D62F	421.	12.17	32.	9.	.26
+	HYDROGRAPH AT	R62F	365.	12.25	24.	7.	.26
+	2 COMBINED AT	CP62F	461.	13.25	126.	40.	1.33
+	ROUTED TO						

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+		62T63	415.	13.58	123.	40.	19.	1.33
+	HYDROGRAPH AT	63	1190.	12.33	177.	58.	28.	.91
+	DIVERSION TO	D63	1190.	12.33	128.	36.	17.	.91
+	HYDROGRAPH AT	R63	787.	12.50	74.	22.	11.	.91
+	2 COMBINED AT	CP63	720.	12.50	190.	61.	29.	2.24
+	ROUTED TO	63T71	354.	15.33	172.	61.	29.	2.24
+	HYDROGRAPH AT	68B1	295.	12.08	35.	12.	6.	.15
+	HYDROGRAPH AT	68B2	129.	12.08	14.	5.	2.	.06
+	HYDROGRAPH AT	68B3	85.	12.00	9.	3.	1.	.04
+	3 COMBINED AT	CP68	496.	12.08	58.	20.	10.	.24
+	DIVERSION TO	D68B	496.	12.08	42.	12.	6.	.24
+	HYDROGRAPH AT	R68	237.	12.25	25.	8.	4.	.24
+	ROUTED TO	68BT69	118.	12.58	24.	8.	4.	.24
+	HYDROGRAPH AT	69	208.	12.00	22.	7.	4.	.09
+	DIVERSION TO	D69	208.	12.00	16.	5.	2.	.09
+	HYDROGRAPH AT	R69	106.	12.17	9.	3.	1.	.09
+	2 COMBINED AT	C69	136.	12.58	33.	11.	5.	.33
+	ROUTED TO	69T71	76.	14.00	30.	11.	5.	.33
+	HYDROGRAPH AT	25	351.	12.17	50.	17.	8.	.21
+	DIVERSION TO	25RET	351.	12.17	39.	11.	5.	.21
+	HYDROGRAPH AT	RET25	138.	12.42	19.	6.	3.	.21
+	ROUTED TO	25T71	53.	13.92	19.	6.	3.	.21
+	HYDROGRAPH AT	71	1963.	12.17	258.	89.	43.	.86
+	DIVERSION TO	D71	1647.	12.08	143.	42.	20.	.86
+	HYDROGRAPH AT	R71	1963.	12.17	158.	46.	22.	.86
+	4 COMBINED AT	C71	1889.	12.17	332.	120.	58.	3.64
+	ROUTED TO	71T72	869.	12.83	328.	120.	58.	3.64
+	HYDROGRAPH AT	72	1422.	12.17	202.	69.	33.	.84
+	DIVERSION TO	D72	1422.	12.17	146.	42.	20.	.84
+	HYDROGRAPH AT	R72	755.	12.42	89.	28.	13.	.84
+	2 COMBINED AT	CEXNOX	1514.	12.42	407.	144.	69.	4.48
+	2 COMBINED AT	EMFWAR	1690.	14.00	911.	369.	184.	21.29
+	ROUTED TO	HARTKN	1657.	14.17	898.	368.	184.	21.29
+	HYDROGRAPH AT							

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+		26	98.	12.00	11.	4.	2.	.05
+	DIVERSION TO							
+		26RET	98.	12.00	9.	2.	1.	.05
+	HYDROGRAPH AT							
+		RET26	31.	12.25	4.	1.	1.	.05
+	ROUTED TO							
+		26T70B	13.	12.75	4.	1.	1.	.05
+	HYDROGRAPH AT							
+		70B	647.	12.17	92.	31.	15.	.34
+	DIVERSION TO							
+		D70B	647.	12.17	58.	17.	8.	.34
+	HYDROGRAPH AT							
+		R70B	512.	12.33	48.	14.	7.	.34
+	2 COMBINED AT							
+		CP70B	512.	12.33	52.	15.	7.	.38
+	ROUTED TO							
+		70BT76	160.	13.75	51.	15.	7.	.38
+	HYDROGRAPH AT							
+		76B	1157.	12.17	158.	53.	26.	.64
+	DIVERSION TO							
+		D76B	1157.	12.17	117.	33.	16.	.64
+	HYDROGRAPH AT							
+		R76B	539.	12.42	66.	20.	10.	.64
+	2 COMBINED AT							
+		KNOX	539.	12.42	116.	36.	17.	1.02
+	2 COMBINED AT							
+		EMFKNX	1800.	14.17	975.	395.	197.	22.31
+	ROUTED TO							
+		KNXTRY	1744.	14.33	962.	395.	196.	22.31
+	HYDROGRAPH AT							
+		65A	2593.	12.50	507.	168.	81.	2.54
+	DIVERSION TO							
+		D65A	2593.	12.50	306.	88.	42.	2.54
+	HYDROGRAPH AT							
+		R65A	2261.	12.67	275.	80.	39.	2.54
+	HYDROGRAPH AT							
+		CAP1A	629.	12.83	106.	31.	15.	6.40
+	ROUTED TO							
+		RCAP1A	517.	13.08	104.	31.	15.	6.40
+	ROUTED TO							
+		RRCF1A	511.	13.17	103.	31.	15.	6.40
+	HYDROGRAPH AT							
+		CAP1B	511.	13.17	103.	31.	15.	6.40
+	ROUTED TO							
+		RCAP1B	474.	13.25	102.	31.	15.	6.40
+	3 COMBINED AT							
+		C65A1	2252.	12.75	440.	132.	63.	15.34
+	ROUTED TO							
+		65ATB1	2052.	12.92	437.	132.	63.	15.34
+	DIVERSION TO							
+		DB65A	1618.	12.92	196.	49.	24.	15.34
+	HYDROGRAPH AT							
+		DIDB65	434.	12.75	241.	82.	40.	15.34
+	ROUTED TO							
+		65BIT2	467.	12.75	240.	82.	40.	15.34
+	HYDROGRAPH AT							
+		DIB65P	1618.	12.92	196.	49.	24.	15.34
+	ROUTED TO							
+		DB65A	74.	14.00	70.	46.	23.	15.34
+	2 COMBINED AT							
+		C65A2	508.	14.00	307.	128.	62.	15.34
+	ROUTED TO							
+		65AT-1	508.	14.00	307.	128.	62.	15.34
+	ROUTED TO							
+		65AT-2	499.	14.42	294.	127.	62.	15.34
+	HYDROGRAPH AT							

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+		65AW	555.	12.25	70.	21.	10.	.43
+	DIVERSION TO	D65AW	555.	12.25	58.	16.	8.	.43
+	HYDROGRAPH AT	R65AW	187.	12.58	19.	6.	3.	.43
+	ROUTED TO	65AT65	58.	13.67	17.	6.	3.	.43
+	HYDROGRAPH AT	65B	1552.	12.42	271.	88.	42.	1.37
+	DIVERSION TO	D65B	1552.	12.42	218.	60.	29.	1.37
+	HYDROGRAPH AT	R65B	669.	12.83	89.	28.	13.	1.37
+	2 COMBINED AT	CP65B	607.	12.83	102.	33.	16.	1.80
+	DIVERSION TO	DIRS65	577.	12.83	73.	18.	9.	1.80
+	HYDROGRAPH AT	DI65B	30.	12.83	30.	14.	7.	1.80
+	2 COMBINED AT	CP65A	529.	14.42	323.	140.	68.	17.14
+	ROUTED TO	65AT-3	529.	14.42	323.	140.	68.	17.14
+	HYDROGRAPH AT	DR65B	577.	12.83	73.	18.	9.	1.80
+	ROUTED TO	RS65A	11.	17.92	11.	10.	7.	1.80
+	2 COMBINED AT	CP65	537.	14.42	332.	148.	73.	17.14
+	ROUTED TO	65I66	537.	14.42	332.	148.	73.	17.14
+	ROUTED TO	65I66A	537.	14.42	332.	148.	73.	17.14
+	ROUTED TO	65I66B	537.	14.42	332.	148.	73.	17.14
+	HYDROGRAPH AT	ADDI-E	246.	14.42	175.	64.	31.	.01
+	ROUTED TO	AEI67A	245.	14.67	174.	64.	31.	.01
+	HYDROGRAPH AT	67A	387.	12.25	52.	17.	8.	.30
+	DIVERSION TO	D67A	387.	12.25	38.	11.	5.	.30
+	HYDROGRAPH AT	R67A	229.	12.50	20.	6.	3.	.30
+	2 COMBINED AT	C67A	255.	14.67	185.	70.	34.	.31
+	ROUTED TO	67ATC	253.	15.08	183.	70.	34.	.31
+	HYDROGRAPH AT	SUP2	422.	13.00	381.	215.	104.	.01
+	ROUTED TO	RSUP2	395.	16.25	379.	215.	104.	.01
+	HYDROGRAPH AT	67B	714.	12.25	105.	35.	17.	.53
+	DIVERSION TO	D67B	714.	12.25	73.	21.	10.	.53
+	HYDROGRAPH AT	R67B	457.	12.50	46.	14.	7.	.53
+	2 COMBINED AT	C67B	494.	12.50	396.	229.	111.	.54
+	ROUTED TO	67BTC	412.	16.33	396.	229.	111.	.54
+	HYDROGRAPH AT	67C	1019.	12.42	146.	44.	21.	.93
+	DIVERSION TO							



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+		D67C	1019.	12.42	126.	34.	16.	.93
+	HYDROGRAPH AT	R67C	295.	12.83	34.	11.	5.	.93
+	3 COMBINED AT	C67C	669.	15.25	595.	307.	149.	1.78
+	ROUTED TO	67CT67	669.	15.33	595.	307.	149.	1.78
+	HYDROGRAPH AT	67D	183.	12.17	20.	6.	3.	.13
+	DIVERSION TO	D67D	183.	12.17	17.	5.	2.	.13
+	HYDROGRAPH AT	R67D	52.	12.50	5.	1.	1.	.13
+	2 COMBINED AT	C67D	671.	15.33	597.	309.	150.	1.91
+	ROUTED TO	67DT66	671.	15.42	596.	309.	150.	1.91
+	HYDROGRAPH AT	66A	446.	12.17	46.	14.	7.	.26
+	DIVERSION TO	D66A	446.	12.17	39.	11.	5.	.26
+	HYDROGRAPH AT	R66A	105.	12.42	11.	3.	2.	.26
+	ROUTED TO	66ATB	39.	13.17	10.	3.	2.	.26
+	HYDROGRAPH AT	66B	605.	12.58	104.	32.	15.	.67
+	DIVERSION TO	D66B	605.	12.58	90.	24.	12.	.67
+	HYDROGRAPH AT	R66B	168.	13.08	24.	7.	4.	.67
+	2 COMBINED AT	CP66B	198.	13.08	34.	11.	5.	.93
+	ROUTED TO	66BTC	150.	13.42	33.	11.	5.	.93
+	HYDROGRAPH AT	66C	707.	12.25	94.	30.	14.	.50
+	DIVERSION TO	D66C	707.	12.25	77.	21.	10.	.50
+	HYDROGRAPH AT	R66C	257.	12.58	29.	9.	4.	.50
+	2 COMBINED AT	CP66C1	232.	12.58	58.	19.	9.	1.43
+	2 COMBINED AT	CP66C2	712.	15.08	633.	326.	158.	3.34
+	DIVERSION TO	DB66	302.	15.08	230.	70.	34.	3.34
+	HYDROGRAPH AT	D166	410.	15.08	403.	255.	124.	3.34
+	ROUTED TO	66C1T2	410.	15.08	403.	255.	124.	3.34
+	2 COMBINED AT	CP66C	942.	14.42	730.	393.	193.	20.48
+	ROUTED TO	66CTD	942.	14.42	730.	393.	193.	20.48
+	HYDROGRAPH AT	DA66	302.	15.08	230.	70.	34.	3.34
+	ROUTED TO	RS66D1	194.	17.92	145.	56.	31.	3.34
+	DIVERSION TO	D-WB	171.	17.92	124.	37.	18.	3.34
+	HYDROGRAPH AT	B-WA	22.	17.92	22.	19.	13.	3.34
+	2 COMBINED AT	C-WA	956.	14.50	747.	408.	205.	23.82
+	ROUTED TO							

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+		RC-WA	956.	14.50	797.	408.	205.
	HYDROGRAPH AT						23.82
+		DR-WA	171.	17.92	124.	37.	18.
	ROUTED TO						3.34
+		RS66D2	29.	23.08	27.	24.	15.
	2 COMBINED AT						3.34
+		CP66D	956.	14.50	754.	427.	219.
	ROUTED TO						23.82
+		66T66D	956.	14.50	754.	427.	219.
	ROUTED TO						23.82
+		66-66D	955.	14.50	754.	427.	219.
	HYDROGRAPH AT						23.82
+		66D	651.	12.08	75.	26.	12.
	DIVERISION TO						.31
+		D66D	651.	12.08	55.	16.	8.
	HYDROGRAPH AT						.31
+		R66D	350.	12.25	32.	10.	5.
	HYDROGRAPH AT						.31
+		61A	794.	12.17	95.	31.	15.
	DIVERSION TO						.52
+		D61A	794.	12.17	76.	21.	10.
	HYDROGRAPH AT						.52
+		R61A	333.	12.42	33.	10.	5.
	ROUTED TO						.52
+		61ATB	122.	13.25	31.	10.	5.
	HYDROGRAPH AT						.52
+		61B	1175.	12.42	176.	55.	26.
	DIVERSION TO						1.09
+		D61B	1175.	12.42	151.	41.	20.
	HYDROGRAPH AT						1.09
+		R61B	339.	12.83	45.	14.	7.
	2 COMBINED AT						1.09
+		CP61B	312.	12.92	73.	23.	11.
	ROUTED TO						1.61
+		61T66D	238.	13.67	71.	23.	11.
	HYDROGRAPH AT						1.61
+		67E	771.	12.25	111.	36.	17.
	DIVERSION TO						.58
+		D67E	771.	12.25	91.	25.	12.
	HYDROGRAPH AT						.58
+		R67E	268.	12.67	34.	11.	5.
	2 COMBINED AT						.58
+		C67E	267.	13.67	98.	33.	16.
	3 COMBINED AT						2.19
+		C66D	1078.	14.42	821.	455.	232.
	ROUTED TO						26.32
+		66T23A	1078.	14.42	821.	455.	232.
	ROUTED TO						26.32
+		66T23B	1077.	14.50	821.	454.	232.
	ROUTED TO						26.32
+		CULVT	1076.	14.50	821.	454.	232.
	ROUTED TO						26.32
+		66T23C	1076.	14.50	820.	454.	232.
	HYDROGRAPH AT						26.32
+		04	513.	12.33	74.	24.	12.
	DIVERSION TO						.31
+		04RET	513.	12.33	55.	15.	7.
	HYDROGRAPH AT						.31
+		RET04	283.	12.50	30.	9.	4.
	2 COMBINED AT						.31
+		CP23	1092.	14.50	831.	461.	235.
	ROUTED TO						26.63
+		66T23D	1087.	14.58	830.	461.	235.
	HYDROGRAPH AT						26.63
+		62C	835.	12.17	98.	32.	15.
	DIVERSION TO						.55

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+		D62C	754.	12.08	55.	16.	8.	.55
+	HYDROGRAPH AT	R62C	796.	12.25	56.	16.	8.	.55
+	ROUTED TO	62CTE	501.	12.50	55.	16.	8.	.55
+	HYDROGRAPH AT	62E	229.	12.17	27.	9.	4.	.15
+	DIVERSION TO	D62E	229.	12.17	22.	6.	3.	.15
+	HYDROGRAPH AT	R62E	75.	12.50	9.	3.	1.	.15
+	2 COMBINED AT	CP62E	576.	12.50	64.	19.	9.	.70
+	ROUTED TO	62T68A	348.	12.92	61.	19.	9.	.70
+	HYDROGRAPH AT	68A1	477.	12.08	35.	9.	4.	.30
+	HYDROGRAPH AT	68A2	107.	12.00	11.	4.	2.	.05
+	2 COMBINED AT	CP68A1	580.	12.08	46.	13.	6.	.34
+	DIVERSION TO	D68A	580.	12.08	46.	13.	6.	.34
+	HYDROGRAPH AT	R68A	0.	.00	0.	0.	0.	.34
+	2 COMBINED AT	CP68A2	348.	12.92	61.	19.	9.	1.05
+	ROUTED TO	68T70A	285.	13.33	60.	19.	9.	1.05
+	HYDROGRAPH AT	70A1	102.	12.08	13.	4.	2.	.05
+	HYDROGRAPH AT	23	390.	12.17	52.	18.	8.	.22
+	DIVERSION TO	23RET	390.	12.17	41.	11.	6.	.22
+	HYDROGRAPH AT	RET23	169.	12.42	20.	6.	3.	.22
+	4 COMBINED AT	C70A1	1156.	14.58	872.	481.	245.	27.94
+	ROUTED TO	70A1T2	1149.	14.75	871.	480.	245.	27.94
+	HYDROGRAPH AT	24	468.	12.17	63.	21.	10.	.25
+	DIVERSION TO	24RET	468.	12.17	47.	13.	6.	.25
+	HYDROGRAPH AT	RET24	223.	12.33	26.	8.	4.	.25
+	HYDROGRAPH AT	70A2	79.	12.08	9.	3.	1.	.04
+	3 COMBINED AT	CP70A2	1164.	14.75	882.	488.	249.	28.23
+	ROUTED TO	70T76A	1154.	15.08	880.	487.	248.	28.23
+	HYDROGRAPH AT	76A	2137.	12.58	427.	136.	66.	1.91
+	DIVERSION TO	D76A	2137.	12.58	338.	93.	45.	1.91
+	HYDROGRAPH AT	R76A	987.	13.00	141.	43.	21.	1.91
+	2 COMBINED AT	C76A	1226.	15.00	934.	516.	262.	30.14
+	ROUTED TO	76ATPR	1215.	15.42	932.	515.	261.	30.14
+	HYDROGRAPH AT	73A	378.	13.33	96.	24.	12.	.95
+	ROUTED TO							

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+		73ATB	355.	13.50	96.	24.	12.	.95
	HYDROGRAPH AT							
+		73B	748.	12.08	68.	20.	10.	.43
	DIVERSION TO							
+		73BRET	748.	12.08	68.	20.	10.	.43
	HYDROGRAPH AT							
+		RET73B	4.	20.75	2.	1.	0.	.43
	2 COMBINED AT							
+		CP73B	355.	13.50	96.	25.	12.	1.38
	ROUTED TO							
+		73BTC	332.	13.83	95.	24.	12.	1.38
	HYDROGRAPH AT							
+		73C	822.	12.25	94.	28.	14.	.58
	DIVERSION TO							
+		73CRET	822.	12.25	70.	19.	9.	.58
	HYDROGRAPH AT							
+		RET73C	501.	12.42	33.	10.	5.	.58
	2 COMBINED AT							
+		CP73C	441.	12.42	124.	33.	16.	1.96
	ROUTED TO							
+		73T74C	347.	14.08	122.	33.	16.	1.96
	HYDROGRAPH AT							
+		74A	306.	13.33	77.	19.	9.	.75
	ROUTED TO							
+		74ATB	300.	13.42	77.	19.	9.	.75
	HYDROGRAPH AT							
+		74B	455.	12.25	55.	16.	8.	.33
	DIVERSION TO							
+		74BRET	455.	12.25	33.	9.	4.	.33
	HYDROGRAPH AT							
+		RET74B	382.	12.33	27.	8.	4.	.33
	2 COMBINED AT							
+		CP74B	445.	12.33	103.	27.	13.	1.08
	ROUTED TO							
+		74BTC	410.	12.42	103.	27.	13.	1.08
	HYDROGRAPH AT							
+		74C	516.	12.25	62.	18.	9.	.34
	DIVERSION TO							
+		74CRET	516.	12.25	42.	11.	5.	.34
	HYDROGRAPH AT							
+		RET74C	360.	12.42	25.	7.	3.	.34
	3 COMBINED AT							
+		CP74C	713.	12.42	239.	64.	31.	3.39
	ROUTED TO							
+		74CT10	628.	12.58	239.	64.	31.	3.39
	HYDROGRAPH AT							
+		10	224.	12.42	30.	9.	4.	.17
	DIVERSION TO							
+		10RET	224.	12.42	24.	6.	3.	.17
	HYDROGRAPH AT							
+		RET10	97.	12.75	9.	3.	1.	.17
	2 COMBINED AT							
+		CP10	628.	12.58	247.	67.	32.	3.56
	ROUTED TO							
+		10T11	645.	12.58	246.	66.	32.	3.56
	HYDROGRAPH AT							
+		11	135.	12.08	11.	4.	2.	.06
	DIVERSION TO							
+		11RET	135.	12.08	9.	3.	1.	.06
	HYDROGRAPH AT							
+		RET11	27.	12.25	4.	1.	1.	.06
	2 COMBINED AT							
+		CP11	652.	12.58	249.	67.	32.	3.62
	ROUTED TO							
+		11T75	546.	13.00	248.	67.	32.	3.62
	HYDROGRAPH AT							

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+		02B	458.	12.17	48.	16.	8.	.24
+	DIVERSION TO							
+		02BRET	458.	12.17	41.	11.	6.	.24
+	HYDROGRAPH AT							
+		RET02B	90.	12.50	14.	4.	2.	.24
+	ROUTED TO							
+		2BT1	80.	12.50	14.	4.	2.	.24
+	HYDROGRAPH AT							
+		01	437.	12.33	51.	14.	7.	.32
+	HYDROGRAPH AT							
+		03	397.	12.08	39.	13.	6.	.16
+	3 COMBINED AT							
+		CP1	633.	12.08	101.	31.	15.	.72
+	DIVERSION TO							
+		01RET	633.	12.08	89.	24.	11.	.72
+	HYDROGRAPH AT							
+		RET01	102.	13.00	23.	8.	4.	.72
+	ROUTED TO							
+		1T6	85.	13.08	23.	8.	4.	.72
+	HYDROGRAPH AT							
+		02A	379.	12.17	35.	11.	5.	.18
+	DIVERSION TO							
+		02ARET	379.	12.17	29.	8.	4.	.18
+	HYDROGRAPH AT							
+		RET02A	113.	12.33	10.	3.	2.	.18
+	HYDROGRAPH AT							
+		02C	360.	12.17	38.	12.	6.	.19
+	DIVERSION TO							
+		02CRET	360.	12.17	38.	12.	6.	.19
+	HYDROGRAPH AT							
+		RET02C	0.	.00	0.	0.	0.	.19
+	2 COMBINED AT							
+		CP2	113.	12.33	10.	3.	2.	.37
+	ROUTED TO							
+		2AT6	59.	12.50	10.	3.	2.	.37
+	HYDROGRAPH AT							
+		06	459.	12.25	51.	16.	8.	.28
+	DIVERSION TO							
+		06RET	459.	12.25	49.	13.	6.	.28
+	HYDROGRAPH AT							
+		RET06	21.	13.33	8.	3.	1.	.28
+	3 COMBINED AT							
+		CP6	87.	13.33	38.	13.	6.	1.37
+	ROUTED TO							
+		6T5	82.	13.42	38.	13.	6.	1.37
+	HYDROGRAPH AT							
+		05	283.	12.17	32.	9.	5.	.14
+	DIVERSION TO							
+		05RET	283.	12.17	25.	7.	3.	.14
+	HYDROGRAPH AT							
+		RET05	80.	12.50	9.	3.	1.	.14
+	HYDROGRAPH AT							
+		07	370.	12.33	41.	12.	6.	.24
+	DIVERSION TO							
+		07RET	370.	12.33	39.	10.	5.	.24
+	HYDROGRAPH AT							
+		RET07	28.	13.00	4.	1.	1.	.24
+	3 COMBINED AT							
+		CP5	99.	12.50	49.	17.	8.	1.76
+	ROUTED TO							
+		5T12	94.	13.50	49.	17.	8.	1.76
+	HYDROGRAPH AT							
+		12	567.	12.17	52.	16.	8.	.27
+	DIVERSION TO							
+		12RET	567.	12.17	46.	12.	6.	.27
+	HYDROGRAPH AT							

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+		RET12	51.	12.50	10.	3.	2.	.27
+	2 COMBINED AT	CP12	120.	12.67	57.	20.	9.	2.03
+	ROUTED TO	12T13	100.	13.58	54.	20.	9.	2.03
+	HYDROGRAPH AT	08	836.	12.58	132.	40.	19.	.77
+	DIVERSION TO	08RET	836.	12.58	122.	33.	16.	.77
+	HYDROGRAPH AT	RET08	134.	13.25	23.	7.	4.	.77
+	ROUTED TO	8T9	108.	13.33	22.	7.	4.	.77
+	HYDROGRAPH AT	09	303.	12.17	27.	9.	4.	.16
+	DIVERSION TO	09RET	303.	12.17	25.	7.	3.	.16
+	HYDROGRAPH AT	RET09	17.	12.67	5.	2.	1.	.16
+	2 COMBINED AT	CP9	117.	13.33	27.	9.	4.	.92
+	ROUTED TO	9T13	83.	13.58	26.	9.	4.	.92
+	HYDROGRAPH AT	13	481.	12.25	55.	18.	8.	.31
+	DIVERSION TO	13RET	481.	12.25	51.	14.	7.	.31
+	HYDROGRAPH AT	RET13	38.	12.92	11.	4.	2.	.31
+	3 COMBINED AT	CP13	157.	13.83	83.	30.	14.	3.26
+	ROUTED TO	13T75	156.	13.83	83.	30.	14.	3.26
+	HYDROGRAPH AT	14	229.	12.42	32.	10.	5.	.19
+	DIVERSION TO	14RET	229.	12.42	29.	8.	4.	.19
+	HYDROGRAPH AT	RET14	37.	13.00	7.	2.	1.	.19
+	3 COMBINED AT	CP75	613.	14.08	301.	91.	44.	7.07
+	ROUTED TO	75TPC	611.	14.17	300.	91.	44.	7.07
+	HYDROGRAPH AT	77A	556.	13.75	174.	43.	21.	1.74
+	ROUTED TO	77ATB	525.	13.83	173.	43.	21.	1.74
+	HYDROGRAPH AT	77B	542.	12.17	48.	14.	7.	.35
+	DIVERSION TO	77BRET	529.	12.08	31.	8.	4.	.35
+	HYDROGRAPH AT	RET77B	455.	12.25	20.	5.	3.	.35
+	2 COMBINED AT	CP77B	529.	13.83	191.	49.	23.	2.09
+	ROUTED TO	77B7C	503.	14.08	188.	49.	23.	2.09
+	HYDROGRAPH AT	77C	407.	12.25	46.	14.	7.	.28
+	DIVERSION TO	77CRET	407.	12.25	31.	8.	4.	.28
+	HYDROGRAPH AT	RET77C	280.	12.33	19.	5.	3.	.28
+	2 COMBINED AT	C77C	510.	14.08	206.	54.	26.	2.37
+	ROUTED TO							

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+		77CT78	493.	14.42	202.	54.	2.37
+	HYDROGRAPH AT						
+		78A	601.	13.75	188.	47.	1.88
+	ROUTED TO						
+		78ATB	520.	14.42	187.	47.	1.88
+	HYDROGRAPH AT						
+		78B	598.	12.25	62.	17.	.40
+	2 COMBINED AT						
+		C7BB	608.	12.25	245.	64.	2.28
+	ROUTED TO						
+		78BYC	501.	14.75	245.	64.	2.28
+	HYDROGRAPH AT						
+		78C	494.	12.17	44.	11.	.29
+	DIVERSION TO						
+		78CRET	83.	11.75	4.	1.	.29
+	HYDROGRAPH AT						
+		RET78C	494.	12.17	40.	10.	.29
+	2 COMBINED AT						
+		C78C	821.	12.17	283.	73.	2.56
+	2 COMBINED AT						
+		C78C2	946.	14.58	470.	124.	4.93
+	ROUTED TO						
+		78CT79	935.	14.75	466.	124.	4.93
+	HYDROGRAPH AT						
+		20	451.	12.33	58.	17.	.31
+	DIVERSION TO						
+		20RET	451.	12.33	46.	12.	.31
+	HYDROGRAPH AT						
+		RET20	198.	12.67	17.	5.	.31
+	2 COMBINED AT						
+		CP22B	939.	14.75	478.	128.	5.24
+	HYDROGRAPH AT						
+		16	212.	12.17	19.	6.	.11
+	DIVERSION TO						
+		16RET	212.	12.17	16.	4.	.11
+	HYDROGRAPH AT						
+		RET16	57.	12.42	5.	1.	.11
+	HYDROGRAPH AT						
+		18	501.	12.25	52.	16.	.32
+	DIVERSION TO						
+		18RET	501.	12.25	48.	13.	.32
+	HYDROGRAPH AT						
+		RET18	52.	12.75	9.	3.	.32
+	ROUTED TO						
+		18I19	49.	12.83	9.	3.	.32
+	2 COMBINED AT						
+		CP19A	58.	12.83	14.	4.	.43
+	HYDROGRAPH AT						
+		19	206.	12.17	20.	6.	.10
+	DIVERSION TO						
+		19RET	206.	12.17	15.	4.	.10
+	HYDROGRAPH AT						
+		RET19	69.	12.42	6.	2.	.10
+	2 COMBINED AT						
+		CP19B	126.	12.42	20.	6.	.53
+	HYDROGRAPH AT						
+		17	151.	12.42	19.	6.	.13
+	DIVERSION TO						
+		17RET	151.	12.42	19.	5.	.13
+	HYDROGRAPH AT						
+		RET17	5.	13.83	2.	1.	.13
+	HYDROGRAPH AT						
+		79A	645.	13.25	155.	39.	1.07
+	4 COMBINED AT						
+		CP79A1	1093.	13.25	625.	166.	6.97
+	HYDROGRAPH AT						

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+		78F	1077.	14.17	403.	102.	49.	4.19
+	HYDROGRAPH AT	82A1	1123.	13.50	307.	77.	37.	3.12
+	2 COMBINED AT	C82A1	1672.	13.50	601.	172.	83.	7.31
+	ROUTED TO	DB82A1	183.	16.17	157.	135.	108.	7.31
+	ROUTED TO	PS-9	183.	16.17	157.	135.	108.	7.31
+	HYDROGRAPH AT	CAP2	64.	2.00	64.	64.	62.	.01
+	ROUTED TO	RCAP2	64.	4.00	64.	64.	61.	.01
+	HYDROGRAPH AT	82A2	879.	14.75	392.	104.	50.	4.13
+	2 COMBINED AT	CP82A2	943.	14.75	456.	168.	111.	4.14
+	HYDROGRAPH AT	82A4	631.	13.83	210.	53.	25.	2.13
+	ROUTED TO	82A4T3	623.	14.00	210.	53.	25.	2.13
+	HYDROGRAPH AT	82A3	536.	14.17	199.	50.	24.	2.02
+	2 COMBINED AT	CP82A3	1108.	14.08	400.	101.	49.	4.15
+	2 COMBINED AT	CP82A5	1643.	14.08	819.	259.	155.	8.29
+	ROUTED TO	DB82B	357.	16.83	324.	252.	153.	8.29
+	ROUTED TO	MN-1	357.	16.92	324.	252.	153.	8.29
+	2 COMBINED AT	CP82A6	477.	17.00	460.	376.	254.	15.60
+	ROUTED TO	82TBOX	477.	17.08	460.	376.	254.	15.60
+	ROUTED TO	BOXCLV	477.	17.08	460.	376.	254.	15.60
+	ROUTED TO	BOXI78	477.	17.33	460.	375.	253.	15.60
+	HYDROGRAPH AT	78D	1545.	12.17	194.	62.	30.	.89
+	DIVERSION TO	D78D	1545.	12.17	154.	42.	20.	.89
+	HYDROGRAPH AT	R78D	550.	12.42	64.	20.	9.	.89
+	HYDROGRAPH AT	82B	1558.	12.17	181.	59.	29.	.92
+	DIVERSION TO	D82	14.	1.08	2.	1.	0.	.92
+	HYDROGRAPH AT	R82	1558.	12.17	181.	59.	28.	.92
+	DIVERSION TO	TRW	1558.	12.17	181.	56.	27.	.92
+	HYDROGRAPH AT	DTTRW	19.	19.33	12.	3.	1.	.92
+	3 COMBINED AT	C78D	1111.	12.58	521.	430.	287.	17.41
+	ROUTED TO	78DTE	495.	17.75	477.	387.	258.	17.41
+	HYDROGRAPH AT	78E	838.	12.75	158.	40.	19.	1.01
+	HYDROGRAPH AT	83	1378.	12.25	198.	65.	31.	1.01
+	DIVERSION TO	D83	1378.	12.25	150.	42.	20.	1.01
+	HYDROGRAPH AT							



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+		R83	731.	12.50	76.	23.	11.	1.01
+	3 COMBINED AT							
		C78C	1184.	12.75	571.	432.	282.	19.43
+	ROUTED TO							
		76ET84	1055.	13.17	568.	431.	282.	19.43
+	HYDROGRAPH AT							
		84	1309.	12.25	193.	63.	30.	.99
+	DIVERSION TO							
		D84	1309.	12.25	154.	43.	21.	.99
+	HYDROGRAPH AT							
		R84	599.	12.58	66.	21.	10.	.99
+	2 COMBINED AT							
		C84	1174.	13.08	610.	445.	288.	20.42
+	ROUTED TO							
		84T79B	1142.	13.17	609.	445.	288.	20.42
+	HYDROGRAPH AT							
		79B	674.	13.08	161.	40.	19.	1.00
+	2 COMBINED AT							
		C79B1	1721.	13.17	723.	477.	304.	21.42
+	ROUTED TO							
		79BTB2	1635.	13.33	721.	476.	304.	21.42
+	2 COMBINED AT							
		C79B2	2508.	13.33	1241.	615.	370.	28.39
+	ROUTED TO							
		79TPC2	2489.	13.42	1238.	615.	370.	28.39
+	2 COMBINED AT							
		CPPWR	2692.	13.42	1415.	670.	395.	35.46
+	ROUTED TO							
		PWKT80	2670.	13.50	1413.	669.	395.	35.46
+	HYDROGRAPH AT							
		80A	2421.	12.83	595.	189.	91.	2.64
+	DIVERSION TO							
		D80A	73.	8.83	53.	17.	8.	2.64
+	HYDROGRAPH AT							
		R80A	2421.	12.83	595.	172.	83.	2.64
+	2 COMBINED AT							
		CP80A	3769.	12.83	1869.	804.	465.	38.10
+	ROUTED TO							
		PWRSAN	3710.	13.00	1866.	804.	465.	38.10
+	2 COMBINED AT							
		CPSAN	3512.	13.00	2373.	1233.	680.	68.24
+	ROUTED TO							
		PWREMF	3454.	13.25	2367.	1233.	679.	68.24
+	2 COMBINED AT							
		ENEP0W	4226.	14.33	3105.	1556.	840.	90.55
+	ROUTED TO							
		POWZWI	4196.	14.50	3097.	1556.	838.	90.55
+	HYDROGRAPH AT							
		80B	1198.	12.50	223.	71.	34.	1.12
+	DIVERSION TO							
		D80B	15.	4.00	8.	2.	1.	1.12
+	HYDROGRAPH AT							
		R80B	1198.	12.50	223.	69.	33.	1.12
+	HYDROGRAPH AT							
		81B	1215.	12.25	180.	61.	29.	.84
+	DIVERSION TO							
		D81B	444.	11.92	55.	18.	8.	.84
+	HYDROGRAPH AT							
		R81B	1215.	12.25	153.	43.	21.	.84
+	2 COMBINED AT							
		80B81B	2107.	12.33	365.	111.	53.	1.96
+	2 COMBINED AT							
		EMFWIL	4261.	14.50	3196.	1633.	880.	92.51
+	ROUTED TO							
		WILTSP	4221.	14.75	3194.	1633.	878.	92.51
+	HYDROGRAPH AT							

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+		SUB258	1024.	13.50	462.	136.	66.	3.65
	ROUTED TO	RO259	952.	15.17	459.	136.	66.	3.65
+	HYDROGRAPH AT	SUB260	1151.	12.33	141.	42.	20.	.98
+	DIVERSION TO	RETDIV	1151.	12.33	131.	35.	17.	.98
+	HYDROGRAPH AT	R260	123.	12.92	21.	7.	3.	.98
+	2 COMBINED AT	CO262	962.	15.17	473.	141.	68.	4.63
+	ROUTED TO	RO263	913.	16.75	471.	141.	68.	4.63
+	HYDROGRAPH AT	SUB264	1066.	12.42	154.	47.	23.	1.00
+	DIVERSION TO	RETDIV	1066.	12.42	137.	37.	18.	1.00
+	HYDROGRAPH AT	R264	235.	12.92	32.	10.	5.	1.00
+	2 COMBINED AT	CO266	920.	16.75	480.	148.	71.	5.63
+	ROUTED TO	RO267	771.	20.08	471.	147.	71.	5.63
+	HYDROGRAPH AT	SUB268	736.	12.75	154.	48.	23.	.97
+	DIVERSION TO	RETDIV	736.	12.75	126.	34.	16.	.97
+	HYDROGRAPH AT	R268	306.	13.33	44.	13.	6.	.97
+	2 COMBINED AT	CO270	774.	20.08	475.	157.	76.	6.60
+	ROUTED TO	RO283	708.	22.92	467.	156.	76.	6.60
+	HYDROGRAPH AT	88A	1139.	12.00	119.	41.	20.	.50
+	DIVERSION TO	D88A	1139.	12.00	88.	25.	12.	.50
+	HYDROGRAPH AT	R88A	602.	12.17	51.	16.	8.	.50
+	ROUTED TO	88A189	198.	12.56	48.	16.	8.	.50
+	HYDROGRAPH AT	89A	1009.	12.08	118.	41.	20.	.50
+	DIVERSION TO	D89A	1009.	12.08	88.	25.	12.	.50
+	HYDROGRAPH AT	R89A	565.	12.25	50.	16.	7.	.50
+	2 COMBINED AT	C89A	566.	12.25	98.	31.	15.	1.00
+	ROUTED TO	89ATR1	319.	12.67	95.	31.	15.	1.00
+	2 COMBINED AT	C283	922.	22.92	480.	162.	88.	7.60
+	ROUTED TO	283T90	919.	23.17	479.	162.	88.	7.60
+	HYDROGRAPH AT	90A	939.	12.08	98.	33.	16.	.48
+	DIVERSION TO	D90A	939.	12.08	76.	21.	10.	.48
+	HYDROGRAPH AT	R90A	456.	12.25	37.	12.	6.	.48
+	2 COMBINED AT	C90A	724.	23.17	481.	190.	92.	8.08
+	ROUTED TO	90ATB	720.	23.33	480.	190.	92.	8.08
+	HYDROGRAPH AT							

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+		87A	1041.	12.08	116.	40.	19.	.49
+	DIVERSION TO	D87A	1041.	12.08	87.	25.	12.	.49
+	HYDROGRAPH AT	R87A	435.	12.25	49.	15.	7.	.49
+	ROUTED TO	87A1B	182.	12.75	47.	15.	7.	.49
+	HYDROGRAPH AT	87B	1022.	12.08	117.	40.	19.	.49
+	DIVERSION TO	D87B	1022.	12.08	87.	25.	12.	.49
+	HYDROGRAPH AT	R87B	512.	12.25	50.	15.	7.	.49
+	2 COMBINED AT	C87	512.	12.25	95.	31.	15.	.98
+	ROUTED TO	87I88B	248.	12.92	92.	31.	15.	.98
+	HYDROGRAPH AT	88B	1034.	12.08	118.	41.	20.	.50
+	DIVERSION TO	D88B	1034.	12.08	89.	25.	12.	.50
+	HYDROGRAPH AT	R88B	518.	12.25	49.	15.	7.	.50
+	2 COMBINED AT	C88B	505.	12.25	137.	45.	22.	1.48
+	ROUTED TO	88T89B	314.	13.33	134.	45.	22.	1.48
+	HYDROGRAPH AT	89B	1047.	12.08	117.	40.	19.	.50
+	DIVERSION TO	D89B	1047.	12.08	87.	25.	12.	.50
+	HYDROGRAPH AT	R89B	438.	12.17	50.	16.	7.	.50
+	2 COMBINED AT	C89B	433.	12.25	177.	60.	29.	1.98
+	ROUTED TO	89T890	355.	13.92	172.	60.	29.	1.98
+	HYDROGRAPH AT	90B	668.	12.67	128.	39.	19.	.82
+	DIVERSION TO	D90B	379.	12.25	39.	12.	6.	.82
+	HYDROGRAPH AT	R90B	668.	12.67	101.	27.	13.	.82
+	3 COMBINED AT	C90	752.	23.33	509.	260.	126.	10.88
+	ROUTED TO	90T91	748.	23.75	508.	259.	126.	10.88
+	HYDROGRAPH AT	85	1328.	12.25	192.	63.	31.	1.00
+	DIVERSION TO	D85	1328.	12.25	152.	42.	20.	1.00
+	HYDROGRAPH AT	R85	591.	12.58	68.	21.	10.	1.00
+	ROUTED TO	85T86	334.	12.92	66.	21.	10.	1.00
+	HYDROGRAPH AT	86	1342.	12.25	193.	64.	31.	1.00
+	DIVERSION TO	D86	1342.	12.25	154.	43.	21.	1.00
+	HYDROGRAPH AT	R86	594.	12.58	67.	21.	10.	1.00
+	2 COMBINED AT	C86	495.	12.58	126.	40.	19.	2.00
+	ROUTED TO	86T91	454.	13.17	123.	40.	19.	2.00
+	HYDROGRAPH AT							

				EMDU89.txt				
+		91	667.	12.25	89.	29.	14.	.46
+	DIVERSION TO	D91	667.	12.25	69.	19.	9.	.46
+	HYDROGRAPH AT	RET91	302.	12.50	33.	10.	5.	.46
+	HYDROGRAPH AT	81A	1656.	12.67	359.	119.	57.	1.81
+	DIVERSION TO	D81A	29.	2.67	10.	3.	1.	1.81
+	HYDROGRAPH AT	R81A	1656.	12.67	359.	117.	56.	1.81
+	4 COMBINED AT	CP91	1667.	12.67	718.	387.	193.	15.15
+	ROUTED TO	91TEMP	1591.	12.75	717.	387.	193.	15.15
+	2 COMBINED AT	EMFRIT	4514.	13.58	3602.	1905.	1017.	107.66

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SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING  
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

I STAQ	ELEMENT	DT (MIN)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	INTERPOLATED TO COMPUTATION INTERVAL			VOLUME (IN)
						PEAK (CFS)	TIME TO PEAK (MIN)		
FOR STORM = 1	STORM AREA (SQ MI) =			.01					
65AT-3	MANE	.66	536.60	864.90	.34	5.00	536.59	865.00	.34
CONTINUITY SUMMARY (AC-FT) - INFLOW= .3128E+03 EXCESS= .0000E+00 OUTFLOW= .3128E+03 BASIN STORAGE= .4892E-02 PERCENT ERROR= .0									
FOR STORM = 2	STORM AREA (SQ MI) =			1.00					
65AT-3	MANE	.66	536.11	865.06	.34	5.00	536.11	865.00	.34
CONTINUITY SUMMARY (AC-FT) - INFLOW= .3107E+03 EXCESS= .0000E+00 OUTFLOW= .3107E+03 BASIN STORAGE= .4697E-02 PERCENT ERROR= .0									
FOR STORM = 3	STORM AREA (SQ MI) =			5.00					
65AT-3	MANE	.66	533.76	865.17	.33	5.00	533.76	865.00	.33
CONTINUITY SUMMARY (AC-FT) - INFLOW= .3011E+03 EXCESS= .0000E+00 OUTFLOW= .3011E+03 BASIN STORAGE= .3984E-02 PERCENT ERROR= .0									
FOR STORM = 4	STORM AREA (SQ MI) =			10.00					
65AT-3	MANE	.66	530.80	864.82	.32	5.00	530.79	865.00	.32
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2893E+03 EXCESS= .0000E+00 OUTFLOW= .2893E+03 BASIN STORAGE= .3200E-02 PERCENT ERROR= .0									
FOR STORM = 5	STORM AREA (SQ MI) =			30.00					
65AT-3	MANE	.66	526.93	864.77	.30	5.00	526.90	865.00	.30
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2740E+03 EXCESS= .0000E+00 OUTFLOW= .2740E+03 BASIN STORAGE= .2309E-02 PERCENT ERROR= .0									
FOR STORM = 6	STORM AREA (SQ MI) =			60.00					
65AT-3	MANE	.66	522.15	865.02	.28	5.00	522.15	865.00	.28
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2580E+03 EXCESS= .0000E+00 OUTFLOW= .2580E+03 BASIN STORAGE= .1674E-02 PERCENT ERROR= .0									
FOR STORM = 7	STORM AREA (SQ MI) =			90.00					
65AT-3	MANE	.66	520.51	864.89	.28	5.00	520.49	865.00	.28
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2522E+03 EXCESS= .0000E+00 OUTFLOW= .2522E+03 BASIN STORAGE= .1465E-02 PERCENT ERROR= .0									
FOR STORM = 8	STORM AREA (SQ MI) =			120.00					
65AT-3	MANE	.66	518.36	864.97	.27	5.00	518.35	865.00	.27
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2458E+03 EXCESS= .0000E+00 OUTFLOW= .2458E+03 BASIN STORAGE= .1312E-02 PERCENT ERROR= .0									
FOR STORM = 9	STORM AREA (SQ MI) =			150.00					
65AT-3	MANE	.66	517.00	864.74	.26	5.00	516.98	865.00	.26
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2415E+03 EXCESS= .0000E+00 OUTFLOW= .2415E+03 BASIN STORAGE= .1222E-02 PERCENT ERROR= .0									
FOR STORM = 1	STORM AREA (SQ MI) =			.01					

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65T66	MANE	.61	547.56	864.97	.38	5.00	547.56	865.00	.38
CONTINUITY SUMMARY (AC-FT) - INFLOW= .3435E+03 EXCESS= .0000E+00 OUTFLOW= .3435E+03 BASIN STORAGE= .1930E-01 PERCENT ERROR= .0									
FOR STORM - 2 STORM AREA (SQ MI) =		1.00							
65T66	MANE	.61	546.92	865.16	.37	5.00	546.91	865.00	.37
CONTINUITY SUMMARY (AC-FT) - INFLOW= .3409E+03 EXCESS= .0000E+00 OUTFLOW= .3409E+03 BASIN STORAGE= .1880E-01 PERCENT ERROR= .0									
FOR STORM - 3 STORM AREA (SQ MI) =		5.00							
65T66	MANE	.61	543.87	864.90	.36	5.00	543.87	865.00	.36
CONTINUITY SUMMARY (AC-FT) - INFLOW= .3288E+03 EXCESS= .0000E+00 OUTFLOW= .3288E+03 BASIN STORAGE= .1629E-01 PERCENT ERROR= .0									
FOR STORM - 4 STORM AREA (SQ MI) =		10.00							
65T66	MANE	.61	539.92	864.94	.34	5.00	539.92	865.00	.34
CONTINUITY SUMMARY (AC-FT) - INFLOW= .3128E+03 EXCESS= .0000E+00 OUTFLOW= .3128E+03 BASIN STORAGE= .8965E-02 PERCENT ERROR= .0									
FOR STORM - 5 STORM AREA (SQ MI) =		30.00							
65T66	MANE	.61	534.69	864.79	.32	5.00	534.69	865.00	.32
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2892E+03 EXCESS= .0000E+00 OUTFLOW= .2892E+03 BASIN STORAGE= .2374E-02 PERCENT ERROR= .0									
FOR STORM - 6 STORM AREA (SQ MI) =		60.00							
65T66	MANE	.61	527.81	865.19	.29	5.00	527.79	865.00	.29
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2651E+03 EXCESS= .0000E+00 OUTFLOW= .2651E+03 BASIN STORAGE= .1573E-02 PERCENT ERROR= .0									
FOR STORM - 7 STORM AREA (SQ MI) =		90.00							
65T66	MANE	.61	524.12	865.18	.28	5.00	524.10	865.00	.28
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2565E+03 EXCESS= .0000E+00 OUTFLOW= .2565E+03 BASIN STORAGE= .1374E-02 PERCENT ERROR= .0									
FOR STORM - 8 STORM AREA (SQ MI) =		120.00							
65T66	MANE	.62	518.25	865.26	.27	5.00	518.23	865.00	.27
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2478E+03 EXCESS= .0000E+00 OUTFLOW= .2478E+03 BASIN STORAGE= .1230E-02 PERCENT ERROR= .0									
FOR STORM - 9 STORM AREA (SQ MI) =		150.00							
65T66	MANE	.62	516.88	865.11	.27	5.00	516.87	865.00	.27
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2427E+03 EXCESS= .0000E+00 OUTFLOW= .2426E+03 BASIN STORAGE= .1144E-02 PERCENT ERROR= .0									
FOR STORM - 1 STORM AREA (SQ MI) =		.01							
65T66A	MANE	.69	547.45	865.27	.38	5.00	547.43	865.00	.38
CONTINUITY SUMMARY (AC-FT) - INFLOW= .3435E+03 EXCESS= .0000E+00 OUTFLOW= .3435E+03 BASIN STORAGE= .2191E-01 PERCENT ERROR= .0									
FOR STORM - 2 STORM AREA (SQ MI) =		1.00							
65T66A	MANE	.69	546.80	865.46	.37	5.00	546.79	865.00	.37
CONTINUITY SUMMARY (AC-FT) - INFLOW= .3409E+03 EXCESS= .0000E+00 OUTFLOW= .3409E+03 BASIN STORAGE= .2134E-01 PERCENT ERROR= .0									
FOR STORM - 3 STORM AREA (SQ MI) =		5.00							
65T66A	MANE	.69	543.78	865.04	.36	5.00	543.77	865.00	.36
CONTINUITY SUMMARY (AC-FT) - INFLOW= .3288E+03 EXCESS= .0000E+00 OUTFLOW= .3288E+03 BASIN STORAGE= .1851E-01 PERCENT ERROR= .0									
FOR STORM - 4 STORM AREA (SQ MI) =		10.00							
65T66A	MANE	.69	539.80	864.92	.34	5.00	539.80	865.00	.34
CONTINUITY SUMMARY (AC-FT) - INFLOW= .3128E+03 EXCESS= .0000E+00 OUTFLOW= .3128E+03 BASIN STORAGE= .1021E-01 PERCENT ERROR= .0									
FOR STORM - 5 STORM AREA (SQ MI) =		30.00							
65T66A	MANE	.69	534.57	865.20	.32	5.00	534.55	865.00	.32
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2892E+03 EXCESS= .0000E+00 OUTFLOW= .2892E+03 BASIN STORAGE= .2703E-02 PERCENT ERROR= .0									
FOR STORM - 6 STORM AREA (SQ MI) =		60.00							
65T66A	MANE	.70	527.63	865.36	.29	5.00	527.60	865.00	.29

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CONTINUITY SUMMARY (AC-FT) - INFLOW= .2651E+03 EXCESS= .0000E+00 OUTFLOW= .2651E+03 BASIN STORAGE= .1791E-02 PERCENT ERROR= .0

FOR STORM = 7 STORM AREA (SQ MI) = 90.00  
 65T66A MARE .70 523.94 865.19 .28 5.00 523.91 865.00 .28

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2565E+03 EXCESS= .0000E+00 OUTFLOW= .2565E+03 BASIN STORAGE= .1565E-02 PERCENT ERROR= .0

FOR STORM = 8 STORM AREA (SQ MI) = 120.00  
 65T66A MARE .70 518.09 865.04 .27 5.00 518.08 865.00 .27

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2478E+03 EXCESS= .0000E+00 OUTFLOW= .2478E+03 BASIN STORAGE= .1401E-02 PERCENT ERROR= .0

FOR STORM = 9 STORM AREA (SQ MI) = 150.00  
 65T66A MARE .70 516.67 865.50 .27 5.00 516.62 865.00 .27

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2426E+03 EXCESS= .0000E+00 OUTFLOW= .2426E+03 BASIN STORAGE= .1302E-02 PERCENT ERROR= .0

FOR STORM = 1 STORM AREA (SQ MI) = .01  
 65T66B MARE .73 547.32 865.26 .38 5.00 547.28 865.00 .38

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3435E+03 EXCESS= .0000E+00 OUTFLOW= .3435E+03 BASIN STORAGE= .2312E-01 PERCENT ERROR= .0

FOR STORM = 2 STORM AREA (SQ MI) = 1.00  
 65T66B MARE .73 546.69 865.47 .37 5.00 546.64 865.00 .37

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3409E+03 EXCESS= .0000E+00 OUTFLOW= .3409E+03 BASIN STORAGE= .2253E-01 PERCENT ERROR= .0

FOR STORM = 3 STORM AREA (SQ MI) = 5.00  
 65T66B MARE .73 543.67 865.68 .36 5.00 543.63 865.00 .36

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3288E+03 EXCESS= .0000E+00 OUTFLOW= .3288E+03 BASIN STORAGE= .1955E-01 PERCENT ERROR= .0

FOR STORM = 4 STORM AREA (SQ MI) = 10.00  
 65T66B MARE .73 539.68 865.51 .34 5.00 539.62 865.00 .34

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3128E+03 EXCESS= .0000E+00 OUTFLOW= .3127E+03 BASIN STORAGE= .1083E-01 PERCENT ERROR= .0

FOR STORM = 5 STORM AREA (SQ MI) = 30.00  
 65T66B MARE .73 534.43 865.73 .32 5.00 534.41 865.00 .32

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2892E+03 EXCESS= .0000E+00 OUTFLOW= .2892E+03 BASIN STORAGE= .2862E-02 PERCENT ERROR= .0

FOR STORM = 6 STORM AREA (SQ MI) = 60.00  
 65T66B MARE .73 527.45 865.78 .29 5.00 527.39 865.00 .29

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2651E+03 EXCESS= .0000E+00 OUTFLOW= .2651E+03 BASIN STORAGE= .1894E-02 PERCENT ERROR= .0

FOR STORM = 7 STORM AREA (SQ MI) = 90.00  
 65T66B MARE .74 523.75 865.54 .28 5.00 523.66 865.00 .28

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2565E+03 EXCESS= .0000E+00 OUTFLOW= .2565E+03 BASIN STORAGE= .1657E-02 PERCENT ERROR= .0

FOR STORM = 8 STORM AREA (SQ MI) = 120.00  
 65T66B MARE .74 517.89 865.25 .27 5.00 517.83 865.00 .27

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2479E+03 EXCESS= .0000E+00 OUTFLOW= .2479E+03 BASIN STORAGE= .1482E-02 PERCENT ERROR= .0

FOR STORM = 9 STORM AREA (SQ MI) = 150.00  
 65T66B MARE .74 516.44 865.75 .27 5.00 516.26 865.00 .27

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2427E+03 EXCESS= .0000E+00 OUTFLOW= .2426E+03 BASIN STORAGE= .1379E-02 PERCENT ERROR= .0

FOR STORM = 1 STORM AREA (SQ MI) = .01  
 66CLT2 MARE .75 410.00 891.72 2.90 5.00 410.00 895.00 2.90

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5167E+03 EXCESS= .0000E+00 OUTFLOW= .5167E+03 BASIN STORAGE= .1175E-02 PERCENT ERROR= .0

FOR STORM = 2 STORM AREA (SQ MI) = 1.00  
 66CLT2 MARE .75 410.00 891.72 2.90 5.00 410.00 895.00 2.90

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5159E+03 EXCESS= .0000E+00 OUTFLOW= .5159E+03 BASIN STORAGE= .1175E-02 PERCENT ERROR= .0

FOR STORM = 3 STORM AREA (SQ MI) = 5.00  
66CIT2 MARE .75 409.93 905.92 2.88 5.00 409.93 905.00 2.88

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5122E+03 EXCESS= .0000E+00 OUTFLOW= .5122E+03 BASIN STORAGE= .1174E-02 PERCENT ERROR= .0

FOR STORM = 4 STORM AREA (SQ MI) = 10.00  
66CIT2 MARE .75 409.67 906.02 2.84 5.00 409.67 905.00 2.84

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5058E+03 EXCESS= .0000E+00 OUTFLOW= .5058E+03 BASIN STORAGE= .1173E-02 PERCENT ERROR= .0

FOR STORM = 5 STORM AREA (SQ MI) = 30.00  
66CIT2 MARE .75 409.24 915.93 2.78 5.00 409.24 915.00 2.78

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4959E+03 EXCESS= .0000E+00 OUTFLOW= .4959E+03 BASIN STORAGE= .1174E-02 PERCENT ERROR= .0

FOR STORM = 6 STORM AREA (SQ MI) = 60.00  
66CIT2 MARE .75 405.86 960.82 2.74 5.00 405.86 960.00 2.74

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4889E+03 EXCESS= .0000E+00 OUTFLOW= .4889E+03 BASIN STORAGE= .1175E-02 PERCENT ERROR= .0

FOR STORM = 7 STORM AREA (SQ MI) = 90.00  
66CIT2 MARE .75 405.37 930.39 2.73 5.00 405.37 930.00 2.73

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4866E+03 EXCESS= .0000E+00 OUTFLOW= .4866E+03 BASIN STORAGE= .1174E-02 PERCENT ERROR= .0

FOR STORM = 8 STORM AREA (SQ MI) = 120.00  
66CIT2 MARE .75 405.32 930.41 2.72 5.00 405.32 930.00 2.72

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4849E+03 EXCESS= .0000E+00 OUTFLOW= .4849E+03 BASIN STORAGE= .1174E-02 PERCENT ERROR= .0

FOR STORM = 9 STORM AREA (SQ MI) = 150.00  
66CIT2 MARE .75 405.29 930.41 2.72 5.00 405.29 930.00 2.72

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4838E+03 EXCESS= .0000E+00 OUTFLOW= .4838E+03 BASIN STORAGE= .1174E-02 PERCENT ERROR= .0

FOR STORM = 1 STORM AREA (SQ MI) = .01  
66CTD MARE .32 956.20 865.37 .79 5.00 956.17 865.00 .79

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8602E+03 EXCESS= .0000E+00 OUTFLOW= .8602E+03 BASIN STORAGE= .1150E-01 PERCENT ERROR= .0

FOR STORM = 2 STORM AREA (SQ MI) = 1.00  
66CTD MARE .32 955.49 865.18 .78 5.00 955.46 865.00 .78

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8568E+03 EXCESS= .0000E+00 OUTFLOW= .8568E+03 BASIN STORAGE= .1121E-01 PERCENT ERROR= .0

FOR STORM = 3 STORM AREA (SQ MI) = 5.00  
66CTD MARE .32 952.15 865.44 .77 5.00 952.11 865.00 .77

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8410E+03 EXCESS= .0000E+00 OUTFLOW= .8410E+03 BASIN STORAGE= .9771E-02 PERCENT ERROR= .0

FOR STORM = 4 STORM AREA (SQ MI) = 10.00  
66CTD MARE .32 947.73 865.33 .75 5.00 947.71 865.00 .75

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8185E+03 EXCESS= .0000E+00 OUTFLOW= .8185E+03 BASIN STORAGE= .5567E-02 PERCENT ERROR= .0

FOR STORM = 5 STORM AREA (SQ MI) = 30.00  
66CTD MARE .32 939.08 865.64 .72 5.00 939.02 865.00 .72

CONTINUITY SUMMARY (AC-FT) - INFLOW= .7852E+03 EXCESS= .0000E+00 OUTFLOW= .7852E+03 BASIN STORAGE= .1770E-02 PERCENT ERROR= .0

FOR STORM = 6 STORM AREA (SQ MI) = 60.00  
66CTD MARE .32 928.75 865.59 .69 5.00 928.67 865.00 .69

CONTINUITY SUMMARY (AC-FT) - INFLOW= .7540E+03 EXCESS= .0000E+00 OUTFLOW= .7540E+03 BASIN STORAGE= .1332E-02 PERCENT ERROR= .0

FOR STORM = 7 STORM AREA (SQ MI) = 90.00  
66CTD MARE .32 924.93 865.40 .68 5.00 924.86 865.00 .68

CONTINUITY SUMMARY (AC-FI) - INFLOW- .7430E+03 EXCESS- .0000E+00 OUTFLOW- .7430E+03 BASIN STORAGE= .1226E-02 PERCENT ERROR= .0

FOR STORM - 8 STORM AREA (SQ MI) - 120.00  
 66CTD MANE .32 918.36 865.99 .67 5.00 918.22 865.00 .67

CONTINUITY SUMMARY (AC-FI) - INFLOW- .7327E+03 EXCESS- .0000E+00 OUTFLOW- .7327E+03 BASIN STORAGE= .1149E-02 PERCENT ERROR= .0

FOR STORM - 9 STORM AREA (SQ MI) - 150.00  
 66CTD MANE .32 915.74 865.80 .67 5.00 915.51 865.00 .67

CONTINUITY SUMMARY (AC-FI) - INFLOW- .7264E+03 EXCESS- .0000E+00 OUTFLOW- .7264E+03 BASIN STORAGE= .1104E-02 PERCENT ERROR= .0

FOR STORM - 1 STORM AREA (SQ MI) - .01  
 RC-WA MANE .69 973.81 866.25 .72 5.00 973.69 865.00 .72

CONTINUITY SUMMARY (AC-FI) - INFLOW- .9162E+03 EXCESS- .0000E+00 OUTFLOW- .9162E+03 BASIN STORAGE= .5332E-01 PERCENT ERROR= .0

FOR STORM - 2 STORM AREA (SQ MI) - 1.00  
 RC-WA MANE .69 973.09 865.71 .72 5.00 972.97 865.00 .72

CONTINUITY SUMMARY (RC-FI) - INFLOW- .9126E+03 EXCESS- .0000E+00 OUTFLOW- .9126E+03 BASIN STORAGE= .5280E-01 PERCENT ERROR= .0

FOR STORM - 3 STORM AREA (SQ MI) - 5.00  
 RC-WA MANE .70 969.48 866.34 .71 5.00 969.30 865.00 .71

CONTINUITY SUMMARY (AC-FI) - INFLOW- .8963E+03 EXCESS- .0000E+00 OUTFLOW- .8962E+03 BASIN STORAGE= .5016E-01 PERCENT ERROR= .0

FOR STORM - 4 STORM AREA (SQ MI) - 10.00  
 RC-WA MANE .70 963.76 866.66 .69 5.00 963.54 870.00 .69

CONTINUITY SUMMARY (AC-FI) - INFLOW- .8731E+03 EXCESS- .0000E+00 OUTFLOW- .8730E+03 BASIN STORAGE= .4299E-01 PERCENT ERROR= .0

FOR STORM - 5 STORM AREA (SQ MI) - 30.00  
 RC-WA MANE .70 953.93 869.84 .66 5.00 953.91 870.00 .66

CONTINUITY SUMMARY (AC-FI) - INFLOW- .8393E+03 EXCESS- .0000E+00 OUTFLOW- .8393E+03 BASIN STORAGE= .3736E-01 PERCENT ERROR= .0

FOR STORM - 6 STORM AREA (SQ MI) - 60.00  
 RC-WA MANE .70 942.31 869.89 .64 5.00 942.30 870.00 .64

CONTINUITY SUMMARY (AC-FI) - INFLOW- .8078E+03 EXCESS- .0000E+00 OUTFLOW- .8077E+03 BASIN STORAGE= .3674E-01 PERCENT ERROR= .0

FOR STORM - 7 STORM AREA (SQ MI) - 90.00  
 RC-WA MANE .70 938.31 869.94 .63 5.00 938.30 870.00 .63

CONTINUITY SUMMARY (AC-FI) - INFLOW- .7967E+03 EXCESS- .0000E+00 OUTFLOW- .7967E+03 BASIN STORAGE= .3658E-01 PERCENT ERROR= .0

FOR STORM - 8 STORM AREA (SQ MI) - 120.00  
 RC-WA MANE .70 931.44 869.81 .62 5.00 931.43 870.00 .62

CONTINUITY SUMMARY (AC-FI) - INFLOW- .7862E+03 EXCESS- .0000E+00 OUTFLOW- .7862E+03 BASIN STORAGE= .3642E-01 PERCENT ERROR= .0

FOR STORM - 9 STORM AREA (SQ MI) - 150.00  
 RC-WA MANE .70 928.44 869.67 .61 5.00 928.42 870.00 .61

CONTINUITY SUMMARY (AC-FI) - INFLOW- .7798E+03 EXCESS- .0000E+00 OUTFLOW- .7798E+03 BASIN STORAGE= .3625E-01 PERCENT ERROR= .0

FOR STORM - 1 STORM AREA (SQ MI) - .01  
 66T66D MANE 1.00 973.60 866.70 .77 5.00 973.39 870.00 .77

CONTINUITY SUMMARY (AC-FI) - INFLOW- .9828E+03 EXCESS- .0000E+00 OUTFLOW- .9827E+03 BASIN STORAGE= .1183E+00 PERCENT ERROR= .0

FOR STORM - 2 STORM AREA (SQ MI) - 1.00  
 66T66D MANE 1.00 972.87 866.82 .77 5.00 972.65 870.00 .77

CONTINUITY SUMMARY (AC-FI) - INFLOW- .9778E+03 EXCESS- .0000E+00 OUTFLOW- .9777E+03 BASIN STORAGE= .1175E+00 PERCENT ERROR= .0

FOR STORM - 3 STORM AREA (SQ MI) - 5.00  
 66T66D MANE 1.01 969.21 866.48 .75 5.00 969.04 870.00 .75

CONTINUITY SUMMARY (AC-FI) - INFLOW- .9577E+03 EXCESS- .0000E+00 OUTFLOW- .9576E+03 BASIN STORAGE= .1118E+00 PERCENT ERROR= .0



EMDU89.txt

FOR STORM = 4	STORM AREA (SQ MI) =	10.00							
66T66D	MANE	1.01	963.53	869.51	.73	5.00	963.49	870.00	.73
CONTINUITY SUMMARY (AC-FT) - INFLOW= .9316E+03 EXCESS= .0000E+00 OUTFLOW= .9316E+03 BASIN STORAGE= .9988E-01 PERCENT ERROR= .0									
FOR STORM = 5	STORM AREA (SQ MI) =	30.00							
66T66D	MANE	1.01	953.76	870.25	.70	5.00	953.73	870.00	.70
CONTINUITY SUMMARY (AC-FT) - INFLOW= .8946E+03 EXCESS= .0000E+00 OUTFLOW= .8946E+03 BASIN STORAGE= .8857E-01 PERCENT ERROR= .0									
FOR STORM = 6	STORM AREA (SQ MI) =	60.00							
66T66D	MANE	1.01	942.14	870.36	.67	5.00	942.11	870.00	.67
CONTINUITY SUMMARY (AC-FT) - INFLOW= .8573E+03 EXCESS= .0000E+00 OUTFLOW= .8573E+03 BASIN STORAGE= .7385E-01 PERCENT ERROR= .0									
FOR STORM = 7	STORM AREA (SQ MI) =	90.00							
66T66D	MANE	1.01	938.15	870.09	.66	5.00	938.13	870.00	.66
CONTINUITY SUMMARY (AC-FT) - INFLOW= .8435E+03 EXCESS= .0000E+00 OUTFLOW= .8435E+03 BASIN STORAGE= .6248E-01 PERCENT ERROR= .0									
FOR STORM = 8	STORM AREA (SQ MI) =	120.00							
66T66D	MANE	1.01	931.21	870.37	.65	5.00	931.15	870.00	.65
CONTINUITY SUMMARY (AC-FT) - INFLOW= .8294E+03 EXCESS= .0000E+00 OUTFLOW= .8294E+03 BASIN STORAGE= .5638E-01 PERCENT ERROR= .0									
FOR STORM = 9	STORM AREA (SQ MI) =	150.00							
66T66D	MANE	1.01	928.18	870.93	.65	5.00	928.16	870.00	.65
CONTINUITY SUMMARY (AC-FT) - INFLOW= .8208E+03 EXCESS= .0000E+00 OUTFLOW= .8208E+03 BASIN STORAGE= .5451E-01 PERCENT ERROR= .0									
FOR STORM = 1	STORM AREA (SQ MI) =	.01							
66-66D	MANE	2.16	973.38	869.99	.77	5.00	973.38	870.00	.77
CONTINUITY SUMMARY (AC-FT) - INFLOW= .9827E+03 EXCESS= .0000E+00 OUTFLOW= .9824E+03 BASIN STORAGE= .2556E+00 PERCENT ERROR= .0									
FOR STORM = 2	STORM AREA (SQ MI) =	1.00							
66-66D	MANE	2.16	972.63	870.13	.77	5.00	972.62	870.00	.77
CONTINUITY SUMMARY (AC-FT) - INFLOW= .9778E+03 EXCESS= .0000E+00 OUTFLOW= .9775E+03 BASIN STORAGE= .2538E+00 PERCENT ERROR= .0									
FOR STORM = 3	STORM AREA (SQ MI) =	5.00							
66-66D	MANE	2.17	968.86	870.77	.75	5.00	968.85	870.00	.75
CONTINUITY SUMMARY (AC-FT) - INFLOW= .9576E+03 EXCESS= .0000E+00 OUTFLOW= .9574E+03 BASIN STORAGE= .2416E+00 PERCENT ERROR= .0									
FOR STORM = 4	STORM AREA (SQ MI) =	10.00							
66-66D	MANE	2.17	963.26	869.61	.73	5.00	963.24	870.00	.73
CONTINUITY SUMMARY (AC-FT) - INFLOW= .9316E+03 EXCESS= .0000E+00 OUTFLOW= .9314E+03 BASIN STORAGE= .2158E+00 PERCENT ERROR= .0									
FOR STORM = 5	STORM AREA (SQ MI) =	30.00							
66-66D	MANE	2.17	953.30	871.37	.70	5.00	953.12	870.00	.70
CONTINUITY SUMMARY (AC-FT) - INFLOW= .8946E+03 EXCESS= .0000E+00 OUTFLOW= .8944E+03 BASIN STORAGE= .1914E+00 PERCENT ERROR= .0									
FOR STORM = 6	STORM AREA (SQ MI) =	60.00							
66-66D	MANE	2.18	941.66	871.34	.67	5.00	941.45	870.00	.67
CONTINUITY SUMMARY (AC-FT) - INFLOW= .8573E+03 EXCESS= .0000E+00 OUTFLOW= .8571E+03 BASIN STORAGE= .1604E+00 PERCENT ERROR= .0									
FOR STORM = 7	STORM AREA (SQ MI) =	90.00							
66-66D	MANE	2.18	937.72	872.08	.66	5.00	937.58	870.00	.66
CONTINUITY SUMMARY (AC-FT) - INFLOW= .8434E+03 EXCESS= .0000E+00 OUTFLOW= .8433E+03 BASIN STORAGE= .1359E+00 PERCENT ERROR= .0									
FOR STORM = 8	STORM AREA (SQ MI) =	120.00							
66-66D	MANE	2.18	930.59	871.20	.65	5.00	930.25	870.00	.65
CONTINUITY SUMMARY (AC-FT) - INFLOW= .8294E+03 EXCESS= .0000E+00 OUTFLOW= .8292E+03 BASIN STORAGE= .1222E+00 PERCENT ERROR= .0									

EMDU89.txt

FOR STORM = 9	STORM AREA (SQ MI) =	150.00							
66-66D	MANE	2.18	927.56	871.75	.65	5.00	927.12	870.00	.65
CONTINUITY SUMMARY (AC-FT) - INFLOW= .8208E+03 EXCESS= .0000E+00 OUTFLOW= .8207E+03 BASIN STORAGE= .1180E+00 PERCENT ERROR= .0									
FOR STORM = 1	STORM AREA (SQ MI) =	.01							
66T23A	MANE	.32	1184.49	835.24	.76	5.00	1184.42	835.00	.76
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1073E+04 EXCESS= .0000E+00 OUTFLOW= .1073E+04 BASIN STORAGE= .3994E-01 PERCENT ERROR= .0									
FOR STORM = 2	STORM AREA (SQ MI) =	1.00							
66T23A	MANE	.33	1179.02	839.95	.76	5.00	1178.97	840.00	.76
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1067E+04 EXCESS= .0000E+00 OUTFLOW= .1067E+04 BASIN STORAGE= .3967E-01 PERCENT ERROR= .0									
FOR STORM = 3	STORM AREA (SQ MI) =	5.00							
66T23A	MANE	.33	1149.35	844.86	.74	5.00	1149.31	845.00	.74
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1038E+04 EXCESS= .0000E+00 OUTFLOW= .1038E+04 BASIN STORAGE= .3776E-01 PERCENT ERROR= .0									
FOR STORM = 4	STORM AREA (SQ MI) =	10.00							
66T23A	MANE	.33	1115.98	850.18	.71	5.00	1115.92	850.00	.71
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1001E+04 EXCESS= .0000E+00 OUTFLOW= .1001E+04 BASIN STORAGE= .3374E-01 PERCENT ERROR= .0									
FOR STORM = 5	STORM AREA (SQ MI) =	30.00							
66T23A	MANE	.33	1074.42	865.24	.68	5.00	1074.35	865.00	.68
CONTINUITY SUMMARY (AC-FT) - INFLOW= .9512E+03 EXCESS= .0000E+00 OUTFLOW= .9512E+03 BASIN STORAGE= .2992E-01 PERCENT ERROR= .0									
FOR STORM = 6	STORM AREA (SQ MI) =	60.00							
66T23A	MANE	.34	979.88	869.91	.64	5.00	979.87	870.00	.64
CONTINUITY SUMMARY (AC-FT) - INFLOW= .9009E+03 EXCESS= .0000E+00 OUTFLOW= .9008E+03 BASIN STORAGE= .2512E-01 PERCENT ERROR= .0									
FOR STORM = 7	STORM AREA (SQ MI) =	90.00							
66T23A	MANE	.34	975.13	870.07	.63	5.00	975.12	870.00	.63
CONTINUITY SUMMARY (AC-FT) - INFLOW= .8822E+03 EXCESS= .0000E+00 OUTFLOW= .8822E+03 BASIN STORAGE= .2129E-01 PERCENT ERROR= .0									
FOR STORM = 8	STORM AREA (SQ MI) =	120.00							
66T23A	MANE	.34	966.61	870.17	.62	5.00	966.46	870.00	.62
CONTINUITY SUMMARY (AC-FT) - INFLOW= .8635E+03 EXCESS= .0000E+00 OUTFLOW= .8634E+03 BASIN STORAGE= .1912E-01 PERCENT ERROR= .0									
FOR STORM = 9	STORM AREA (SQ MI) =	150.00							
66T23A	MANE	.34	942.44	870.17	.61	5.00	942.35	870.00	.61
CONTINUITY SUMMARY (AC-FT) - INFLOW= .8520E+03 EXCESS= .0000E+00 OUTFLOW= .8520E+03 BASIN STORAGE= .1846E-01 PERCENT ERROR= .0									
FOR STORM = 1	STORM AREA (SQ MI) =	.01							
66T23B	MANE	1.48	1183.52	838.86	.76	5.00	1183.43	840.00	.76
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1073E+04 EXCESS= .0000E+00 OUTFLOW= .1073E+04 BASIN STORAGE= .1817E+00 PERCENT ERROR= .0									
FOR STORM = 2	STORM AREA (SQ MI) =	1.00							
66T23B	MANE	1.48	1178.14	839.64	.76	5.00	1178.05	840.00	.76
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1067E+04 EXCESS= .0000E+00 OUTFLOW= .1066E+04 BASIN STORAGE= .1804E+00 PERCENT ERROR= .0									
FOR STORM = 3	STORM AREA (SQ MI) =	5.00							
66T23B	MANE	1.49	1148.32	845.41	.74	5.00	1148.20	845.00	.74
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1038E+04 EXCESS= .0000E+00 OUTFLOW= .1038E+04 BASIN STORAGE= .1718E+00 PERCENT ERROR= .0									
FOR STORM = 4	STORM AREA (SQ MI) =	10.00							
66T23B	MANE	1.49	1114.96	854.90	.71	5.00	1114.92	855.00	.71
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1001E+04 EXCESS= .0000E+00 OUTFLOW= .1001E+04 BASIN STORAGE= .1537E+00 PERCENT ERROR= .0									

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FOR STORM - 5	STORM AREA (SQ MI) =	30.00							
66T23B	MANE	1.51	1073.66	868.94	.68	5.00	1073.53	870.00	.68
CONTINUITY SUMMARY (AC-FT) - INFLOW= .9512E+03 EXCESS= .0000E+00 OUTFLOW= .9510E+03 BASIN STORAGE= .1362E+00 PERCENT ERROR= .0									
FOR STORM - 6	STORM AREA (SQ MI) =	60.00							
66T23B	MANE	1.53	979.22	871.28	.64	5.00	979.17	870.00	.64
CONTINUITY SUMMARY (AC-FT) - INFLOW= .9008E+03 EXCESS= .0000E+00 OUTFLOW= .9007E+03 BASIN STORAGE= .1145E+00 PERCENT ERROR= .0									
FOR STORM - 7	STORM AREA (SQ MI) =	90.00							
66T23B	MANE	1.54	974.42	870.58	.63	5.00	974.28	870.00	.63
CONTINUITY SUMMARY (AC-FT) - INFLOW= .8822E+03 EXCESS= .0000E+00 OUTFLOW= .8821E+03 BASIN STORAGE= .9712E-01 PERCENT ERROR= .0									
FOR STORM - 8	STORM AREA (SQ MI) =	120.00							
66T23B	MANE	1.54	965.08	873.68	.62	5.00	965.07	875.00	.62
CONTINUITY SUMMARY (AC-FT) - INFLOW= .8635E+03 EXCESS= .0000E+00 OUTFLOW= .8633E+03 BASIN STORAGE= .8705E-01 PERCENT ERROR= .0									
FOR STORM - 9	STORM AREA (SQ MI) =	150.00							
66T23B	MANE	1.55	941.50	875.02	.61	5.00	941.50	875.00	.61
CONTINUITY SUMMARY (AC-FT) - INFLOW= .8520E+03 EXCESS= .0000E+00 OUTFLOW= .8519E+03 BASIN STORAGE= .8405E-01 PERCENT ERROR= .0									
FOR STORM - 1	STORM AREA (SQ MI) =	.01							
CULVT	MANE	.31	1182.75	839.96	.76	5.00	1182.74	840.00	.76
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1073E+04 EXCESS= .0000E+00 OUTFLOW= .1073E+04 BASIN STORAGE= .3863E-01 PERCENT ERROR= .0									
FOR STORM - 2	STORM AREA (SQ MI) =	1.00							
CULVT	MANE	.31	1177.21	840.14	.76	5.00	1177.19	840.00	.76
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1066E+04 EXCESS= .0000E+00 OUTFLOW= .1066E+04 BASIN STORAGE= .3836E-01 PERCENT ERROR= .0									
FOR STORM - 3	STORM AREA (SQ MI) =	5.00							
CULVT	MANE	.32	1147.35	845.09	.74	5.00	1147.31	845.00	.74
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1038E+04 EXCESS= .0000E+00 OUTFLOW= .1038E+04 BASIN STORAGE= .3654E-01 PERCENT ERROR= .0									
FOR STORM - 4	STORM AREA (SQ MI) =	10.00							
CULVT	MANE	.32	1114.31	854.82	.71	5.00	1114.30	855.00	.71
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1001E+04 EXCESS= .0000E+00 OUTFLOW= .1001E+04 BASIN STORAGE= .3269E-01 PERCENT ERROR= .0									
FOR STORM - 5	STORM AREA (SQ MI) =	30.00							
CULVT	MANE	.32	1073.02	869.95	.68	5.00	1073.01	870.00	.68
CONTINUITY SUMMARY (AC-FT) - INFLOW= .9511E+03 EXCESS= .0000E+00 OUTFLOW= .9511E+03 BASIN STORAGE= .2896E-01 PERCENT ERROR= .0									
FOR STORM - 6	STORM AREA (SQ MI) =	60.00							
CULVT	MANE	.33	978.76	870.16	.64	5.00	978.73	870.00	.64
CONTINUITY SUMMARY (AC-FT) - INFLOW= .9007E+03 EXCESS= .0000E+00 OUTFLOW= .9006E+03 BASIN STORAGE= .2436E-01 PERCENT ERROR= .0									
FOR STORM - 7	STORM AREA (SQ MI) =	90.00							
CULVT	MANE	.33	973.84	870.05	.63	5.00	973.82	870.00	.63
CONTINUITY SUMMARY (AC-FT) - INFLOW= .8821E+03 EXCESS= .0000E+00 OUTFLOW= .8821E+03 BASIN STORAGE= .2068E-01 PERCENT ERROR= .0									
FOR STORM - 8	STORM AREA (SQ MI) =	120.00							
CULVT	MANE	.33	964.53	874.94	.62	5.00	964.51	875.00	.62
CONTINUITY SUMMARY (AC-FT) - INFLOW= .8634E+03 EXCESS= .0000E+00 OUTFLOW= .8633E+03 BASIN STORAGE= .1852E-01 PERCENT ERROR= .0									
FOR STORM - 9	STORM AREA (SQ MI) =	150.00							
CULVT	MANE	.33	941.07	875.01	.61	5.00	941.06	875.00	.61
CONTINUITY SUMMARY (AC-FT) - INFLOW= .8518E+03 EXCESS= .0000E+00 OUTFLOW= .8517E+03 BASIN STORAGE= .1787E-01 PERCENT ERROR= .0									

END089.txt

\*\*\* NORMAL END OF HEC-1 \*\*\*

-----DSS---2CLOSE Unit: 71, File: NS2-NEM.DSS  
Pointer Utilization: .30  
Number of Records: 70  
File Size: 131.5 Kbytes  
Percent Inactive: .0

## **NOAA Atlas Precipitation Data**



### POINT PRECIPITATION FREQUENCY ESTIMATES FROM NOAA ATLAS 14



Arizona 33.3325 N 111.62 W 1420 feet  
 from "Precipitation-Frequency Atlas of the United States" NOAA Atlas 14, Volume 1, Version 4  
 G.M. Bonnin, D. Martin, B. Liu, T. Parzybok, M. Yekta, and D. Riley  
 NOAA, National Weather Service, Silver Spring, Maryland, 2006

Extracted: Mon Oct 22 2007

Confidence Limits | Seasonality | Location Maps | Other info. | GIS data | Maps | Help | D

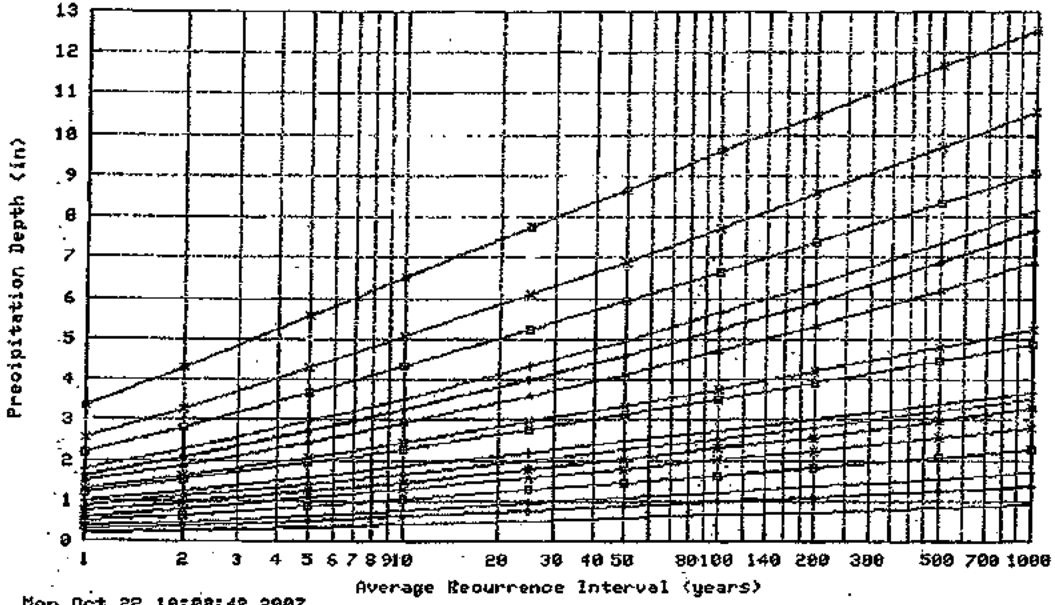
Precipitation Frequency Estimates (inches)																		
ARI* (years)	5 min	10 min	15 min	30 min	60 min	120 min	3 hr	6 hr	12 hr	24 hr	48 hr	4 day	7 day	10 day	20 day	30 day	45 day	60 day
1	0.19	0.29	0.36	0.48	0.60	0.68	0.73	0.88	0.99	1.21	1.27	1.47	1.62	1.77	2.19	2.56	2.99	3.33
2	0.25	0.38	0.47	0.63	0.78	0.89	0.93	1.11	1.26	1.52	1.61	1.87	2.07	2.25	2.81	3.28	3.84	4.28
5	0.34	0.51	0.64	0.85	1.06	1.18	1.22	1.42	1.58	1.95	2.08	2.45	2.71	2.96	3.68	4.29	5.02	5.58
10	0.40	0.61	0.76	1.03	1.27	1.40	1.45	1.66	1.84	2.29	2.44	2.92	3.24	3.52	4.35	5.06	5.90	6.53
25	0.50	0.76	0.94	1.26	1.56	1.71	1.77	2.00	2.19	2.75	2.95	3.59	3.98	4.32	5.25	6.10	7.05	7.76
50	0.57	0.86	1.07	1.44	1.78	1.95	2.03	2.26	2.46	3.12	3.35	4.13	4.59	4.96	5.94	6.91	7.93	8.68
100	0.64	0.97	1.21	1.63	2.01	2.19	2.30	2.54	2.74	3.51	3.77	4.70	5.24	5.64	6.65	7.73	8.81	9.60
200	0.71	1.09	1.35	1.81	2.24	2.44	2.58	2.82	3.02	3.90	4.20	5.31	5.92	6.36	7.37	8.57	9.69	10.49
500	0.81	1.24	1.53	2.06	2.56	2.78	2.97	3.21	3.40	4.45	4.79	6.17	6.89	7.36	8.35	9.70	10.84	11.66
1000	0.89	1.35	1.68	2.26	2.79	3.04	3.28	3.52	3.69	4.88	5.25	6.87	7.68	8.17	9.10	10.57	11.71	12.53

Text version of table

\* These precipitation frequency estimates are based on a partial duration series. ARI is the Average Recurrence Interval. Please refer to the documentation for more information. NOTE: Formatting forces estimates near zero to appear as zero.

MESA PROUING GROUNDS ONSITE PRECIPITATION DEPTHS

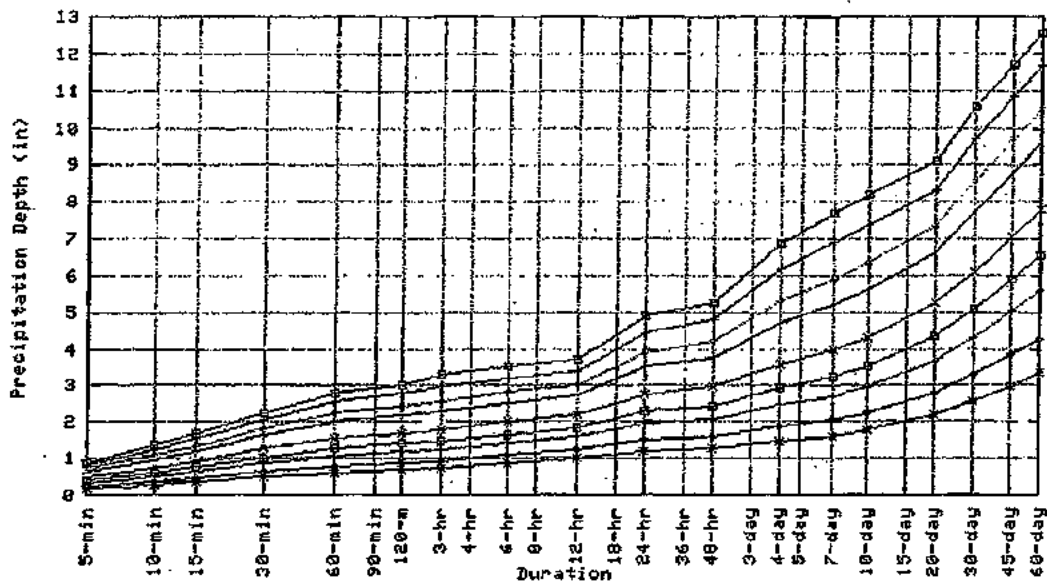
Partial duration based Point Precipitation Frequency Estimates Version: 4  
 33.3325 N 111.62 H 1420 ft



Mon Oct 22 10:08:42 2007

Duration			
5-min	→	48-hr	✕
10-min	→	4-day	→
15-min	→	7-day	→
30-min	→	10-day	→
60-min	→	20-day	→
3-hr	✕	30-day	✕
6-hr	→	60-day	✕
12-hr	→		
24-hr	→		

Partial duration based Point Precipitation Frequency Estimates Version: 4  
33.3325 N 111.62 W 1428 ft



Mon Oct 22 10:08:42 2007

Average Recurrence Interval (years)	
1	↑↑
2	↑↑
5	↑↑
10	↑↑
25	↑↑
50	↑↑
100	↑↑
200	↑↑
500	↑↑
1000	↑↑

Confidence Limits -

* Upper bound of the 90% confidence interval Precipitation Frequency Estimates (inches)																		
ARI** (years)	5 min	10 min	15 min	30 min	60 min	120 min	3 hr	6 hr	12 hr	24 hr	48 hr	4 day	7 day	10 day	20 day	30 day	45 day	60 day
1	0.23	0.35	0.44	0.59	0.73	0.82	0.89	1.03	1.14	1.36	1.42	1.62	1.79	1.95	2.42	2.81	3.29	3.65
2	0.30	0.46	0.57	0.77	0.96	1.06	1.14	1.31	1.44	1.72	1.81	2.07	2.28	2.48	3.10	3.60	4.22	4.70
5	0.41	0.62	0.77	1.04	1.29	1.41	1.49	1.67	1.81	2.19	2.33	2.70	2.99	3.25	4.05	4.70	5.51	6.12
10	0.49	0.75	0.93	1.25	1.54	1.68	1.76	1.95	2.10	2.57	2.74	3.22	3.57	3.86	4.79	5.54	6.48	7.16
25	0.60	0.91	1.13	1.52	1.88	2.04	2.14	2.33	2.49	3.09	3.30	3.94	4.38	4.73	5.77	6.68	7.75	8.52
50	0.68	1.04	1.29	1.74	2.15	2.32	2.44	2.63	2.79	3.50	3.75	4.54	5.05	5.43	6.54	7.56	8.71	9.53
100	0.77	1.17	1.45	1.95	2.42	2.61	2.76	2.95	3.11	3.93	4.22	5.18	5.76	6.18	7.33	8.48	9.69	10.55
200	0.86	1.30	1.61	2.17	2.69	2.90	3.09	3.28	3.43	4.38	4.71	5.86	6.53	6.97	8.13	9.41	10.67	11.55
500	0.98	1.49	1.84	2.48	3.07	3.30	3.56	3.74	3.88	5.01	5.40	6.83	7.62	8.11	9.24	10.69	11.98	12.88
1000	1.07	1.63	2.02	2.72	3.36	3.63	3.94	4.11	4.24	5.53	5.95	7.63	8.53	9.02	10.11	11.71	12.99	13.88

\* The upper bound of the confidence interval at 90% confidence level is the value which 5% of the simulated quantile values for a given frequency are greater than.  
\*\* These precipitation frequency estimates are based on a partial duration series. ARI is the Average Recurrence Interval.  
Please refer to the documentation for more information. NOTE: Formatting prevents estimates near zero to appear as zero.

* Lower bound of the 90% confidence interval																
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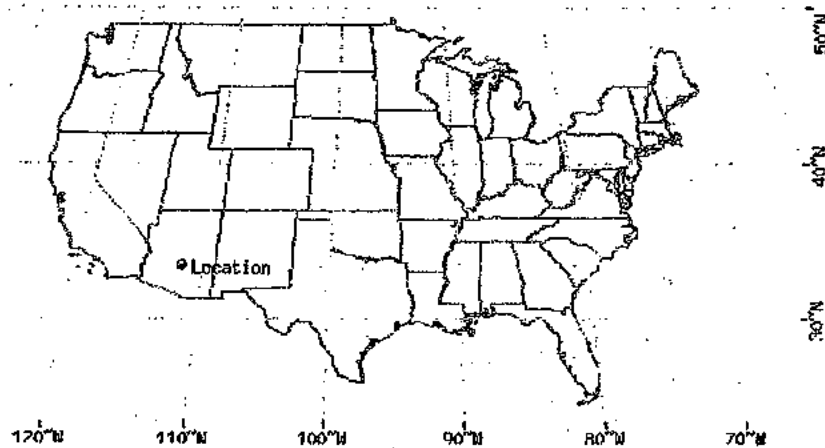
Precipitation Frequency Estimates (inches)																		
ARI** (years)	5 min	10 min	15 min	30 min	60 min	120 min	3 hr	6 hr	12 hr	24 hr	48 hr	4 day	7 day	10 day	20 day	30 day	45 day	60 day
1	0.16	0.24	0.30	0.41	0.50	0.58	0.61	0.76	0.87	1.08	1.14	1.34	1.48	1.61	1.99	2.33	2.72	3.04
2	0.21	0.32	0.40	0.53	0.66	0.75	0.79	0.96	1.10	1.37	1.44	1.70	1.88	2.06	2.55	2.98	3.49	3.90
5	0.28	0.43	0.53	0.71	0.88	0.99	1.03	1.22	1.39	1.74	1.86	2.22	2.46	2.69	3.34	3.90	4.56	5.88
10	0.34	0.51	0.63	0.85	1.05	1.17	1.21	1.42	1.60	2.04	2.18	2.64	2.93	3.19	3.93	4.60	5.35	5.93
25	0.41	0.62	0.76	1.03	1.27	1.41	1.46	1.69	1.88	2.43	2.61	3.23	3.58	3.90	4.73	5.52	6.38	7.03
50	0.46	0.70	0.86	1.16	1.44	1.58	1.64	1.88	2.09	2.73	2.94	3.69	4.10	4.44	5.33	6.22	7.14	7.83
100	0.51	0.77	0.96	1.29	1.59	1.75	1.82	2.07	2.29	3.04	3.27	4.17	4.64	5.01	5.94	6.93	7.90	8.62
200	0.56	0.84	1.05	1.41	1.75	1.91	2.01	2.26	2.49	3.34	3.60	4.66	5.20	5.60	6.54	7.63	8.63	9.38
500	0.62	0.94	1.16	1.57	1.94	2.12	2.25	2.51	2.73	3.74	4.03	5.34	5.96	6.40	7.33	8.55	9.58	10.35
1000	0.66	1.00	1.25	1.68	2.08	2.27	2.42	2.68	2.92	4.04	4.35	5.88	6.57	7.03	7.92	9.23	10.28	11.05

\* The lower bound of the confidence interval at 90% confidence level is the value which 5% of the simulated quantile values for a given frequency are less than.

\*\* These precipitation frequency estimates are based on a partial duration maxima series. ARI is the Average Recurrence Interval.

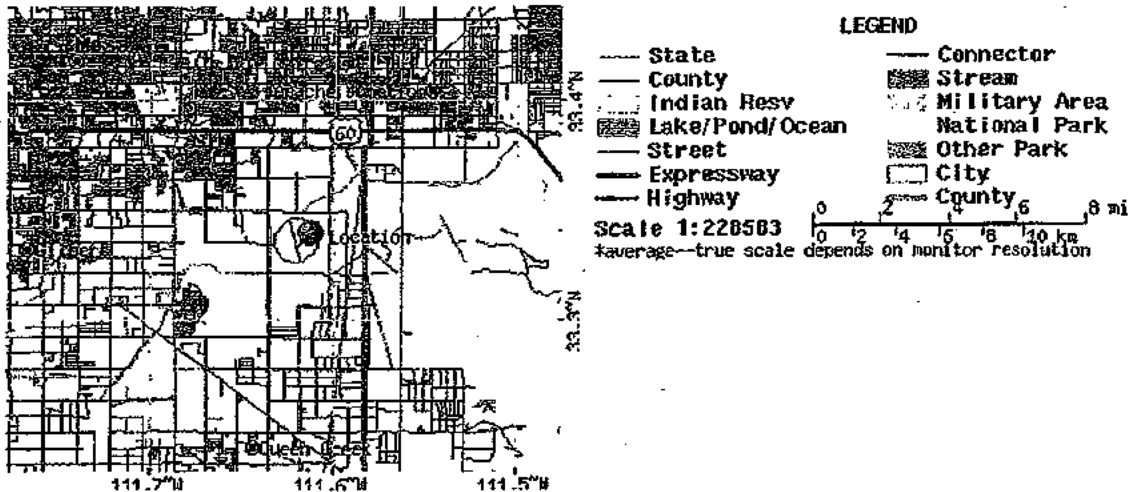
Please refer to the [documentation](#) for more information. NOTE: Formatting prevents estimates near zero to appear as zero.

Maps -



These maps were produced using a direct map request from the U.S. Census Bureau Mapping and Cartographic Resources Tiger Map Server.

Please read [disclaimer](#) for more information.



**Other Maps/Photographs -**

View [USGS digital orthophoto quadrangle \(DOQ\)](#) covering this location from TerraServer; [USGS Aerial Photograph](#) may also be available from this site. A DOQ is a computer-generated image of an aerial photograph in which image displacement caused by terrain relief and camera tilts has been removed. It combines the image characteristics of a photograph with the geometric qualities of a map. Visit the [USGS](#) for more information.

**Watershed/Stream Flow Information -**

Find the [Watershed](#) for this location using the U.S. Environmental Protection Agency's site.

**Climate Data Sources -**

*Precipitation frequency results are based on data from a variety of sources, but largely NCDC. The following links provide general information about observing sites in the area, regardless of if their data was used in this study. For detailed information about the stations used in this study, please refer to our documentation.*

Using the [National Climatic Data Center's \(NCDC\)](#) station search engine, locate other climate stations within:

...OR...  of this location (33.3325/-111.62). Digital ASCH data can be obtained directly from [NCDC](#).

Find [Natural Resources Conservation Service \(NRCS\)](#) SNOTEL (SNOWpack TELemetry) stations by visiting the [Western Regional Climate Center's state-specific SNOTEL station maps](#).

Hydrometeorological Design Studies Center  
 DOC/NOAA/National Weather Service  
 1325 East-West Highway  
 Silver Spring, MD 20910  
 (301) 713-1669  
 Questions?: [HDSC.Questions@noaa.gov](mailto:HDSC.Questions@noaa.gov)

[Disclaimer](#)

**Proposed Condition HEC-1 Sub-Basin Data**

**Table 1 - Post Developed HEC-1 Sub-Basin Data**

Description: Sub-basin data based on aerial photo and proposed topography

Location: Development Units 8 & 9 - Eastmark - East Mesa, Arizona

Reference: DDMSW Version 4.6.0

ONSITE BASINS										
Sub-Basin ID	Basin Area (sq. ft.)	Basin Area (acres)	Basin Area (sq. mi)	Length (ft)	Length (mi)	Length +10%	USGE (ft)	DSGE (ft)	Lca (ft)	Lca (mi)
16	2,949,384	67.71	0.106	2134	0.40	0.44	1425.0	1410.0	1100	0.21
18	8,921,616	204.81	0.320	3445	0.65	0.72	1435.0	1420.0	1761	0.33
19	2,856,235	65.57	0.102	2394	0.45	0.50	1420.0	1410.0	1082	0.20
20	8,518,170	195.55	0.306	4890	0.93	1.02	1430.0	1412.0	2361	0.45
<b>Totals</b>	<b>23,245,405</b>	<b>533.64</b>	<b>0.834</b>							

**Proposed Condition HEC-1 Soil Data**

**WOOD/PATEL**

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**Table 2 - Post Developed HEC-1 - Soils Data**

Description: Post Developed Soil Data

Location: Development Units 8 &amp; 9 - Eastmark - East Mesa, Arizona

Reference: NRCS Web Soil Survey  
Aguila-Carefree Area Soil Survey

Sub-Basin ID	Soil ID	Soil Type	Area (acres)	Area (sq. mi.)
16	2	Antho Gravelly Sandy Loams	11.65	0.018
	78	Mohall Clay Loam, Calcareous Solum	40.65	0.064
	112	Tremant Gravelly Sandy Loams	15.17	0.024
		<b>TOTAL</b>	<b>67.47</b>	<b>0.106</b>
18	1	Antho Sandy Loams	4.81	0.008
	2	Antho Gravelly Sandy Loams	20.65	0.032
	50	Estrella Loams	79.46	0.124
	77	Mohall Clay Loam	28.64	0.045
	78	Mohall Clay Loam, Calcareous Solum	47.58	0.074
	112	Tremant Gravelly Sandy Loams	13.37	0.021
	115	Tremant-Antho Complex, 1-5 %Slope	10.88	0.017
		<b>TOTAL</b>	<b>205.39</b>	<b>0.321</b>
19	50	Estrella Loams	4.27	0.007
	77	Mohall Clay Loam	22.59	0.035
	78	Mohall Clay Loam, Calcareous Solum	38.32	0.060
		<b>TOTAL</b>	<b>65.18</b>	<b>0.102</b>
20	22	Contine Clay Loam	118.37	0.185
	77	Mohall Clay Loam	27.87	0.044
	78	Mohall Clay Loam, Calcareous Solum	0.27	0.000
	112	Tremant Gravelly Sandy Loams	49.30	0.077
		<b>TOTAL</b>	<b>195.81</b>	<b>0.306</b>

**Proposed Condition HEC-1 Land Use Data**

**WOOD/PATEL**

CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS

**Table 3 - Post Developed HEC-1 Land Use Data**

Description: Land use data based on proposed development

Location Development Units 8 & 9 - Eastmark - East Mesa, Arizona

Reference: DDMSW Version 4.6.0

Sub-Basin ID	Basin Area (sq. ft.)	Basin Area (acres)	Basin Area (sq. mi)	DU	DU Area (ac)	Land Use	Land Use Area (sq. ft.)	Land Use Area (acres)	Land Use Area (sq. mi.)	Kn
16	2949384	67.7	0.1058	DU3	5.7	Medium Lot Residential (2-4 DU/Acre)	248292	5.7	0.0089	0.045
						Medium Lot Residential (2-4 DU/Acre)	2117388	48.6	0.0759	0.040
						Active Open Space	291852	6.7	0.0105	0.050
18	8921616	204.8	0.3200	DU8	204.8	Medium Lot Residential (2-4 DU/Acre)	6691213	153.6	0.0105	0.030
						Active Open Space	1338242	30.7	0.0480	0.040
						General Transportation	892161	20.5	0.0320	0.050
19	2856235	65.6	0.1025	DU8	65.6	Medium Lot Residential (2-4 DU/Acre)	1999365	45.9	0.0717	0.040
						Active Open Space	571247	13.1	0.0205	0.050
						General Transportation	285624	6.6	0.0103	0.030
20	8518170	195.6	0.3056	DU9	195.6	Medium Lot Residential (2-4 DU/Acre)	6388628	146.7	0.2292	0.040
						Active Open Space	1277726	29.3	0.0458	0.050
						General Transportation	851817	19.6	0.0306	0.030



**Proposed Condition HEC-1 Routing Data**

Table 4 - Post Developed HEC-1 Routing Data

Description: Routing parameters based on proposed channels and drainage corridors

Location: Development Units 8 & 9 - Eastmark - East Mesa - Arizona

Reference: DDM/SW Version 4.6.0

Routing ID	Routing Method	LOB N	CHAN N	ROBN	Length (ft)	Slope (ft/ft)	RX1	RX2	LB	RX4	RX5	RB	RX7	RX8	RY1	RY2	LB	RY4	RY5	RB	RY7	RY8	
19719	Normal Depth	0.013	0.013	0.013	1150	0.0020	0.0	15.0	17.0	36.0	112.0	130.0	132.0	146.0	4.00	0.00	0.00	3.00	3.00	0.00	0.00	0.00	4.00
77ATB	Normal Depth	0.045	0.040	0.045	3000	0.0050	0.0	5.0	10.0	37.0	47.0	74.0	79.0	84.0	5.50	5.00	4.50	0.00	0.00	0.00	4.50	5.00	5.50
77BTC	Normal Depth	0.045	0.040	0.045	4750	0.0042	0.0	5.0	10.0	26.0	35.0	105.0	110.0	115.0	5.00	4.00	3.00	0.00	0.00	0.00	3.00	4.00	5.00
77CTA	Normal Depth	0.032	0.032	0.032	4435	0.0020	0.0	5.0	10.0	24.0	32.0	138.0	143.0	148.0	4.50	4.00	3.50	0.00	0.00	0.00	3.50	4.00	4.50
77ATB	Normal Depth	0.045	0.040	0.045	3500	0.0042	0.0	500.0	980.0	1003.0	1007.0	1633.0	1511.0	2011.0	4.50	3.50	3.00	0.00	0.00	0.00	3.00	3.50	4.50
77BTC	Normal Depth	0.035	0.022	0.035	4500	0.0033	0.0	100.0	110.0	115.0	120.0	125.0	130.0	135.0	5.00	4.00	3.50	0.00	0.00	0.00	3.50	4.00	5.00
75CTA	Normal Depth	0.032	0.032	0.032	4215	0.0033	0.0	5.0	10.0	25.0	31.0	97.0	102.0	107.0	5.00	4.50	4.00	0.00	0.00	0.00	4.00	4.50	5.00

**Proposed Condition Onsite Retention Volume Summary**

**Table 5 - Onsite Retention Volume Summary**

Description: Calculation of Required Retention Volume Using the Rational Method

Location Eastmark

Reference: Drainage Design Manual for Maricopa County, Vol. I, Hydrology

Known Values: Design storm: 100-yr, 2-hr  
Rainfall, D: 2.19 inches

Calc. Values:  $V = DAC$   
Where: V = Retention Volume Required  
D = Depth of Rainfall (ft)  
A = Area of Watershed Contributing  
C = Runoff Coefficient

Retention - 100YR, 2HR Volumes (South of the Powerline Floodway)									
Retention Basin	Retention Location	Sub-basin		Development Unit		Weighted "C <sub>100</sub> "	100YR - 2HR Volume Required (acre-feet)	Total 100YR - 2HR Volume Required (acre-feet)	
		Contributing Sub-basin	Contributing Sub-basin Area (acres)	Contributing DUs	DU Area Within Sub-basin (acres)				
RET16	DU-3	16	67.70	DU-3	5.70	0.65	0.7	8.4	
	DU-8			DU-8	62.00	0.68	7.7		
RET18	DU-8	18	204.80	DU-8	204.80	0.68	25.4	25.4	
RET19	DU-8	19	65.60	DU-8	65.60	0.68	8.1	8.1	
RET20	DU-9	20	195.60	DU-9	195.60	0.68	24.3	24.3	
<b>Total</b>								<b>66.2</b>	<b>ac-ft</b>



**APPENDIX B**  
**Existing and Preliminary Channel Calculations**

## Worksheet for 77CT78B - Ray to Galveston

### Project Description

Friction Method                      Manning Formula  
Solve For                                Normal Depth

### Input Data

Roughness Coefficient	0.032	
Channel Slope	0.00240	ft/ft
Left Side Slope	6.00	ft/ft (H:V)
Right Side Slope	3.00	ft/ft (H:V)
Bottom Width	32.00	ft
Discharge	510.00	ft <sup>3</sup> /s

### Results

Normal Depth	2.91	ft	<i>PROPOSED CHANNEL DEPTH = 4 ft FREEBOARD = 1 ft</i>
Flow Area	131.37	ft <sup>2</sup>	
Wetted Perimeter	58.93	ft	
Hydraulic Radius	2.23	ft	
Top Width	58.21	ft	
Critical Depth	1.82	ft	
Critical Slope	0.01316	ft/ft	
Velocity	3.88	ft/s	
Velocity Head	0.23	ft	
Specific Energy	3.15	ft	
Froude Number	0.46		
Flow Type	Subcritical		

### SVF Input Data

Downstream Depth	0.00	ft
Length	0.00	ft
Number Of Steps	0	

### SVF Output Data

Upstream Depth	0.00	ft
Profile Description		
Profile Headloss	0.00	ft
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	2.91	ft
Critical Depth	1.82	ft
Channel Slope	0.00240	ft/ft

## Cross Section for 77CT78B - Ray to Galveston

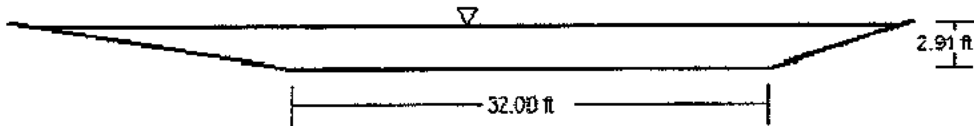
### Project Description

Friction Method                      Manning Formula  
Solve For                                Normal Depth

### Input Data

Roughness Coefficient	0.032
Channel Slope	0.00240 ft/ft
Normal Depth	2.91 ft
Left Side Slope	6.00 ft/ft (H:V)
Right Side Slope	3.00 ft/ft (H:V)
Bottom Width	32.00 ft
Discharge	510.00 ft <sup>3</sup> /s

### Cross Section Image



V: 1  
H: 1

## Worksheet for 77CT78B - Galveston to Williamsfield

### Project Description

Friction Method                      Manning Formula  
Solve For                                Normal Depth

### Input Data

Roughness Coefficient	0.032	
Channel Slope	0.00190	ft/ft
Left Side Slope	6.00	ft/ft (H:V)
Right Side Slope	3.00	ft/ft (H:V)
Bottom Width	70.00	ft
Discharge	946.00	ft <sup>3</sup> /s

### Results

Normal Depth	2.99	ft
Flow Area	249.87	ft <sup>2</sup>
Wetted Perimeter	97.68	ft
Hydraulic Radius	2.56	ft
Top Width	96.94	ft
Critical Depth	1.72	ft
Critical Slope	0.01295	ft/ft
Velocity	3.79	ft/s
Velocity Head	0.22	ft
Specific Energy	3.22	ft
Froude Number	0.42	
Flow Type	Subcritical	

*PROPOSED CHANNEL  
DEPTH = 4 ft  
FREE BOARD = 1 ft*

### GVF Input Data

Downstream Depth	0.00	ft
Length	0.00	ft
Number Of Steps	0	

### GVF Output Data

Upstream Depth	0.00	ft
Profile Description		
Profile Headloss	0.00	ft
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	2.99	ft
Critical Depth	1.72	ft
Channel Slope	0.00190	ft/ft



---

## Worksheet for 77CT78B - Galveston to Williamsfield

---

### CVE Output Data

Critical Slope

0.01295 ft/ft

### Messages

Notes

From Galveston Street to Williams Field Road



# WILLIAMSFIELD CHANNEL SIGNAL BUTTE TO PACIFIC PROVING GROUNDS

## Worksheet for 78CT79

### Project Description

Friction Method                      Manning Formula  
Solve For                                Normal Depth

### Input Data

Roughness Coefficient	0.032	
Channel Slope	0.00330	ft/ft
Left Side Slope	3.00	ft/ft (H:V)
Right Side Slope	6.00	ft/ft (H:V)
Bottom Width	51.00	ft
Discharge	946.00	ft <sup>3</sup> /s

### Results

Normal Depth	3.01	ft	<b>PROPOSED CHANNEL DEPTH = 4 ft FREEBOARD = 1 ft</b>
Flow Area	194.33	ft <sup>2</sup>	
Wetted Perimeter	78.83	ft	
Hydraulic Radius	2.47	ft	
Top Width	78.10	ft	
Critical Depth	2.07	ft	
Critical Slope	0.01241	ft/ft	
Velocity	4.87	ft/s	
Velocity Head	0.37	ft	
Specific Energy	3.38	ft	
Froude Number	0.54		
Flow Type	Subcritical		

### GVF Input Data

Downstream Depth	0.00	ft
Length	0.00	ft
Number Of Steps	0	

### GVF Output Data

Upstream Depth	0.00	ft
Profile Description		
Profile Headloss	0.00	ft
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	3.01	ft
Critical Depth	2.07	ft
Channel Slope	0.00330	ft/ft

---

## Worksheet for 78CT79

---

GVE Output Data

Critical Slope

0.01241 ft/ft

## Cross Section for 78CT79

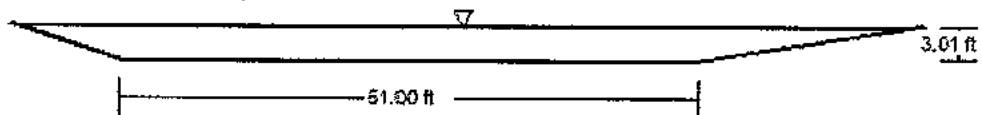
### Project Description

Friction Method                      Manning Formula  
Solve For                                Normal Depth

### Input Data

Roughness Coefficient	0.032
Channel Slope	0.00330 ft/ft
Normal Depth	3.01 ft
Left Side Slope	3.00 ft/ft (H:V)
Right Side Slope	6.00 ft/ft (H:V)
Bottom Width	51.00 ft
Discharge	946.00 ft <sup>3</sup> /s

### Cross Section Image



V: 1  
H: 1

**PLATE 1**  
**Vicinity Map**



**PLATE 2**  
**Soils Map**



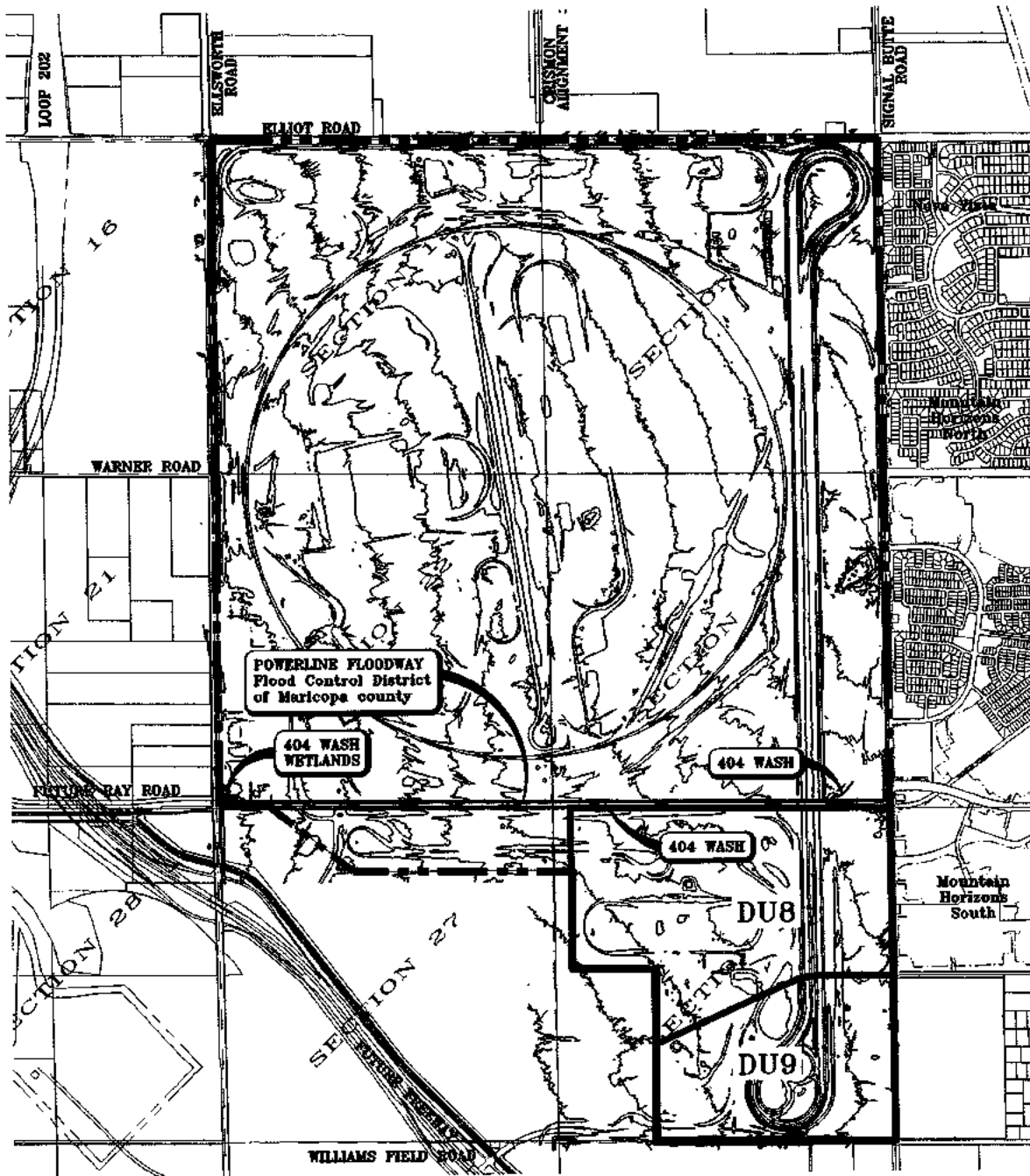


**PLATE 3**  
**Flood Insurance Rate Map**



**PLATE 4**

**Section 404 Jurisdictional Delineation Map**



LEGEND	
404 WASH	—————
404 WASH WETLANDS	—————
PROPERTY BOUNDARY	—————
5 FT. CONTOUR	~~~~~ 1430



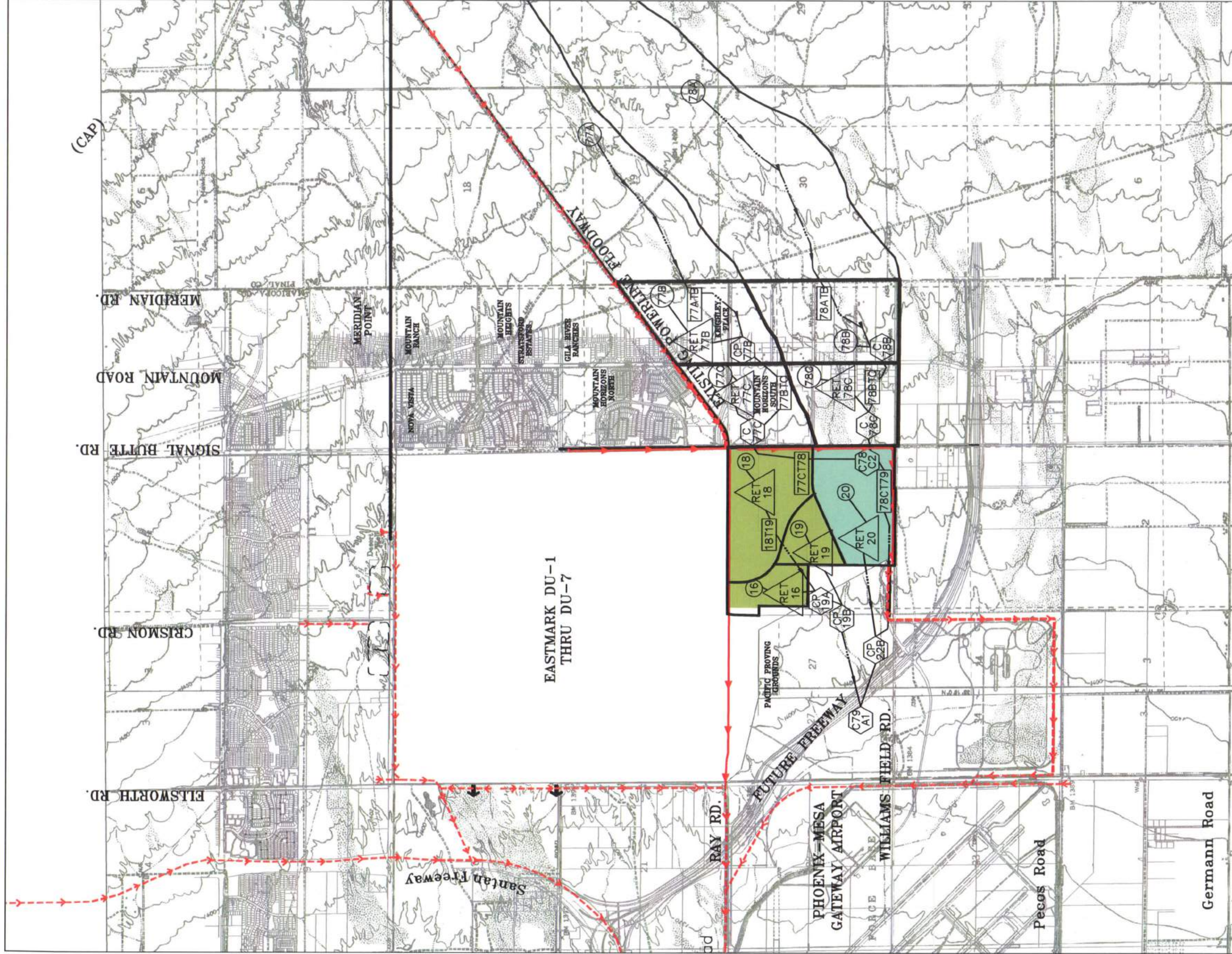
\Users\perry\mwork\001232008\Project\Report\Map\404\404\_Crossing\404.dwg - Feb 03/2013 1:10pm

**PLATE 4: 404 JURISDICTIONAL DELINEATION MAP**  
 EASTMARK - DEVELOPMENT UNITS 8 & 9  
 MESA, ARIZONA  
 FEBRUARY 1, 2013

NOT FOR CONSTRUCTION  
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 www.woodpatel.com  
 PROJECT: MESA-TUCSON

**PLATE 5**  
**Post-Developed HEC-1 Schematic**



EASTMARK DU-1  
THRU DU-7

LOCATION ID	DISCHARGE (CFS)
CP19A	58
CP19B	126
CP22B	939
C79A1	1093

- LEGEND**
- SUB-BASIN BOUNDARY
  - EXISTING STORM DRAIN
  - PROPOSED CHANNEL AND/OR STORM DRAIN SYSTEM
  - - - EXISTING CHANNEL OR STORM DRAIN
  - ROUTING
  - 5 FT. CONTOUR
  - FLOW DIRECTION ARROW

- DU-8
- DU-9
- WATERSHED ID (77C)
- ROUTING ID (77BTC)
- CONCENTRATION POINT ID (C 77C)
- RETENTION ID (RET 73B)



NOT FOR CONSTRUCTION  
OR RECORDING

PLATE 5: POST DEVELOPED HEC-1 SCHEMATIC - SHEET 1  
EASTMARK - DEVELOPMENT UNITS 8 & 9  
MARICOPA COUNTY, ARIZONA  
FEBRUARY 1, 2013

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